

Performance-Based Seismic Design Guidelines for Tall Buildings

Ch. 5: Seismic Input (Ground Motion)



TBI Committee Members



PEER

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Presentation

1. Seismic Hazard Analysis (SHA)

- Probabilistic
- Deterministic
- Site-Response Analysis

2. Soil-Foundation-Structure Interaction

- Kinematic
- Inertial
- Input Motion



Presentation (cont.)

3. Selection and Scaling of Accelerograms

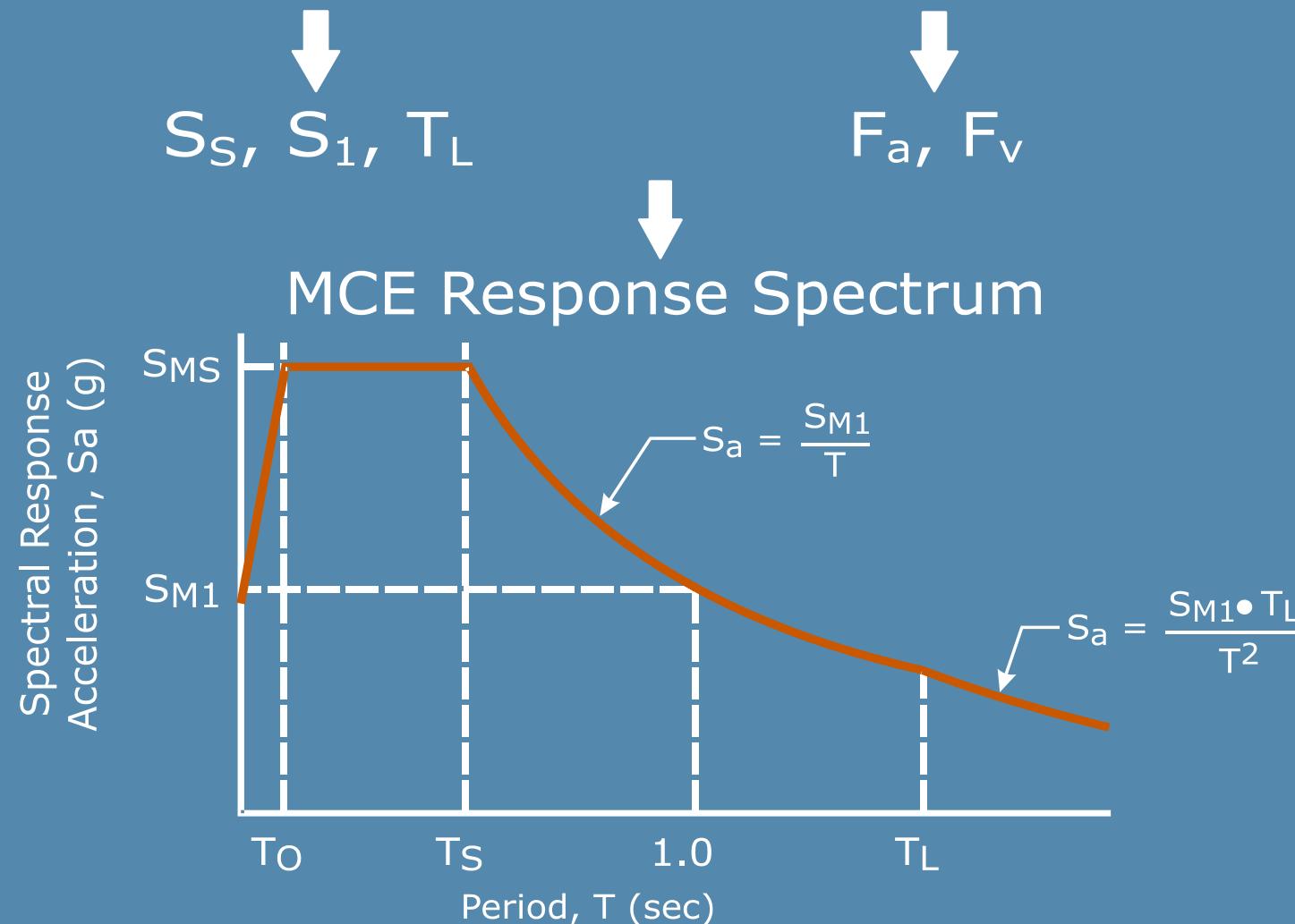
- Identification of Controlling Seismic Sources
- Accelerogram Selection Guidelines
- Accelerogram Modifications



Two SHA Approaches

1. General Procedure - Ch. 11 of ASCE 7-05

USGS MCE Maps & Site Coefficients



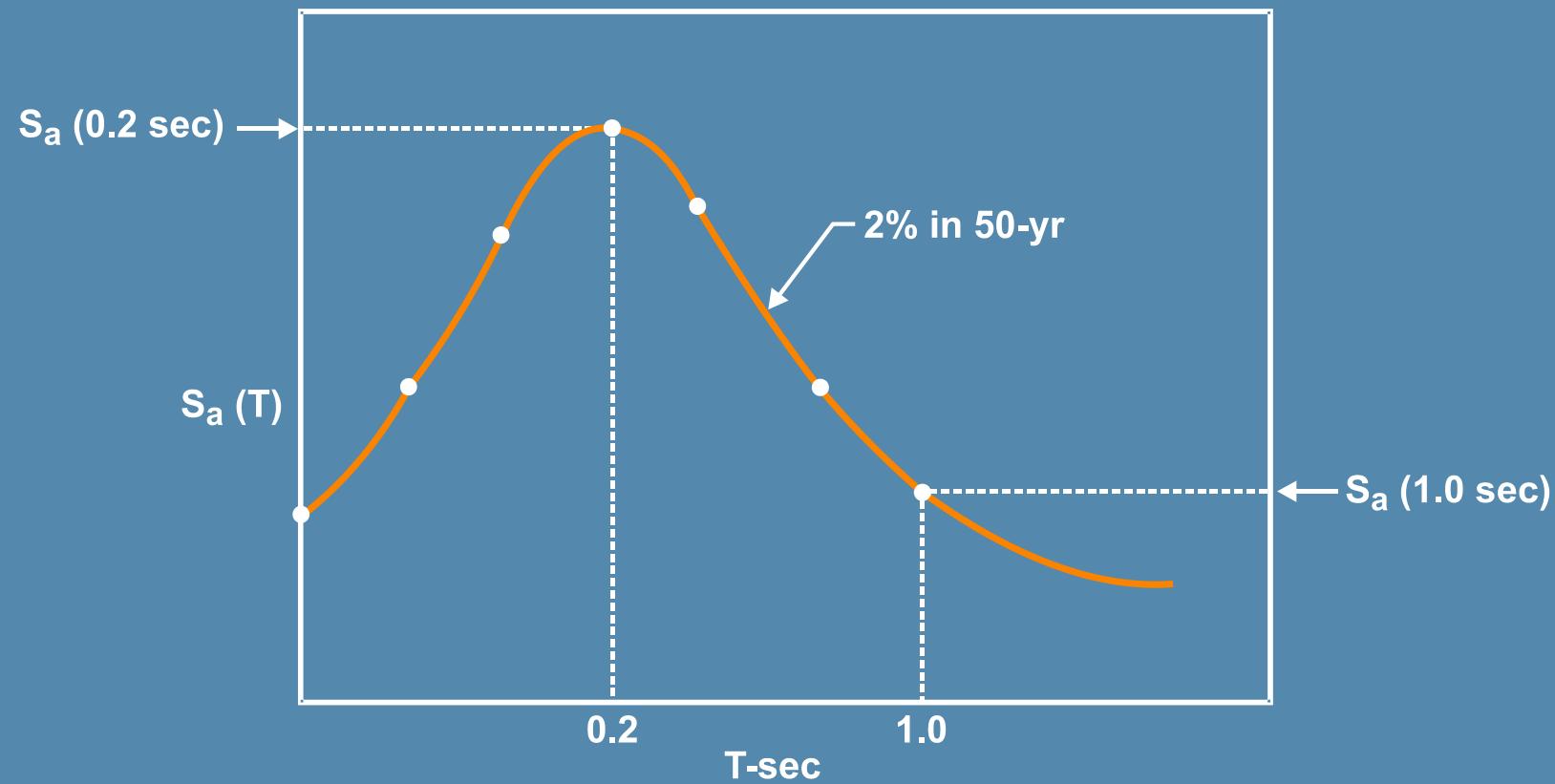
Two SHA Approaches (cont.)

2. Site-Specific (Preferred)

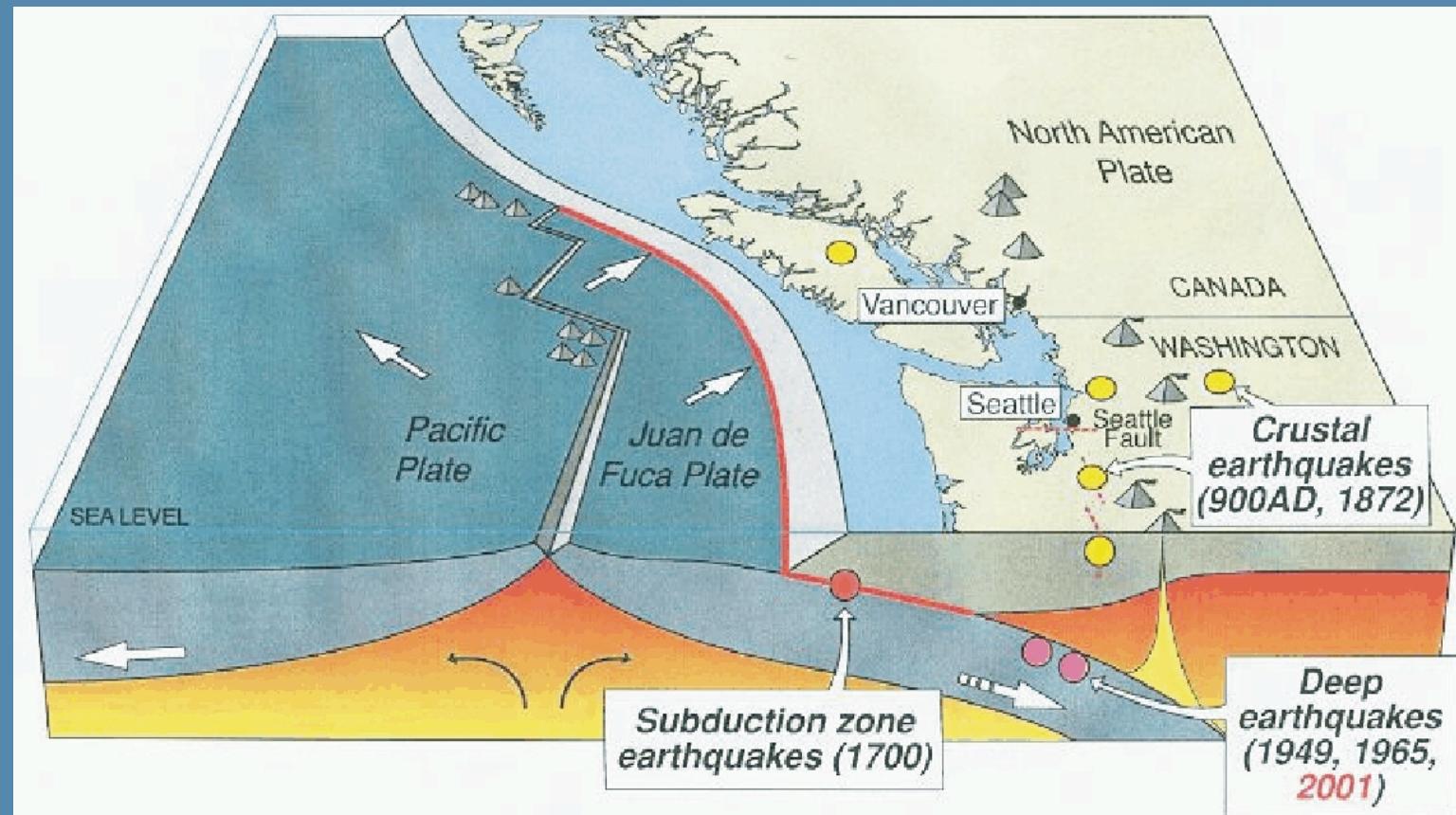
- Probabilistic
- Deterministic



Uniform Hazard Spectrum



Cascadia Earthquake Sources

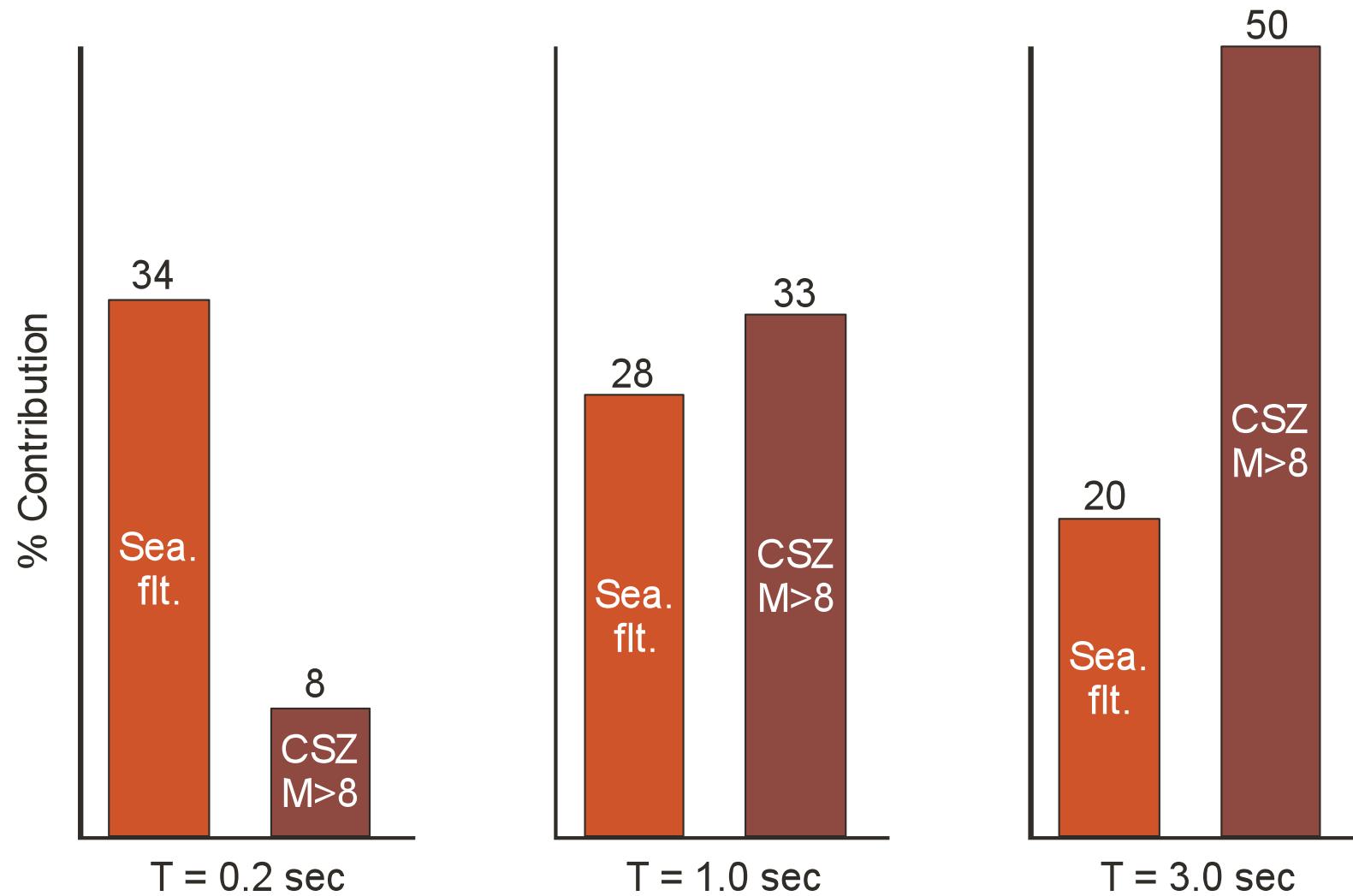


| Source | Affected area | Max. Size | Recurrence |
|---------------------------|---------------|-----------|-----------------|
| ● Subduction Zone | W.WA, OR, CA | M 9 | 500-600 yr |
| ● Deep Juan de Fuca plate | W.WA, OR, | M 7+ | 30-50 yr |
| ● Crustal faults | WA, OR, CA | M 7+ | Hundreds of yr? |

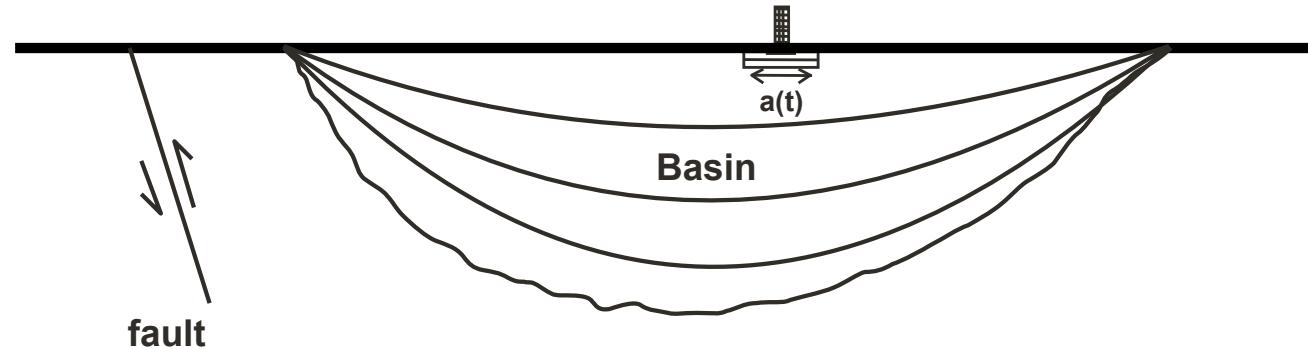


Contribution to 2475-yr Ground Motion Hazard

Lavizzo Park, Seattle (47.6° N, 122.3° W)
2008 USGS PSHA



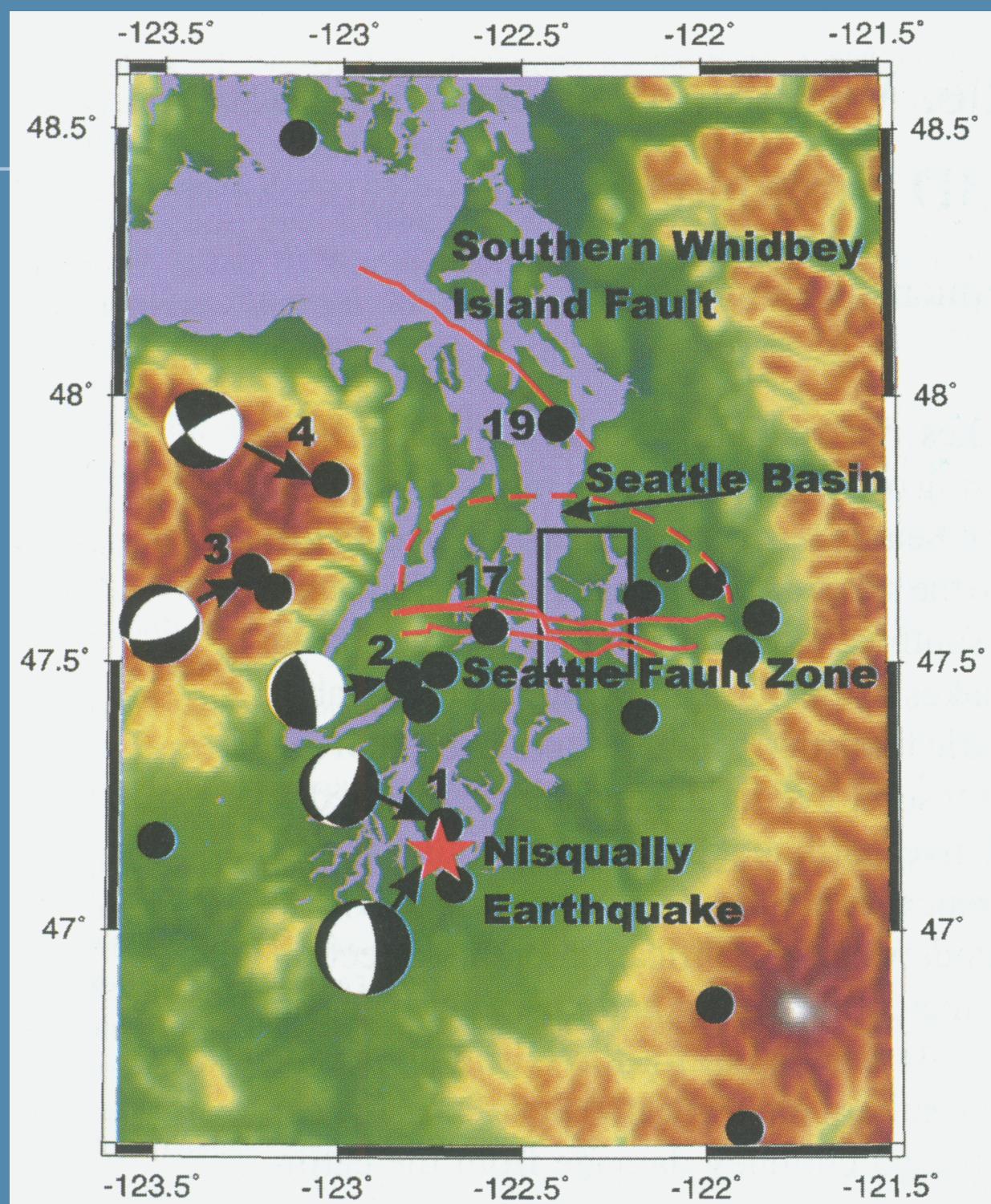
Basin Effects



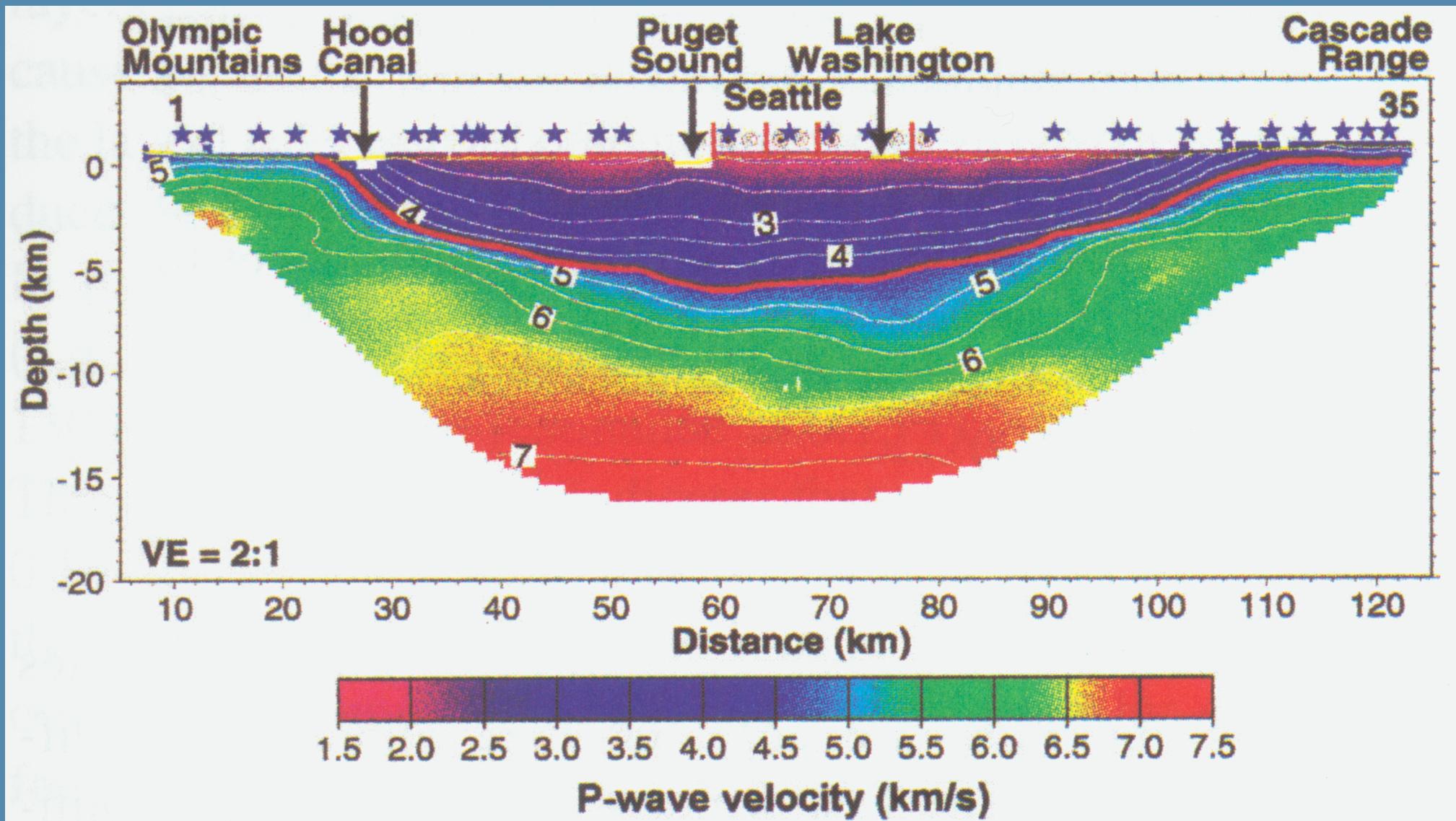
- Amplify long period motions
- Increase duration

Seattle Basin

Ref.: Frankel et al. (2009)



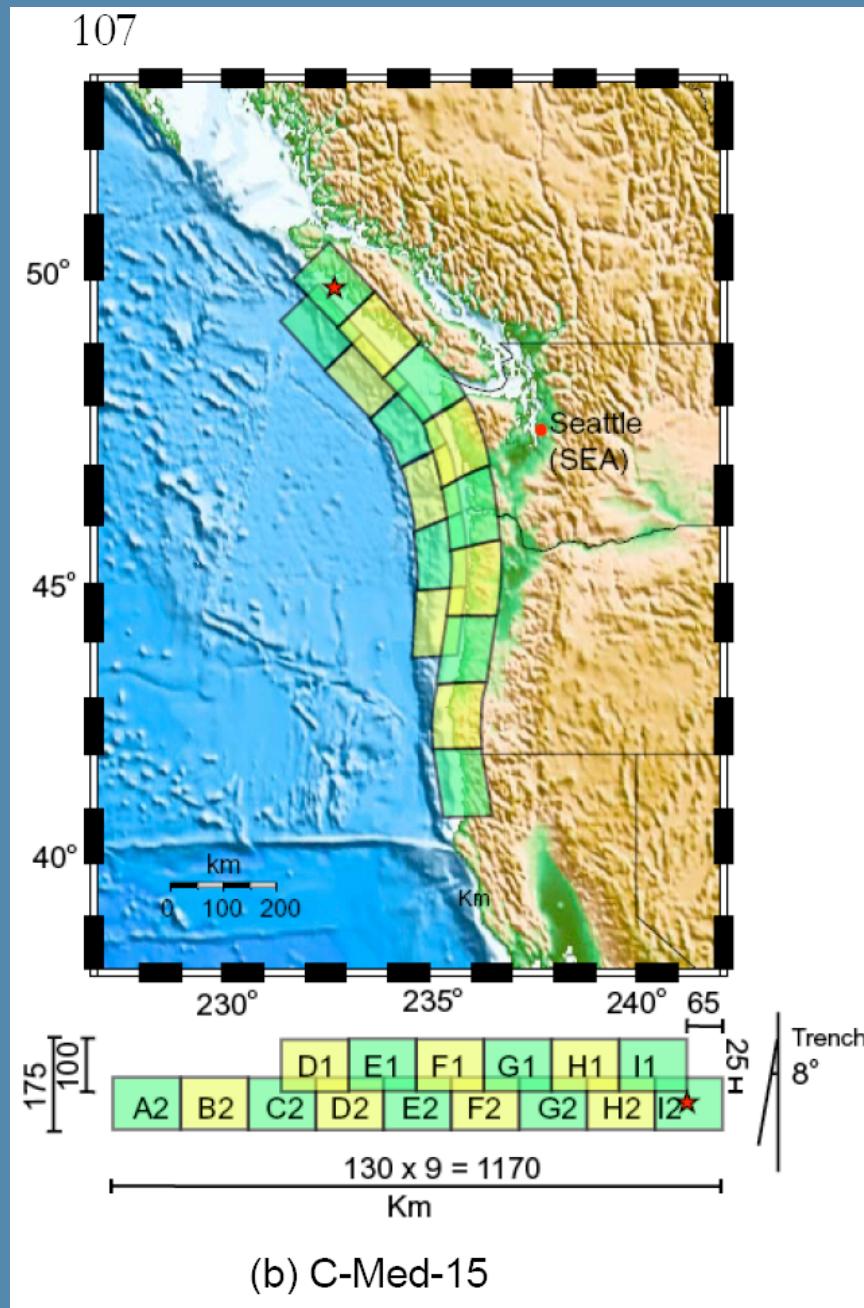
Seattle Basin – EW Profile



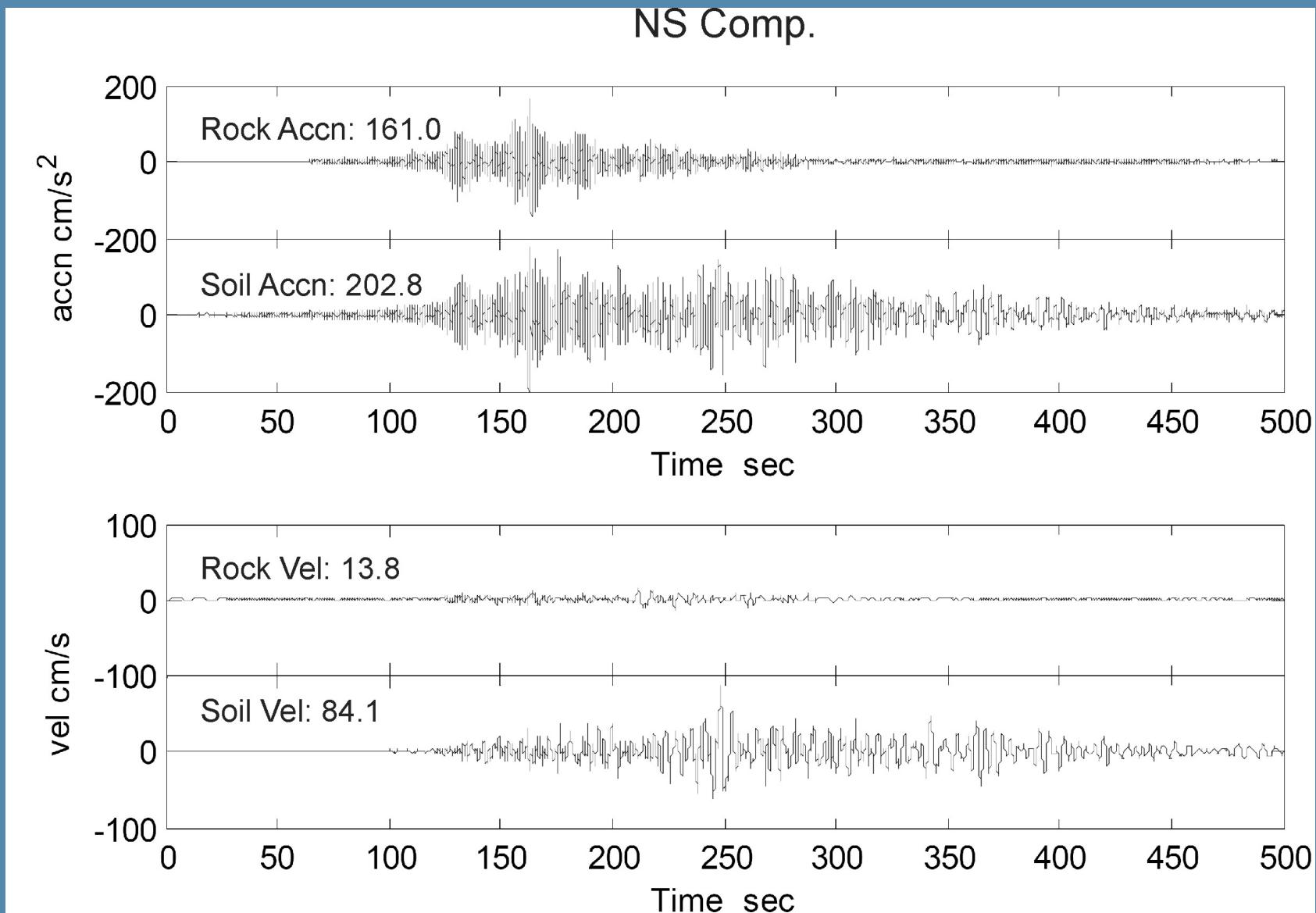
Ref.: Pratt et al. (2003)



CSZ M 9.2 Scenario (Yang, 2009)



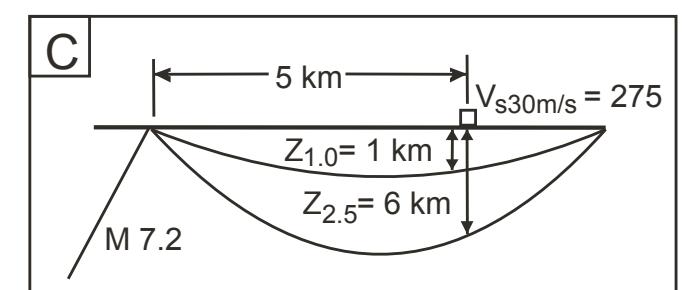
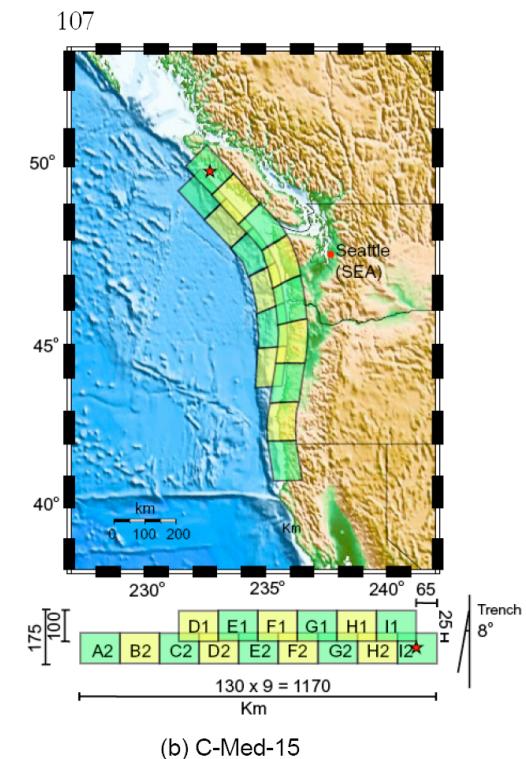
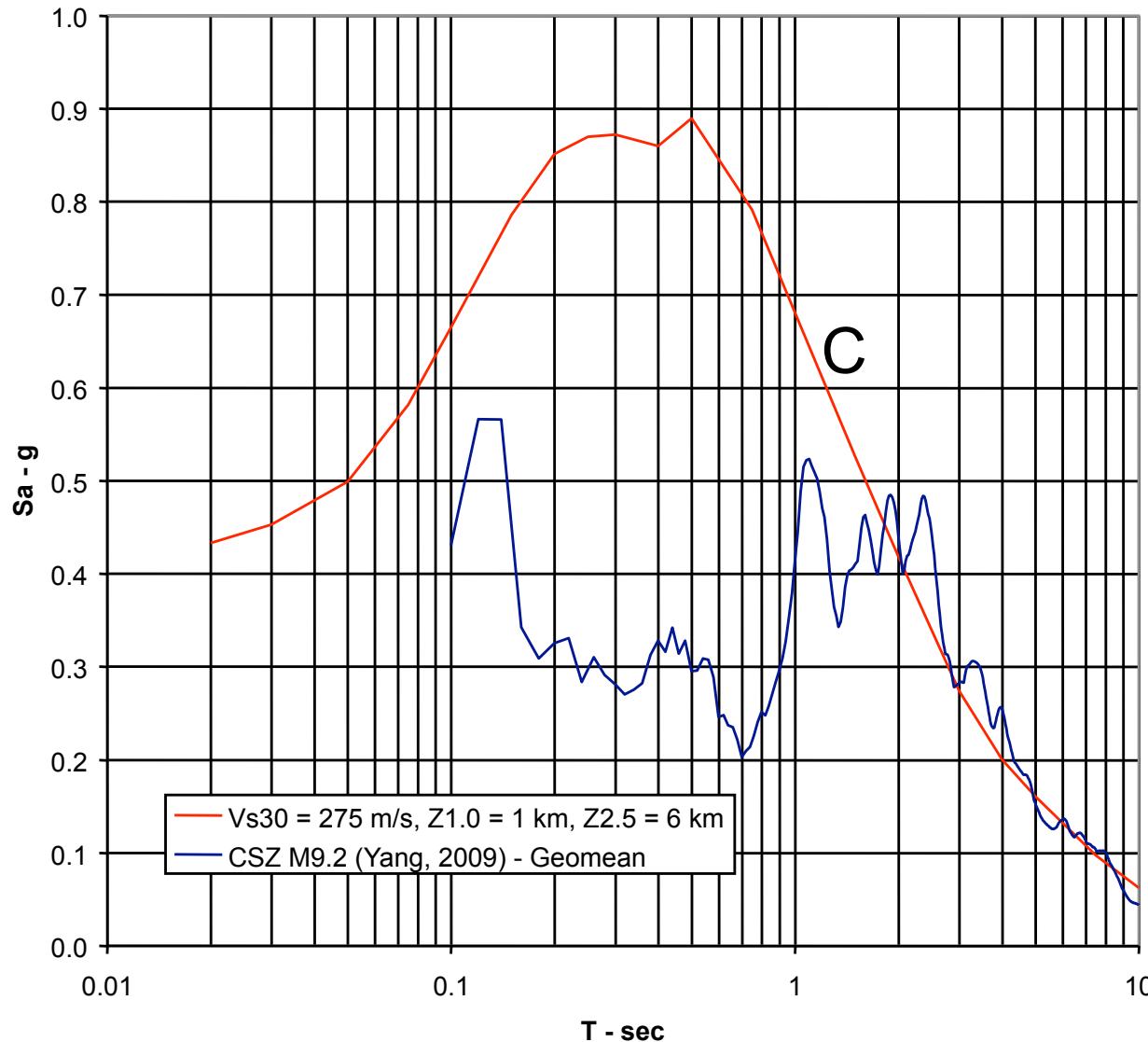
Simulated CSZ M 9.2 Rock & Soil (Basin) Ground Motions for Seattle



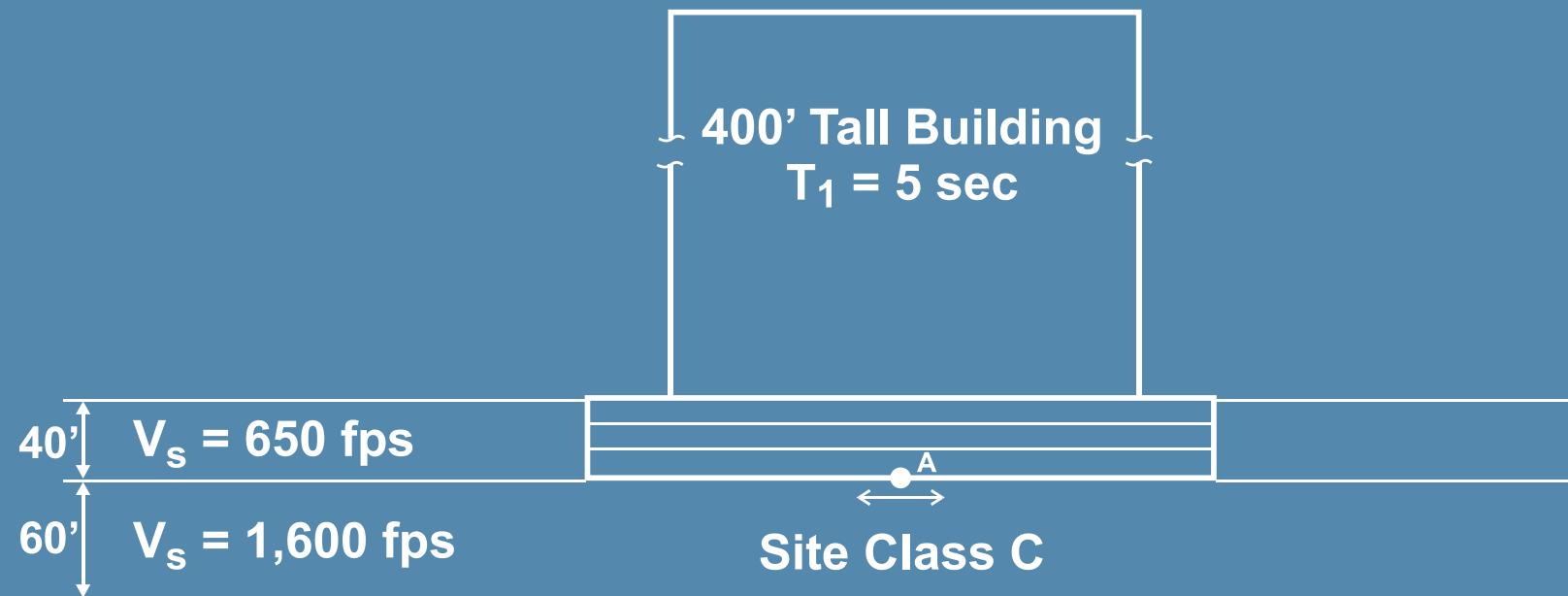
Ref.: Yang (2009)



Response Spectra for Seattle Fault & CSZ Scenarios

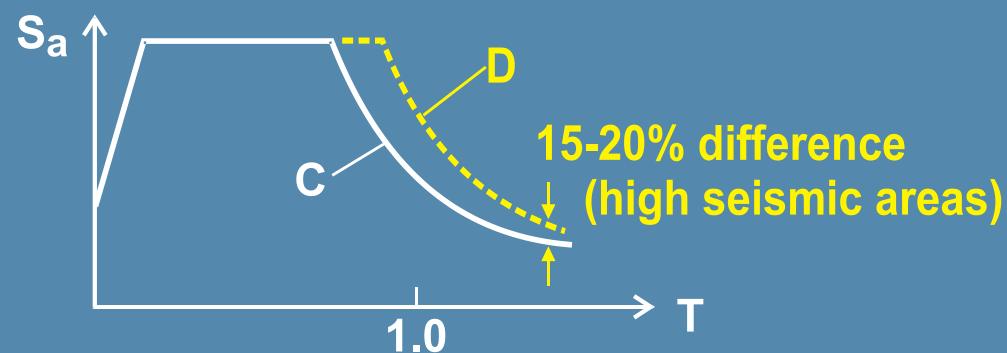


Building Input Motion

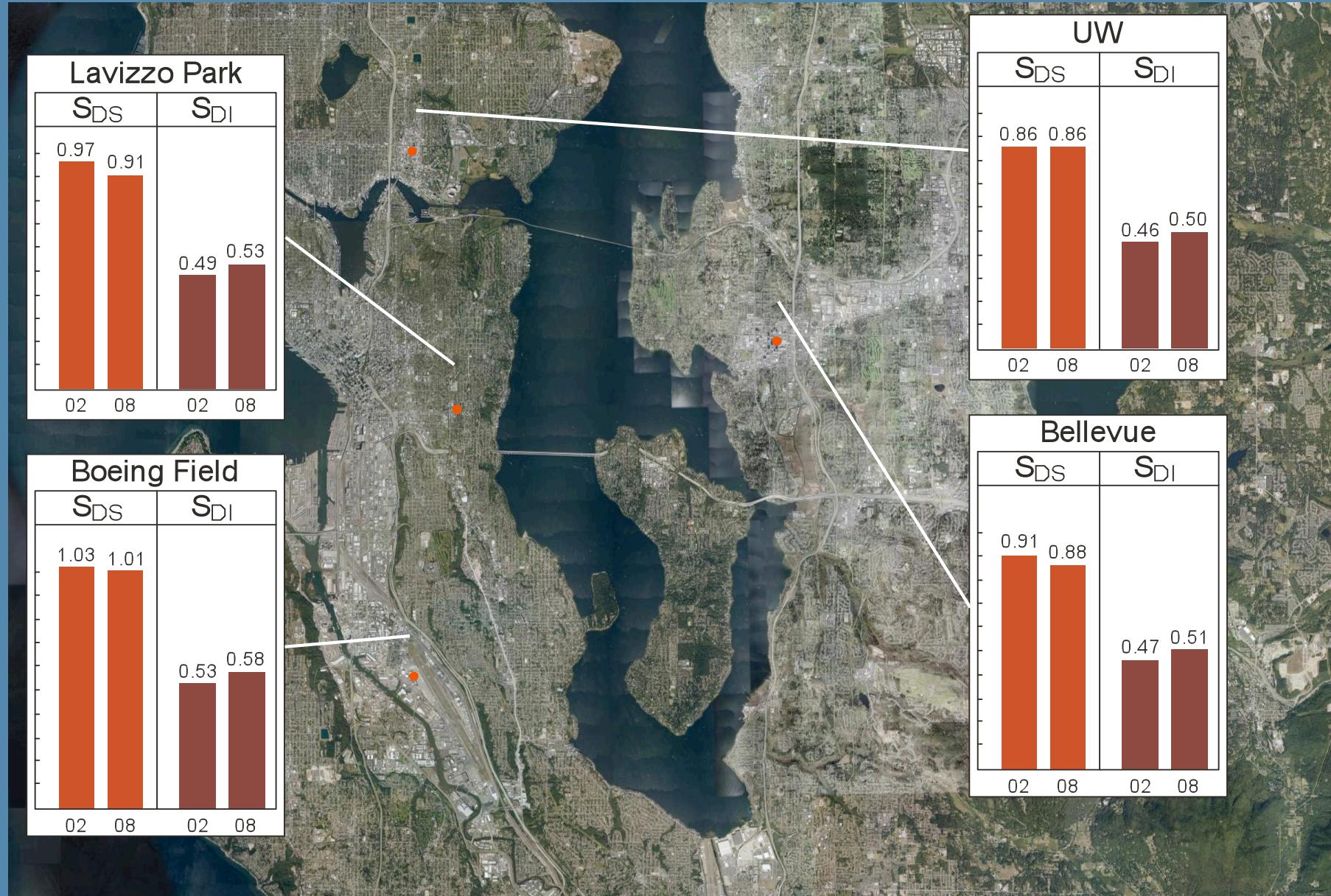


$$\bar{V}_s^{30} = 1,010 \text{ fps}$$

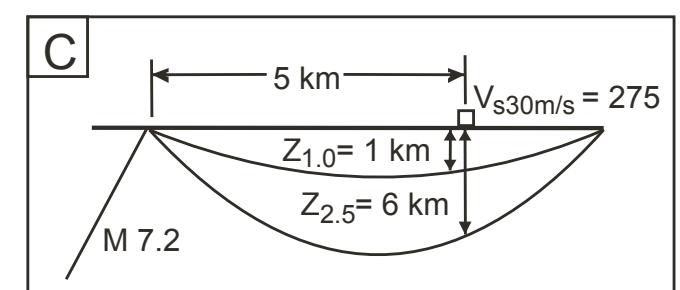
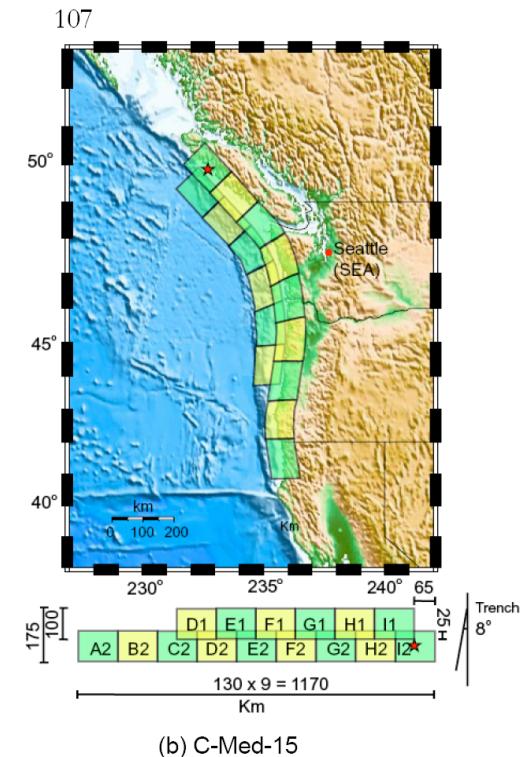
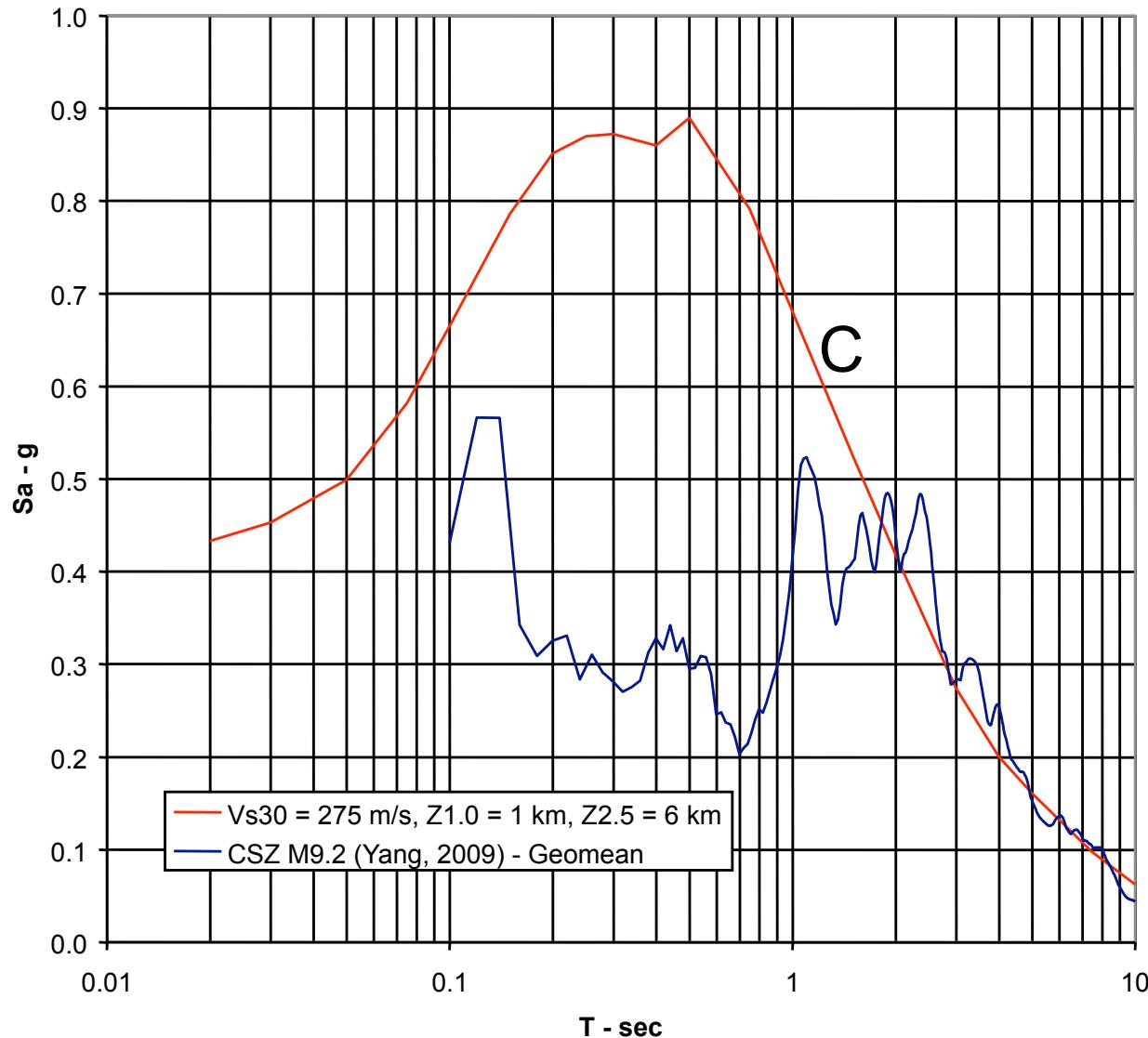
Site Class D



2002 & 2008 USGS S_{DS} & S_{DI} , Site Class D, Seattle



Response Spectra for Seattle Fault & CSZ Scenarios



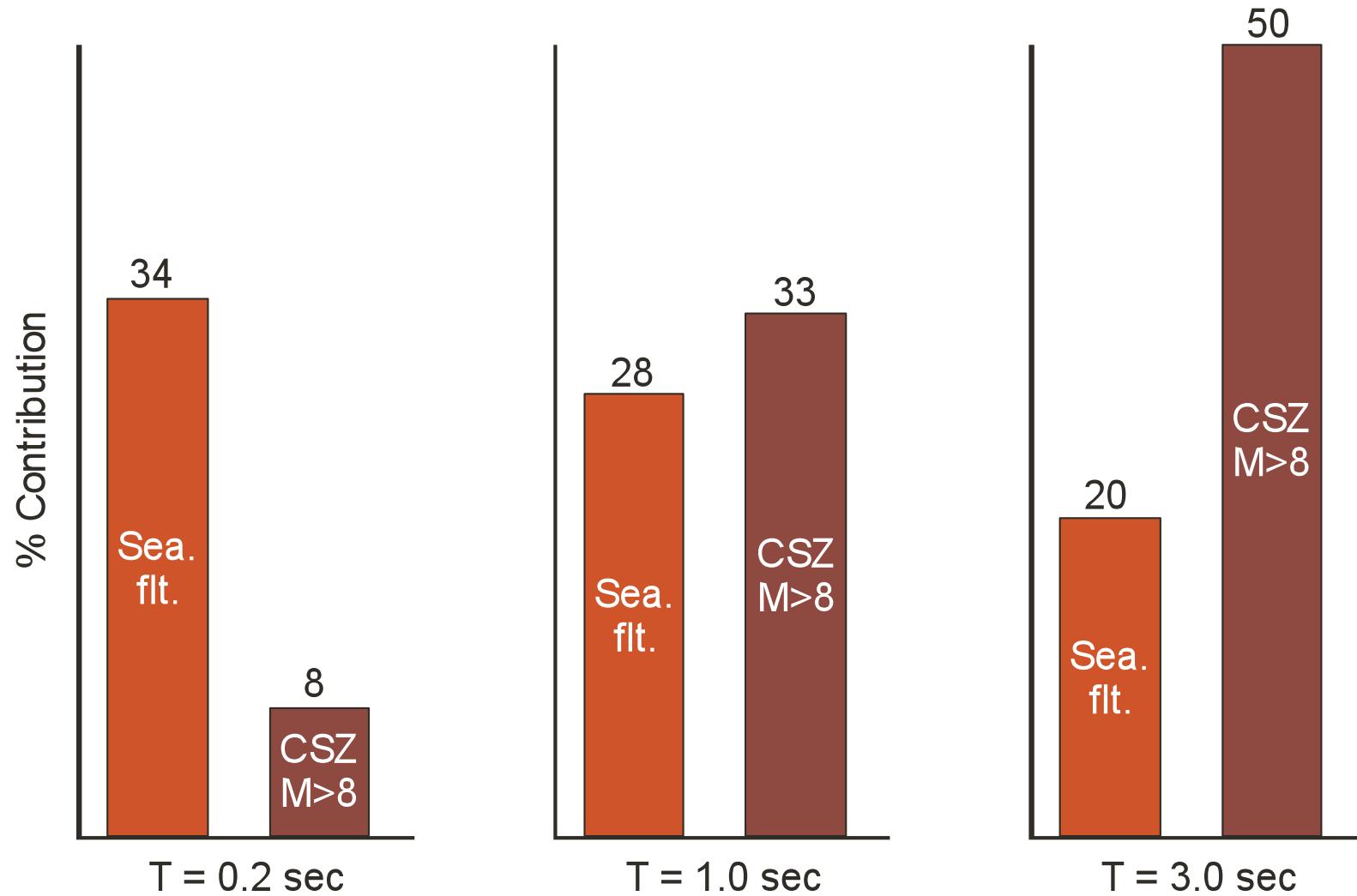
Accelerogram Selection and Scaling

- Identify controlling earthquakes
- Select representative accelerograms
- Modify accelerograms to match target
 S_a

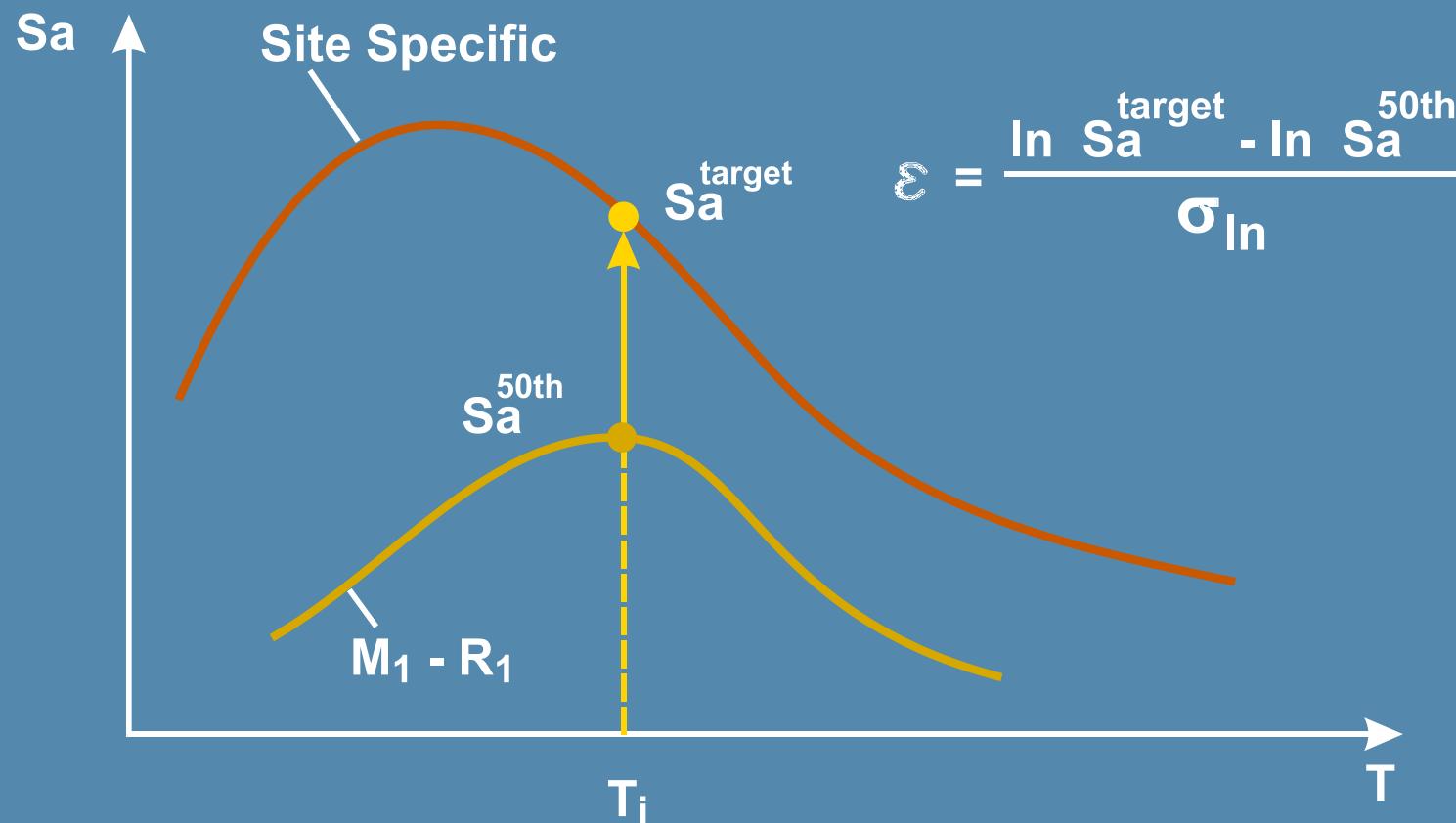


Contribution to 2475-yr Ground Motion Hazard

Lavizzo Park, Seattle (47.6° N, 122.3° W)
2008 USGS PSHA



CMS – ε Parameter



$M_1, R_1, \varepsilon \longrightarrow$ select accelerograms

Baker and Cornell (WUS shallow crustal EQ's)



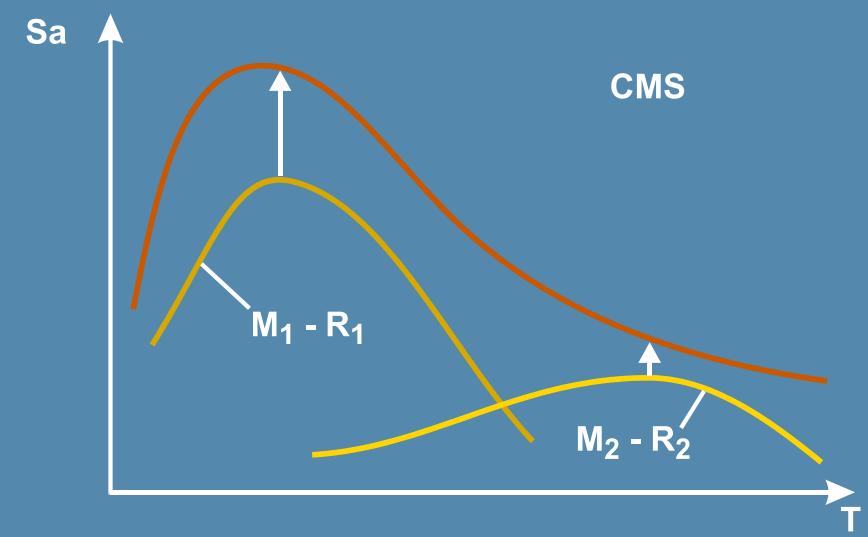
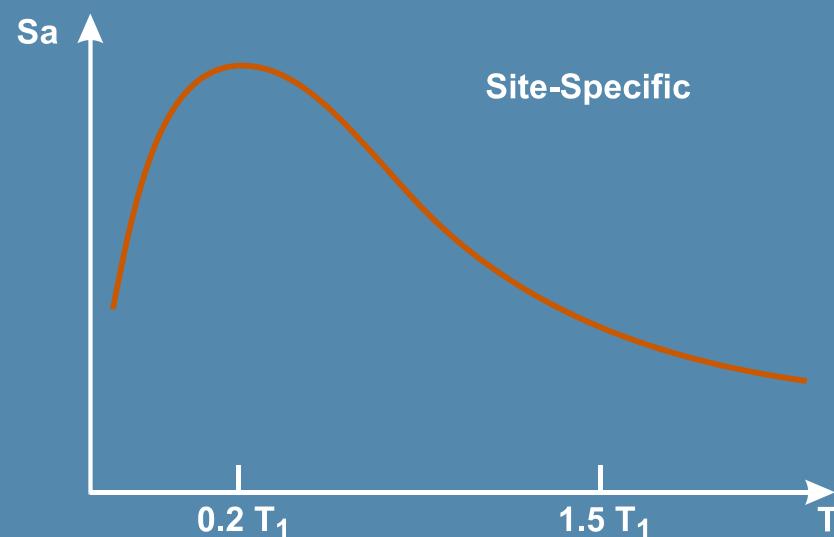
Number of Accelerograms - N

- 7 (minimum)
- Maximum number
 - SE and GE decision



N depends on:

- # controlling earthquakes
- Median/mean or maximum structural response
- Target Sa

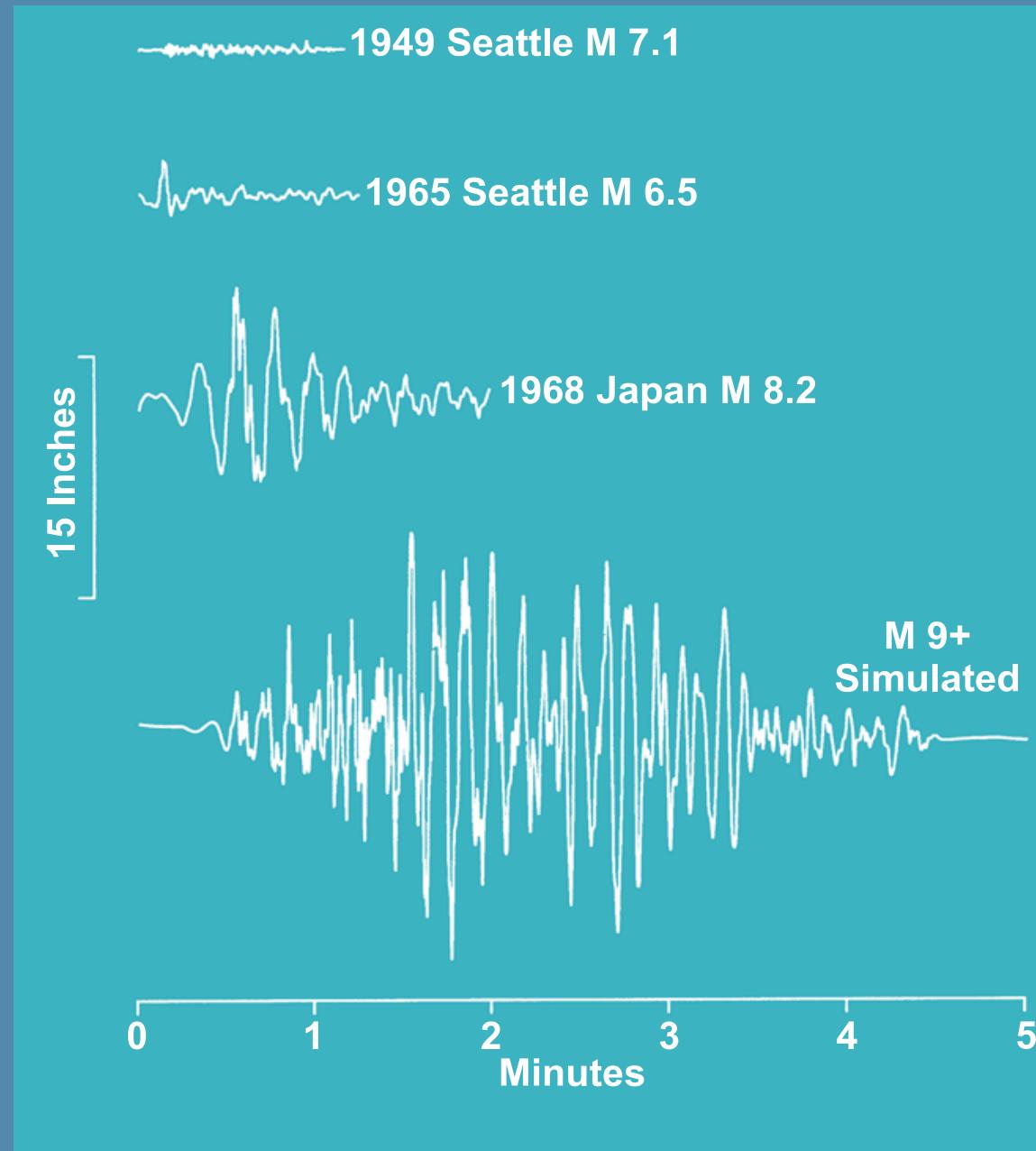


$M \geq \sim 8$, Long Duration Motion

- San Andreas fault $M \sim 8$
- Cascadia and S. Alaska subduction zone
 $M 9+$



Past and Future Seattle Ground Motions



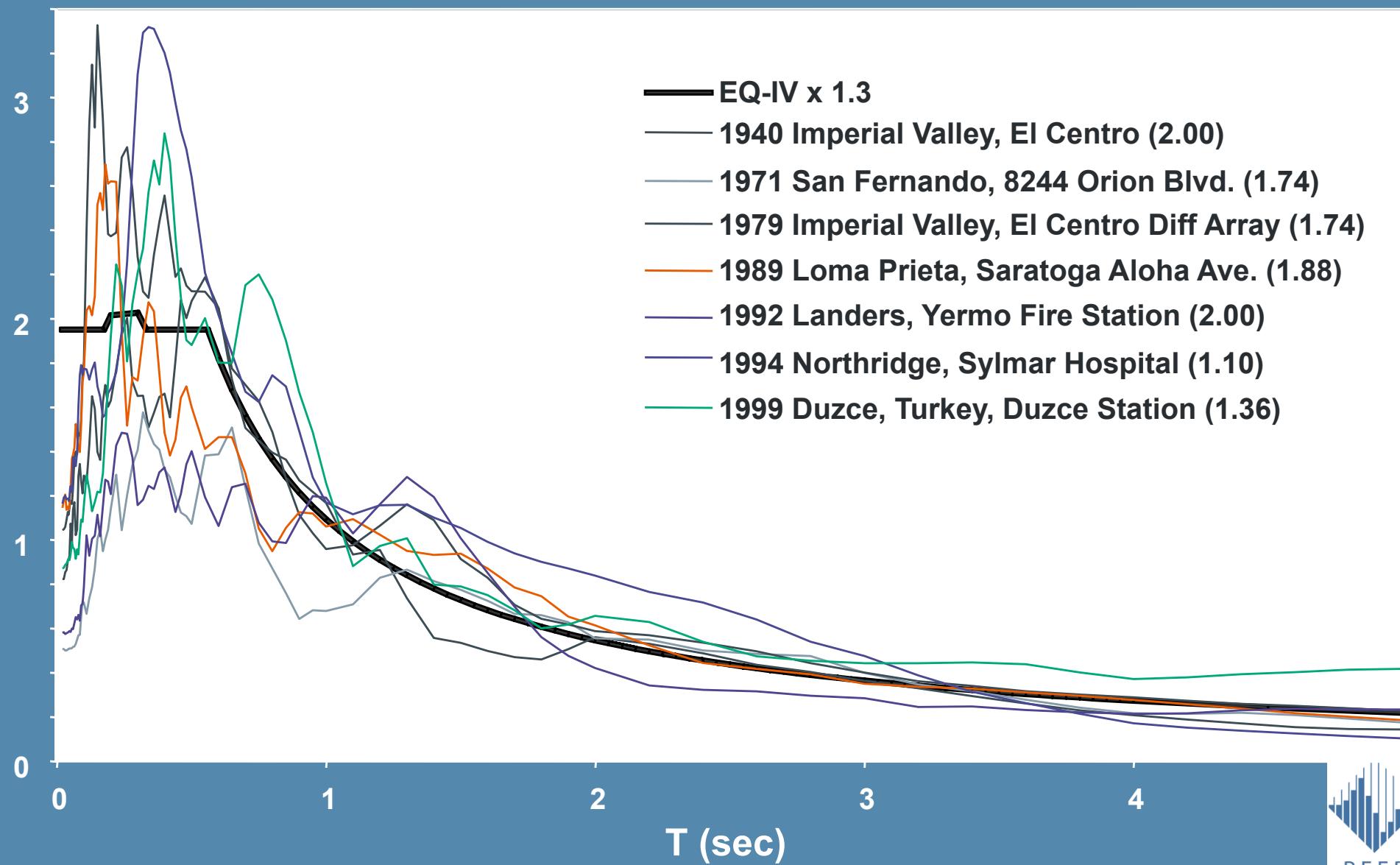
Accelerogram Modification

- Constant Scaling
- Spectral Matching

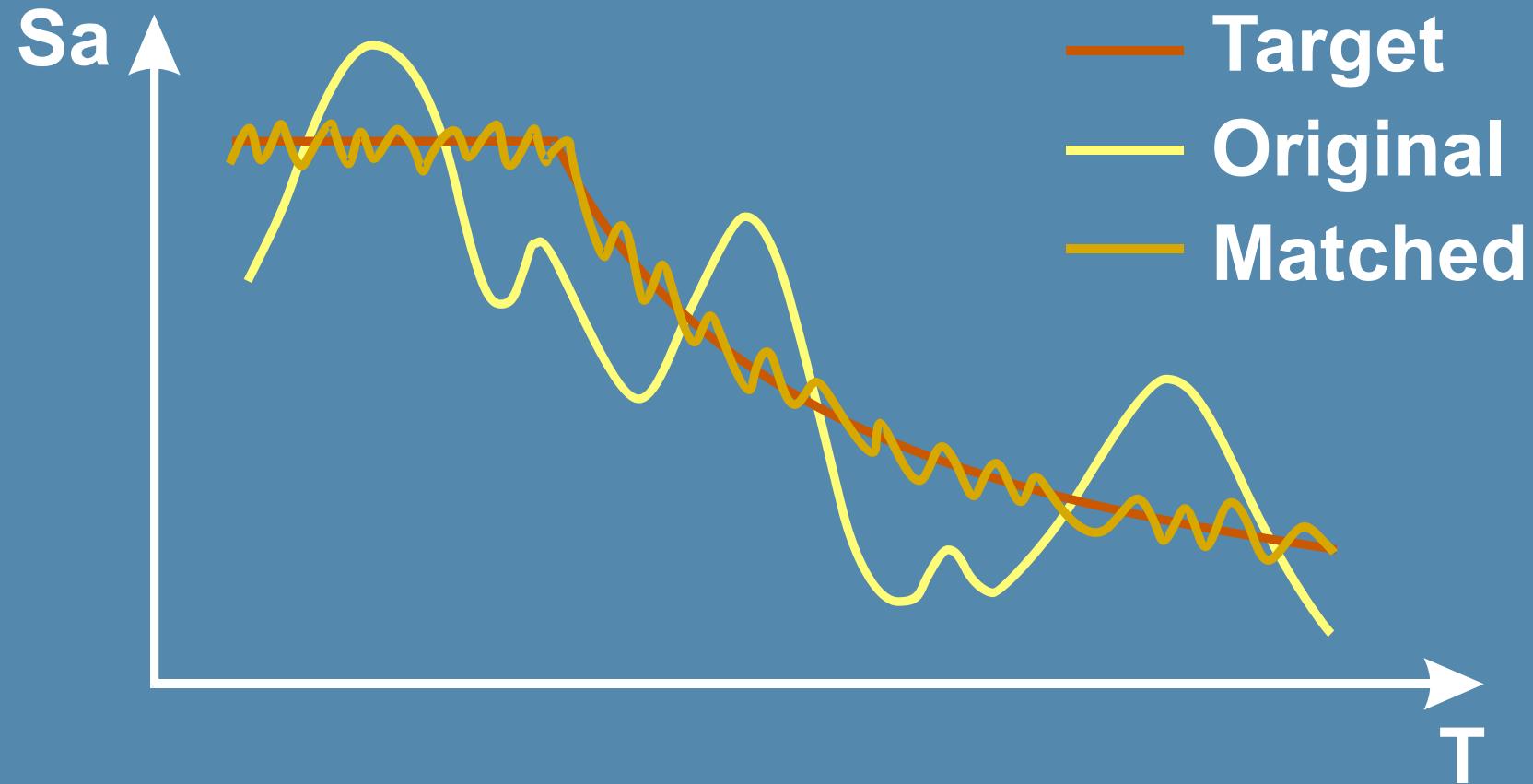


Constant Scaling Method

S_a (g)



Spectral Matching



Accelerogram Selection and Scaling Recommendations

- $N \geq 7$ (max N limited by \$ and time)
- Use M-R deaggregations → controlling EQs
- CMS – use for multiple M-R → different Sa shapes
- Scaling (constant or spectral matching)
SE's decision
- Simulated Accelerograms ($M \geq \sim 8$)
 - + - long duration and basin effects
 - - very limited no. qualified providers
- Peer Review – Extremely Important



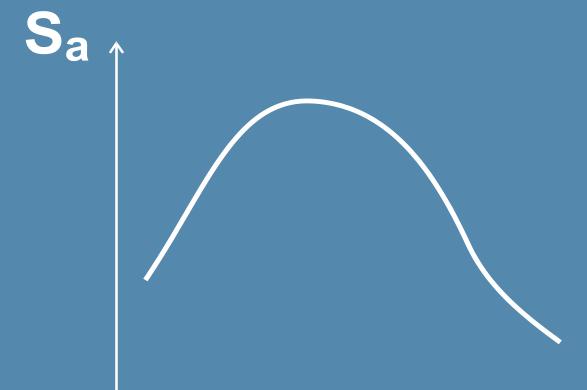
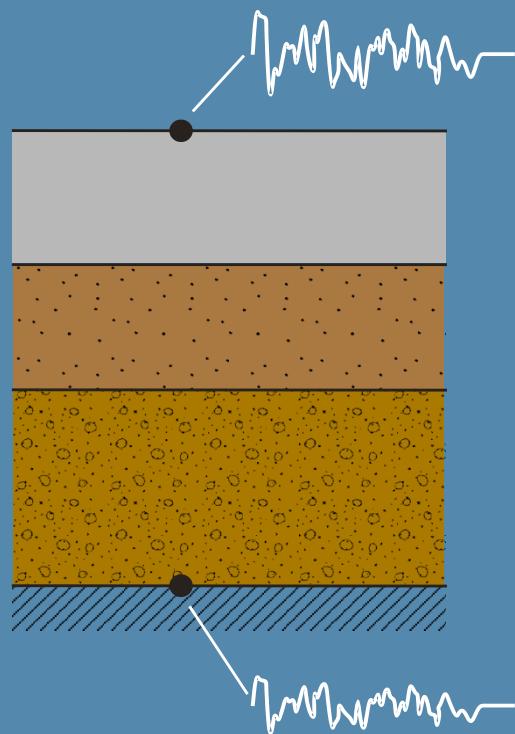
Site Response Analysis

ASCE 7-05; Ch.21
Site-Specific Ground Motion

SRA

PSHA/DSHA

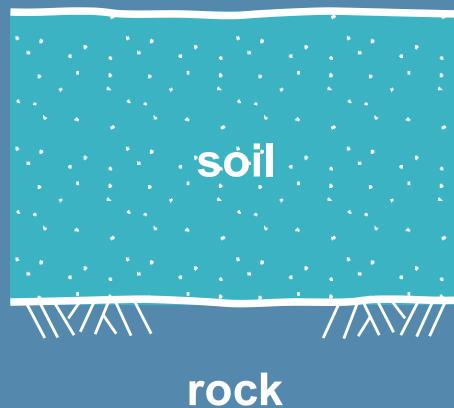
and/or



Recommendations

- Don't do SRA for stiff soil sites; account for local geology through GMPE (i.e., select stiff soil GMPE or appropriate site terms in GMPE)

Possible Exceptions

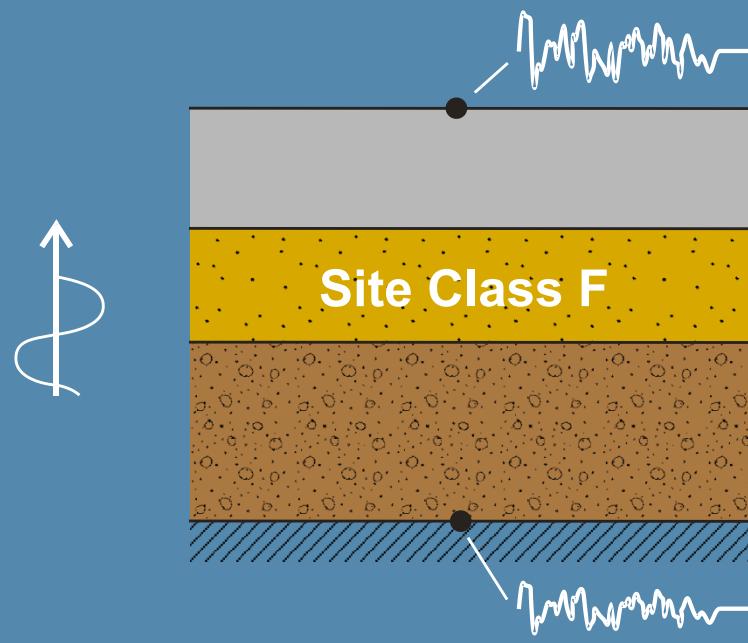


Required in ASCE 7-05

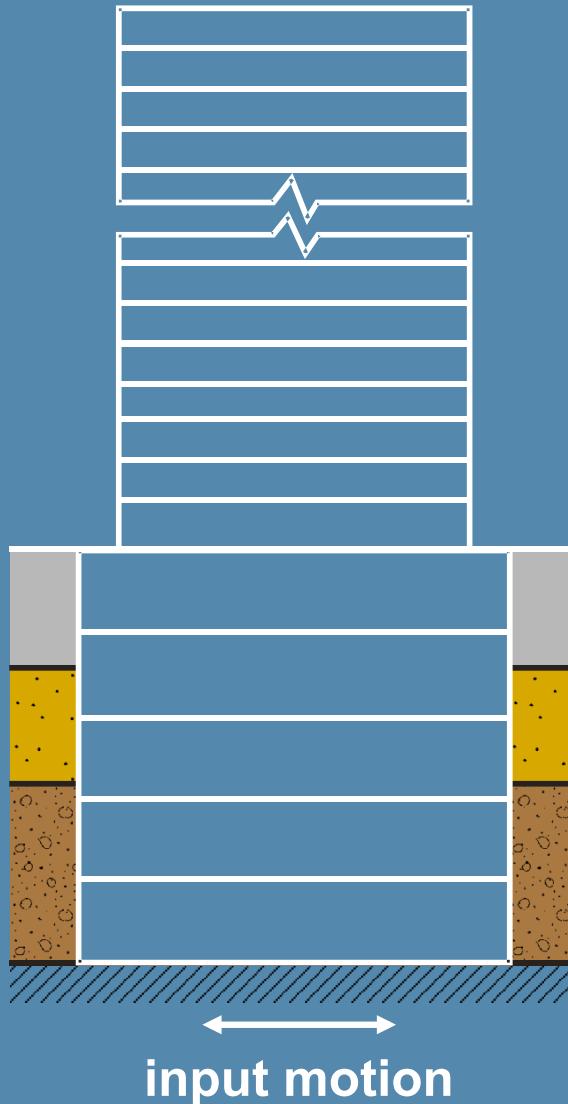
$$V_{s,rock} \gg V_{s,soil}$$

SRA (cont.)

**Site Response
Analysis**
(may not be necessary)



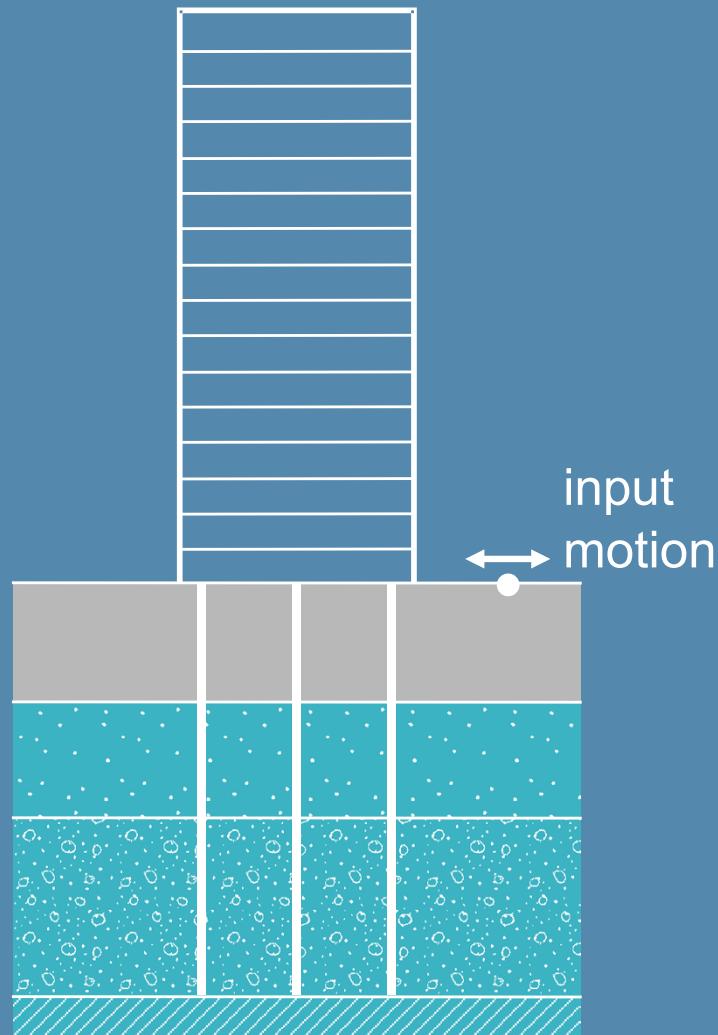
Tall Embedded Building



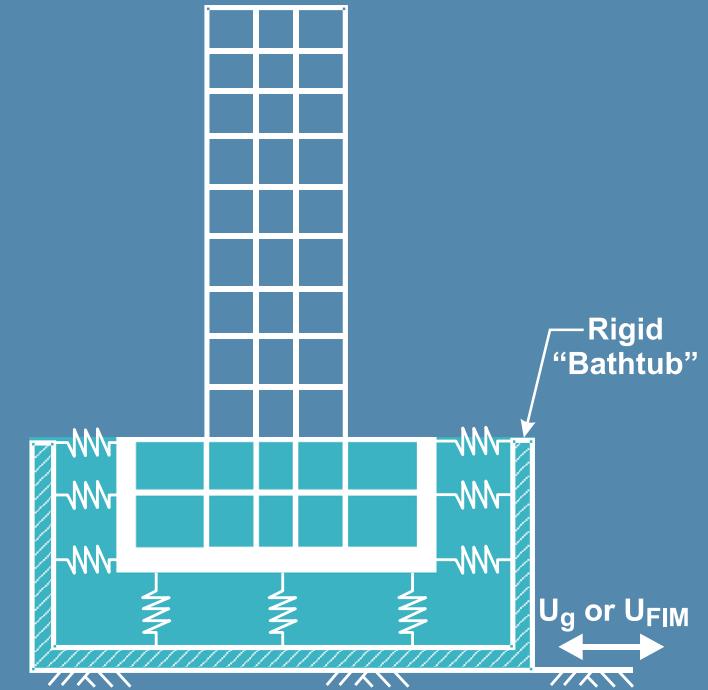
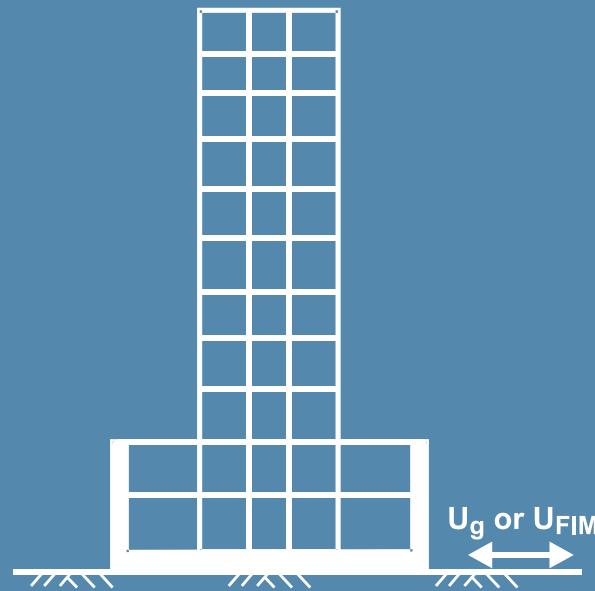
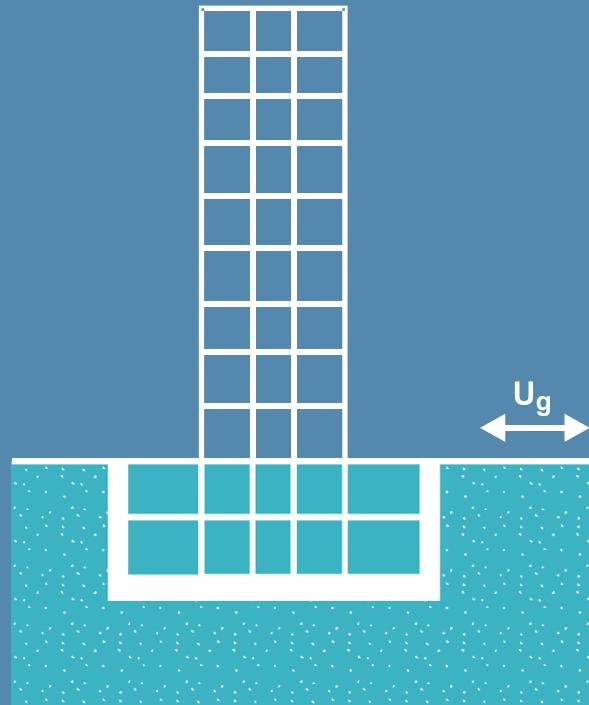
SRA (cont.)

Tall Building on Piles

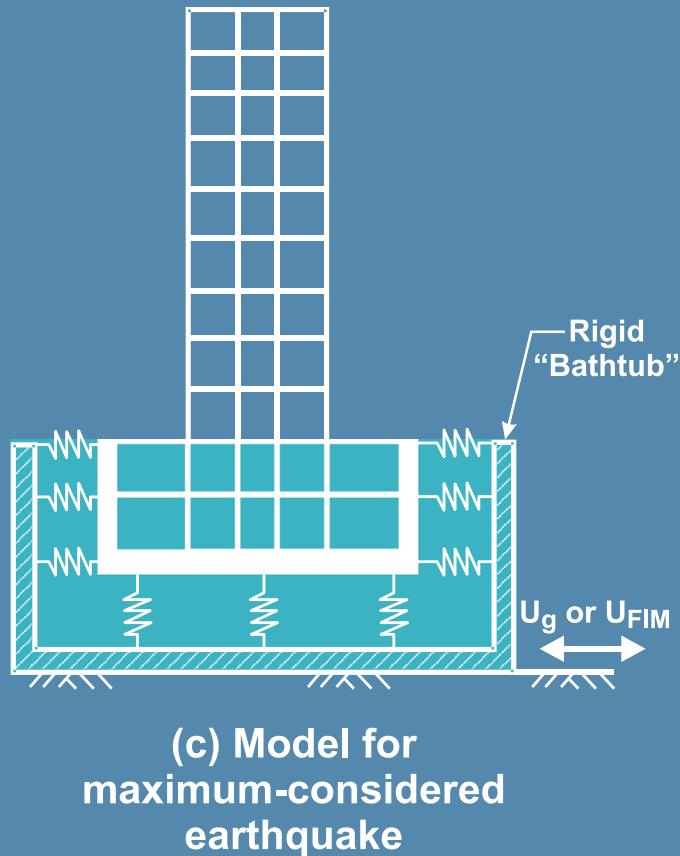
**Site Response
Analysis
(necessary)**



Soil-Foundation-Structure Interaction (SFSI)



SFSI for MCE



- Linear springs and dashpots model soil -foundation interaction
- Input motion same at all points along foundation
- See FEMA 356 and 440, ASCE 41-06, ATC 40, and other references for details

Basement Wall – Soil Interaction

