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Toward Probabilistic Performance-based Seismic Design of Concrete Bridges

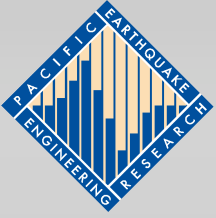


Bozidar Stojadinovic
Kevin Mackie

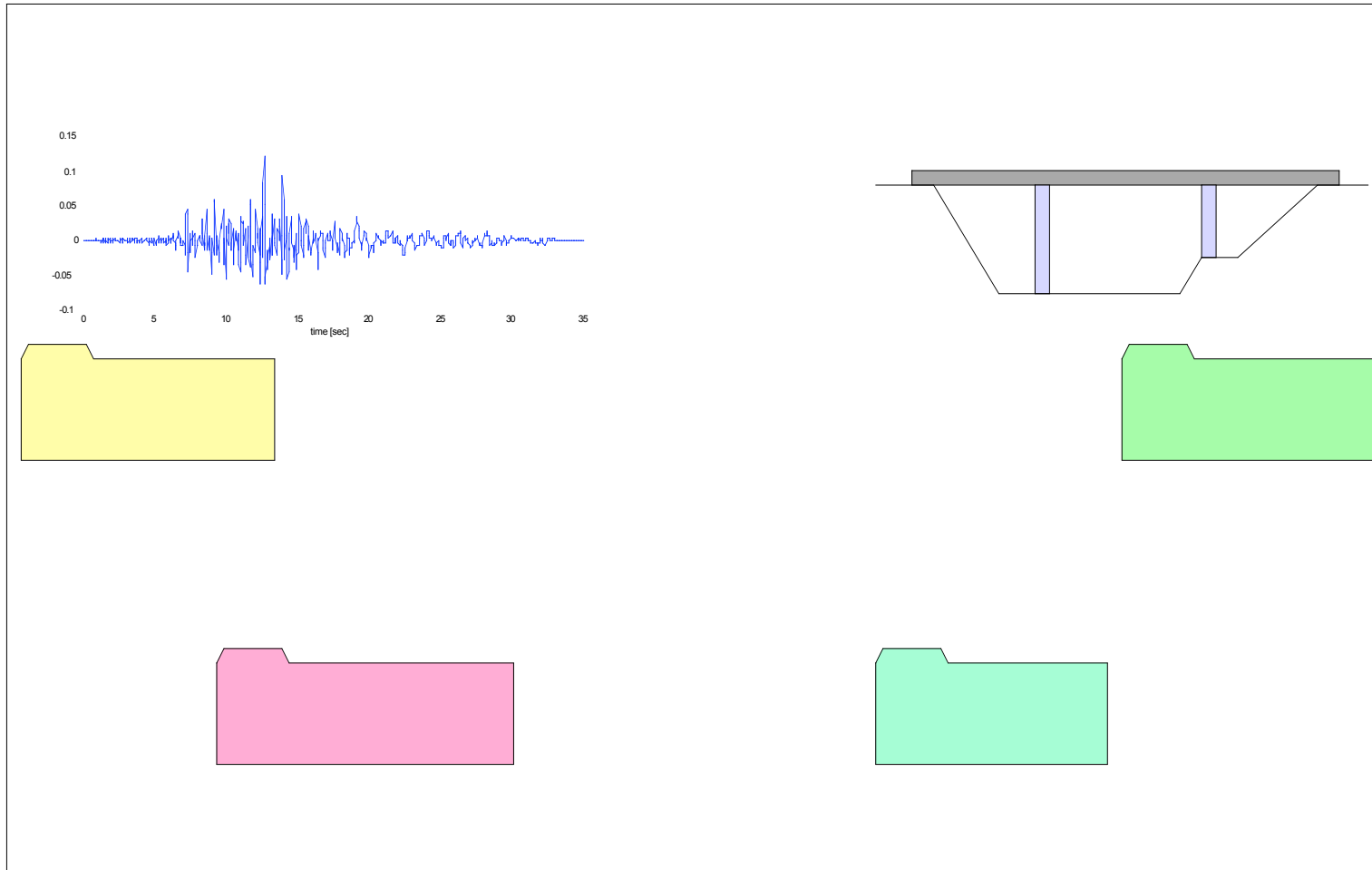
University of California, Berkeley

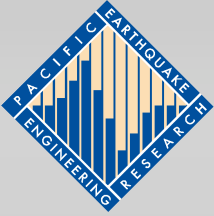
ACI
Phoenix, AZ
October 25-30, 2002





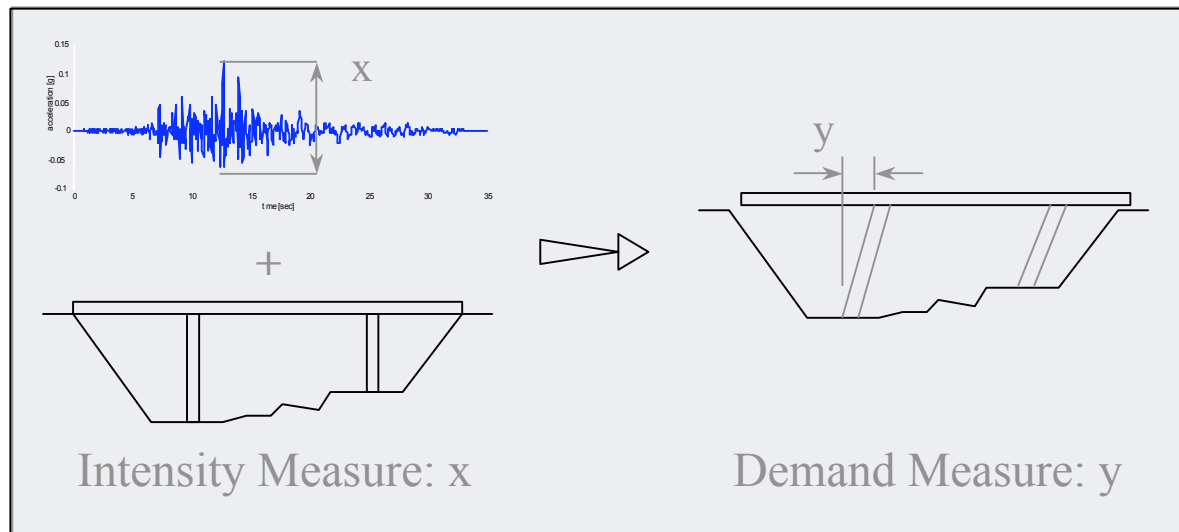
PEER PBD Framework





Why a demand model?

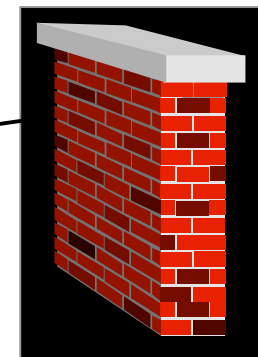
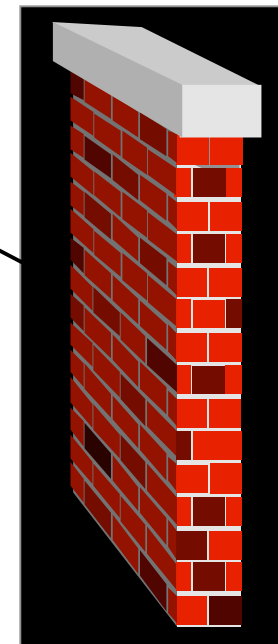
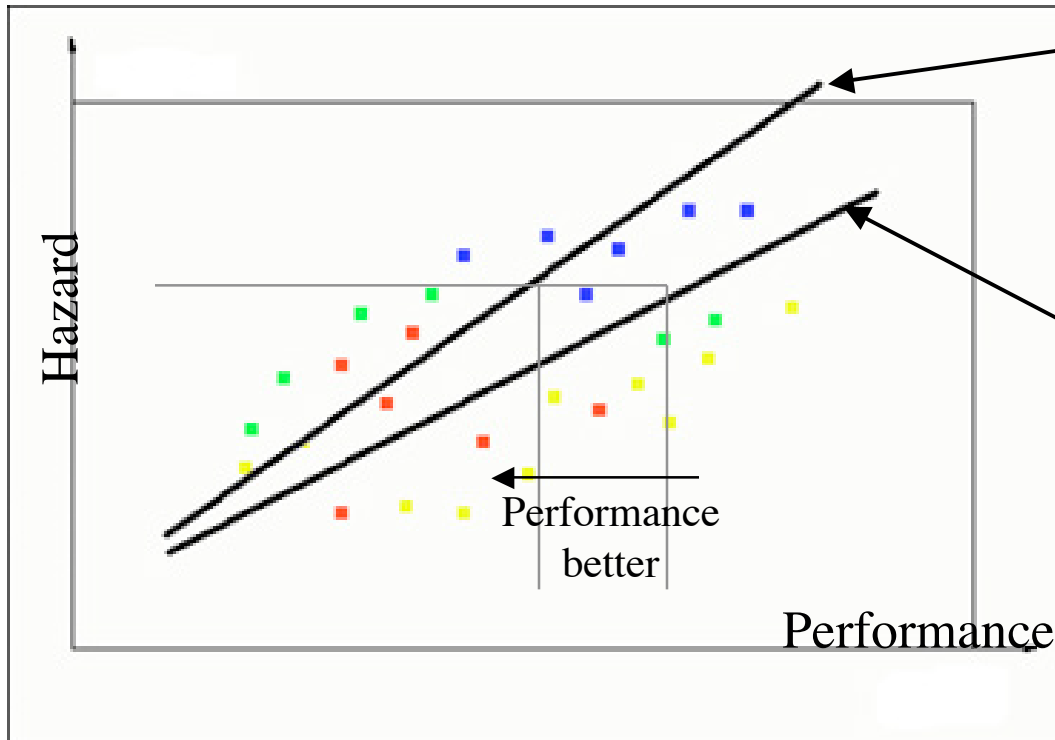
1.) Quantitative Performance Based Earthquake Engineering tool for designers of bridges



What is probability of y , given x ?

Why a demand model?

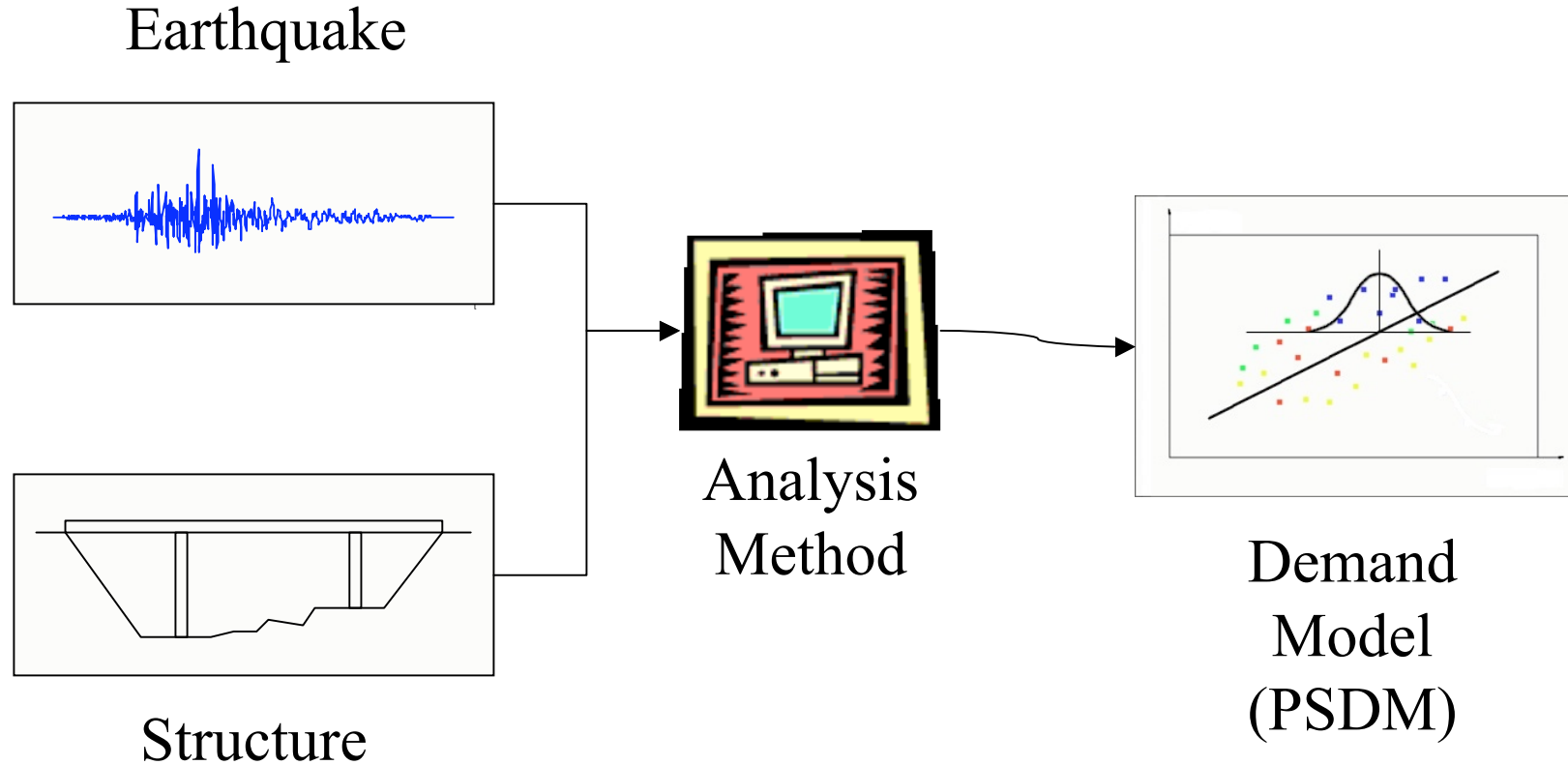
2.) How do design parameters affect performance?





The Big Picture

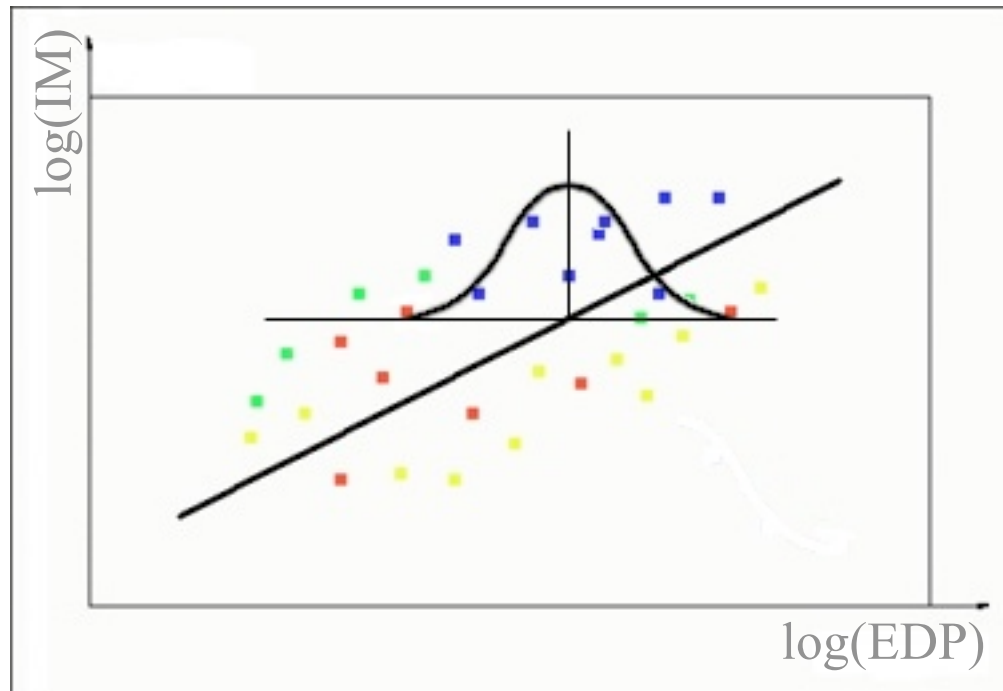
General Probabilistic Seismic Demand Analysis





What is a PSDM?

PSDM = Probabilistic Seismic Demand Model

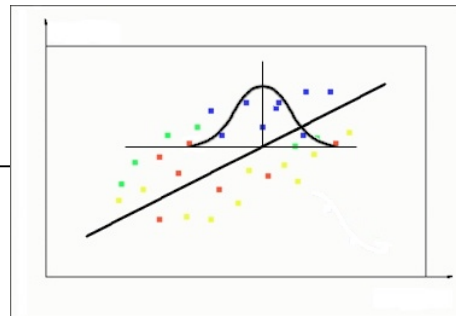


Relationship of seismic Intensity Measures (IM) to structural Engineering Demand Parameters (EDP)

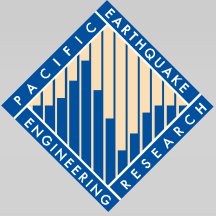


Probabilistic Seismic Demand Analysis

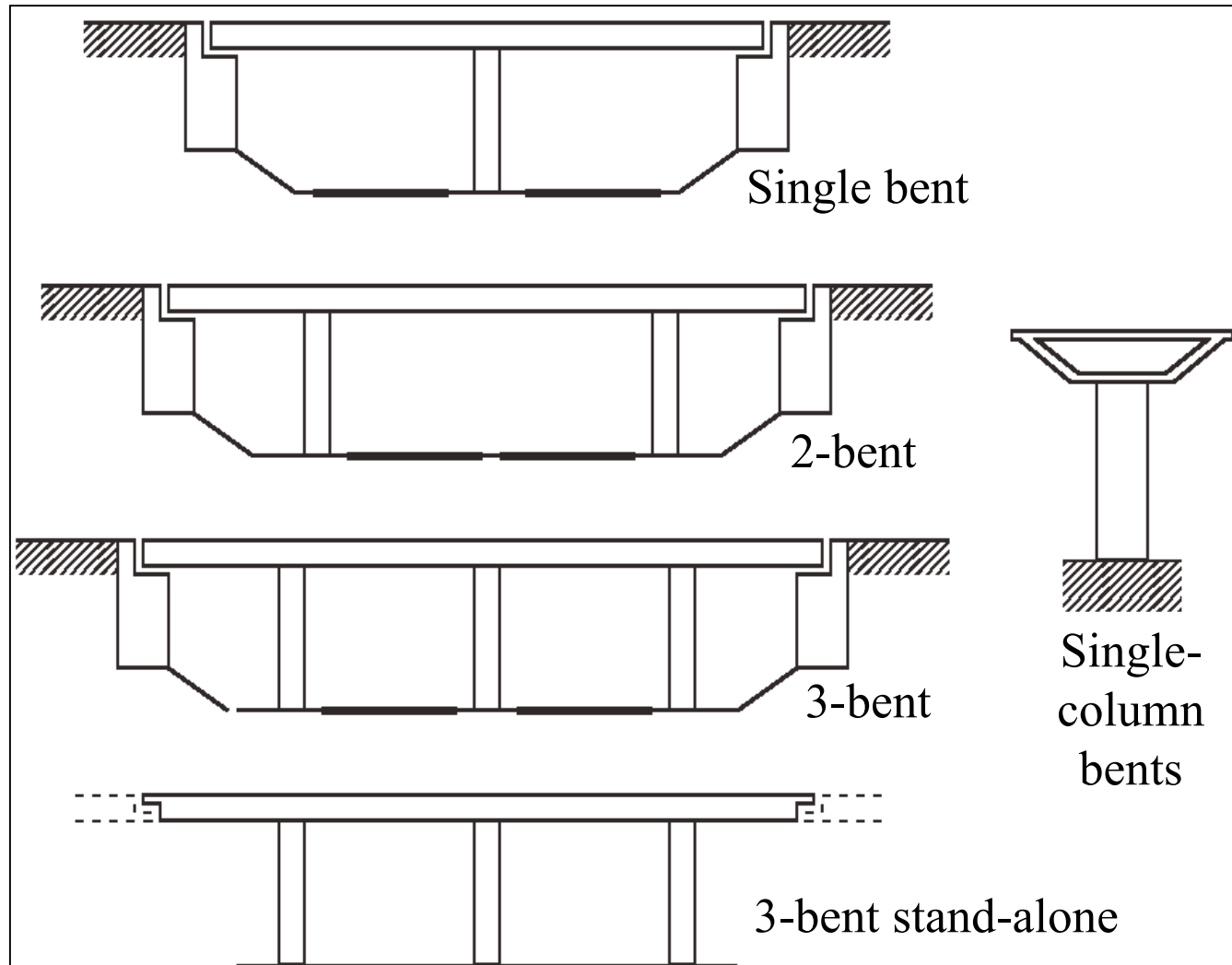
- Define motions (IM)
- Define class of structures (EDP)
- Define analysis model
- Nonlinear analysis



- Design parameter sensitivity
- Ground motion bin sensitivity
- Residual dependence (M, R)



Demand: Bridge Classes

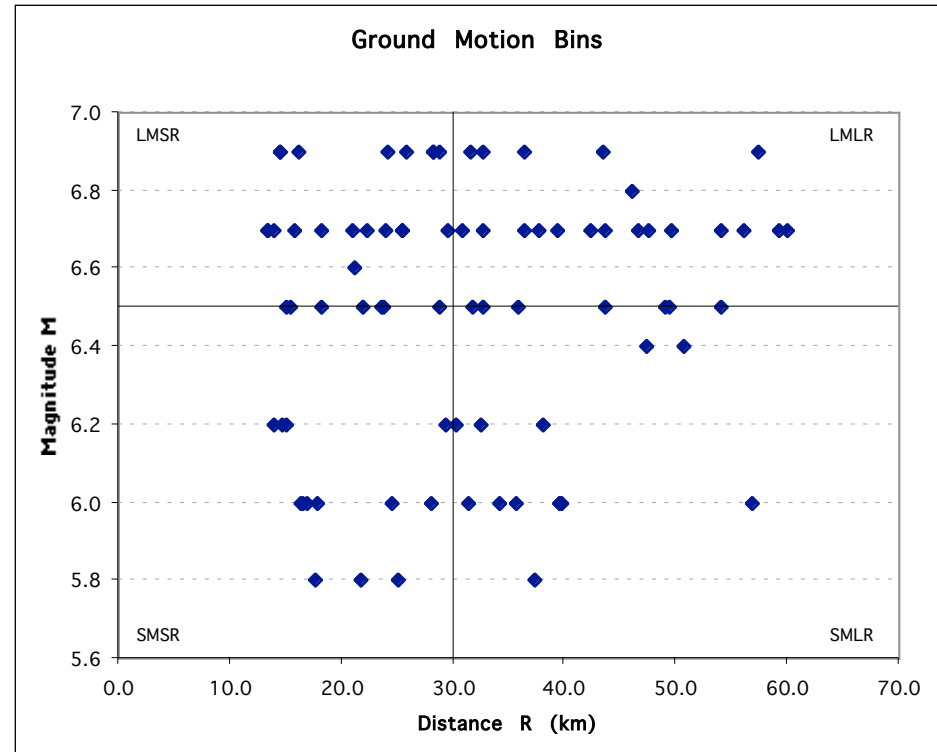


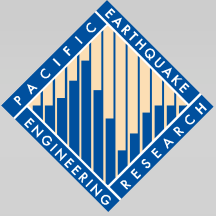


Seismicity: Ground Motions & IMs

- Period Independent Intensity Measures (in this study)
 - Arias intensity
 - PGA, PGV, PGD
- Period Dependent Intensity Measures
(Cordova, 2000)

$$Sa_C = Sa(T_1) \sqrt{\frac{Sa(2T_1)}{Sa(T_1)}}$$



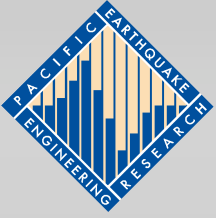


Demand: EDPs

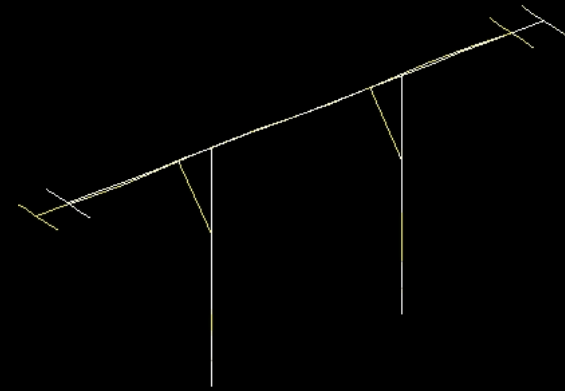
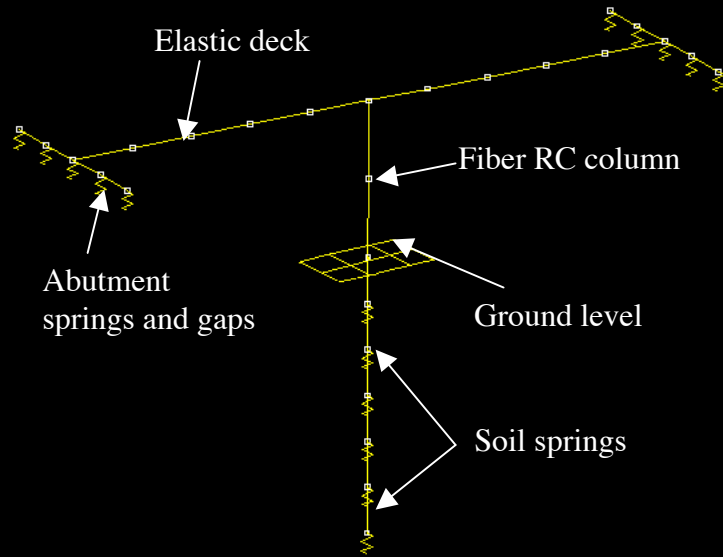
- Local EDPs
 - Steel stress & strain
 - Concrete stress & strain
- Intermediate EDPs
 - Column curvature ductility
 - Maximum column moment
 - Plastic rotation
 - Hysteretic energy
- Global EDPs
 - Displacement ductility
 - Drift ratio
 - Residual displacement index

Single column/bent
highway overpasses in
California

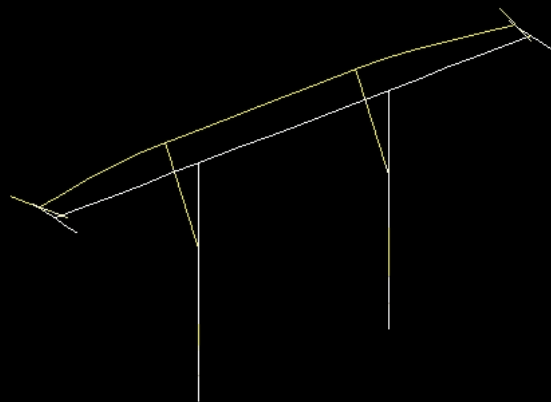




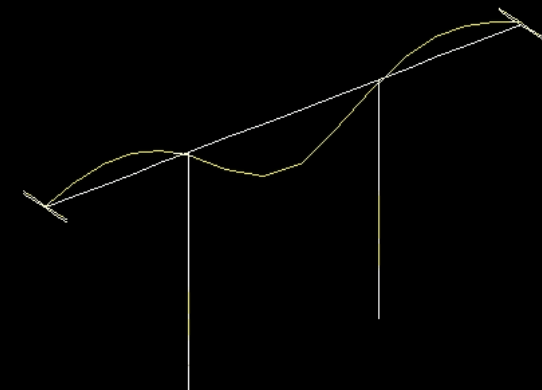
OpenSees Bridge Model



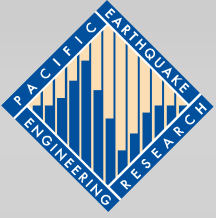
Mode 1 - Longitudinal



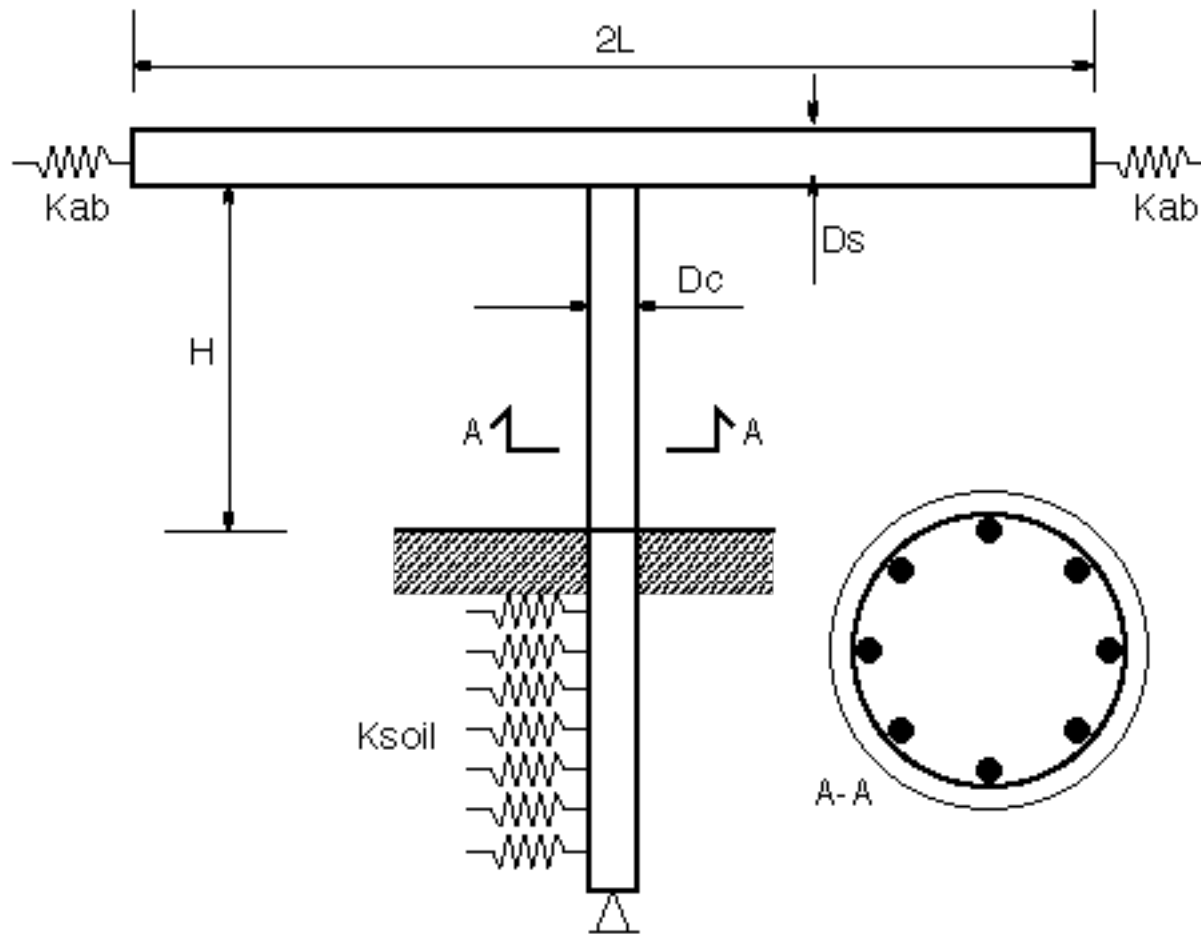
Mode 2 - Transverse



Mode 3 - Vertical



Bridge Design Parameters



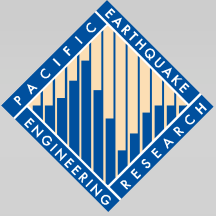
Single-column, single-bent bridges



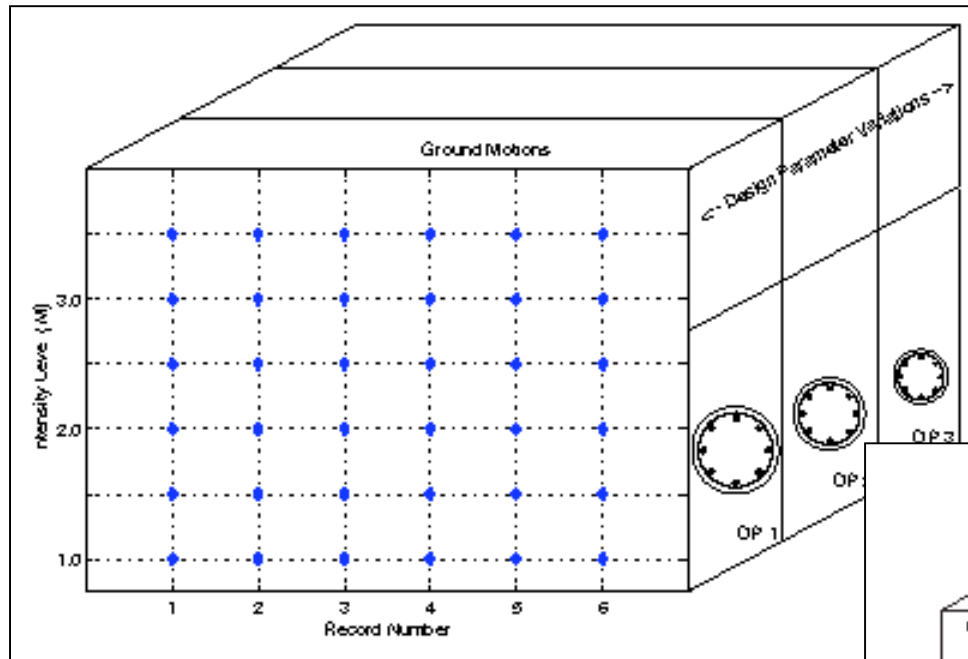
Bridge Design Parameters

Range of design parameter variations

- | | | |
|----------------------------|-------------------------------------|-----------|
| • L | span length | 60-180 ft |
| • L/H | span to column height ratio | 1.2-3.5 |
| • $\rho_{s, \text{long}}$ | column longitudinal reinforcement | 1-4% |
| • D_c/D_s | column to superstructure dimensions | 0.67-1.33 |
| • $\rho_{s, \text{trans}}$ | column transverse reinforcement | 0.4-1.2% |
| • W_t | additional superstructure weight | 10-150% |
| • K_{soil} | soil stiffness | USGS A-D |
| • A_{abut} | abutment mass/stiffness models | various |

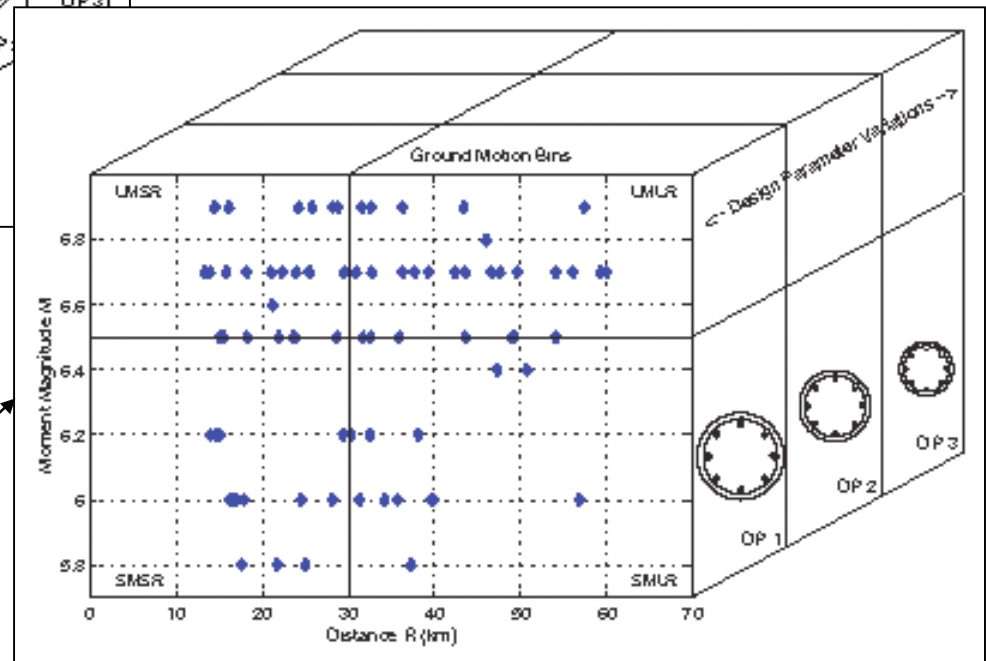


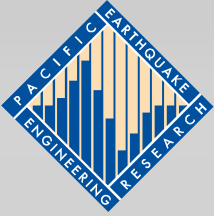
Analysis Method



Incremental
Dynamic Analysis
(IDA)

Probabilistic Seismic
Demand Analysis
(PSDA)





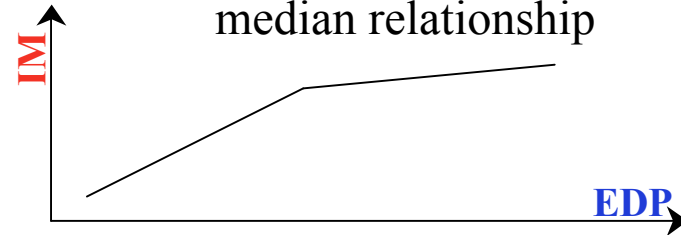
Demand Model Criteria

Practical

- Definition: Makes engineering sense
 - Practical: column drift
 - Not practical: steel stress

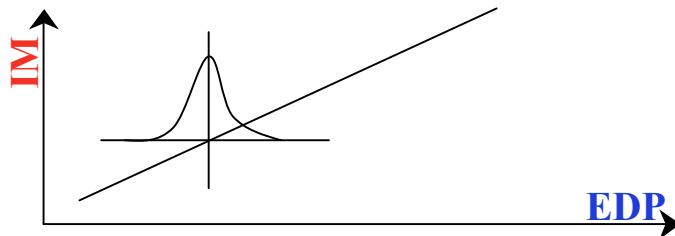
Effective

- Definition: linear or mult-linear median relationship



Efficient

- Definition: low variability



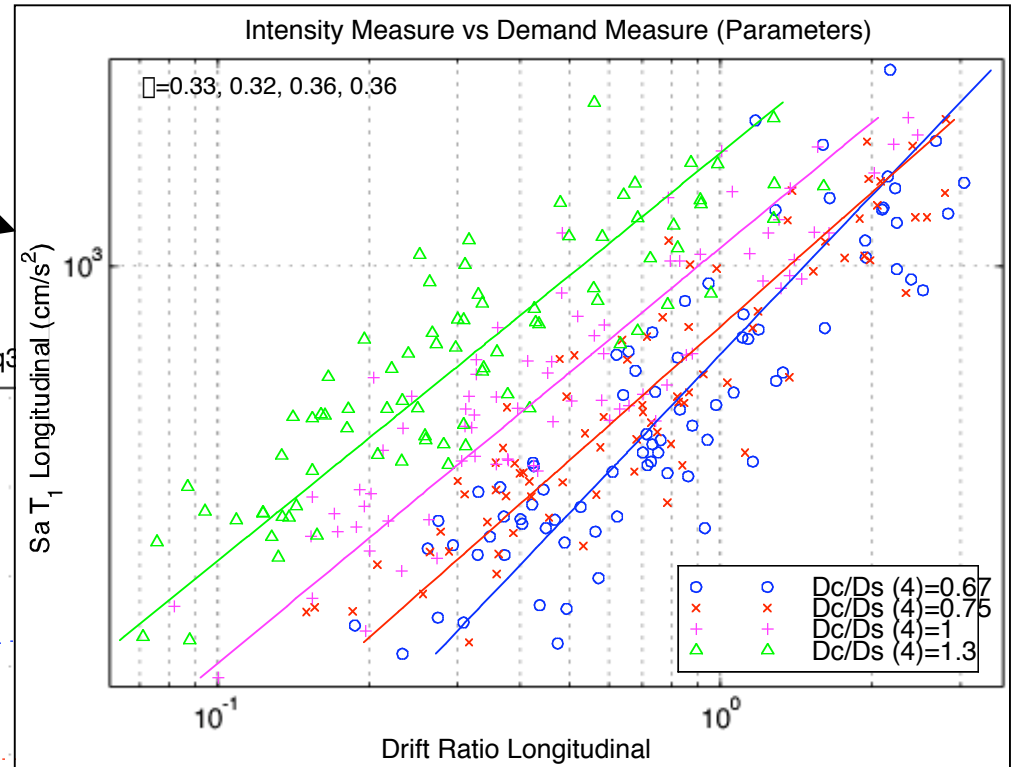
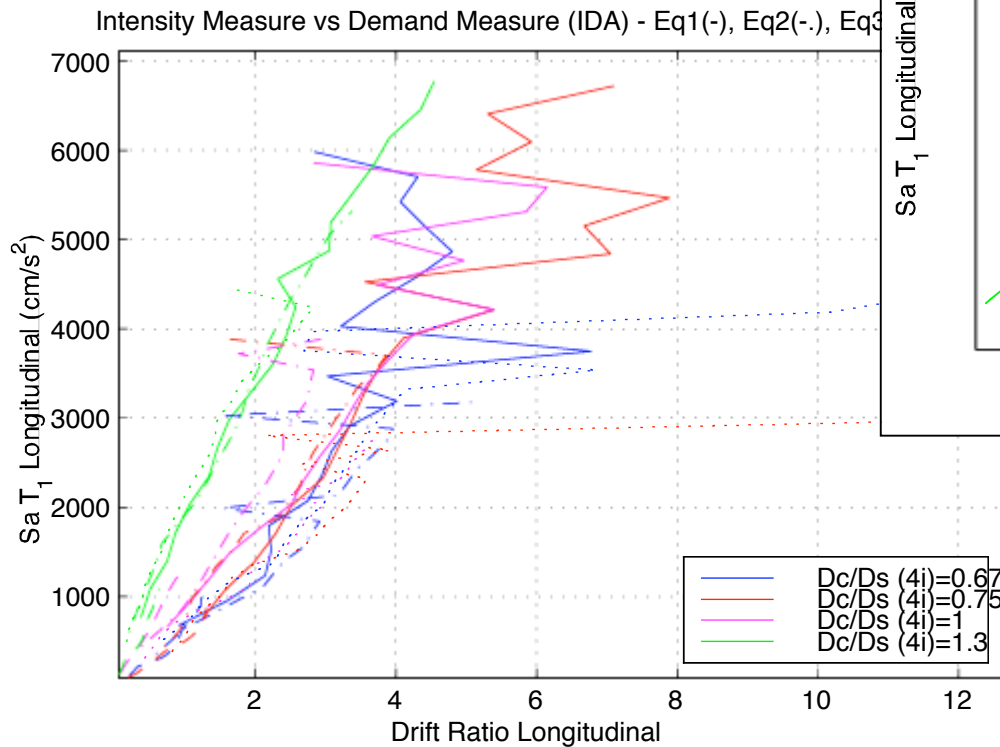
Sufficient

- Definition: No conditional dependence on M, R
- $P[EDP|IM, M, R] = P[EDP|IM]$
- Can also be applied to strong motion duration



Resulting Demand Models

PSDA

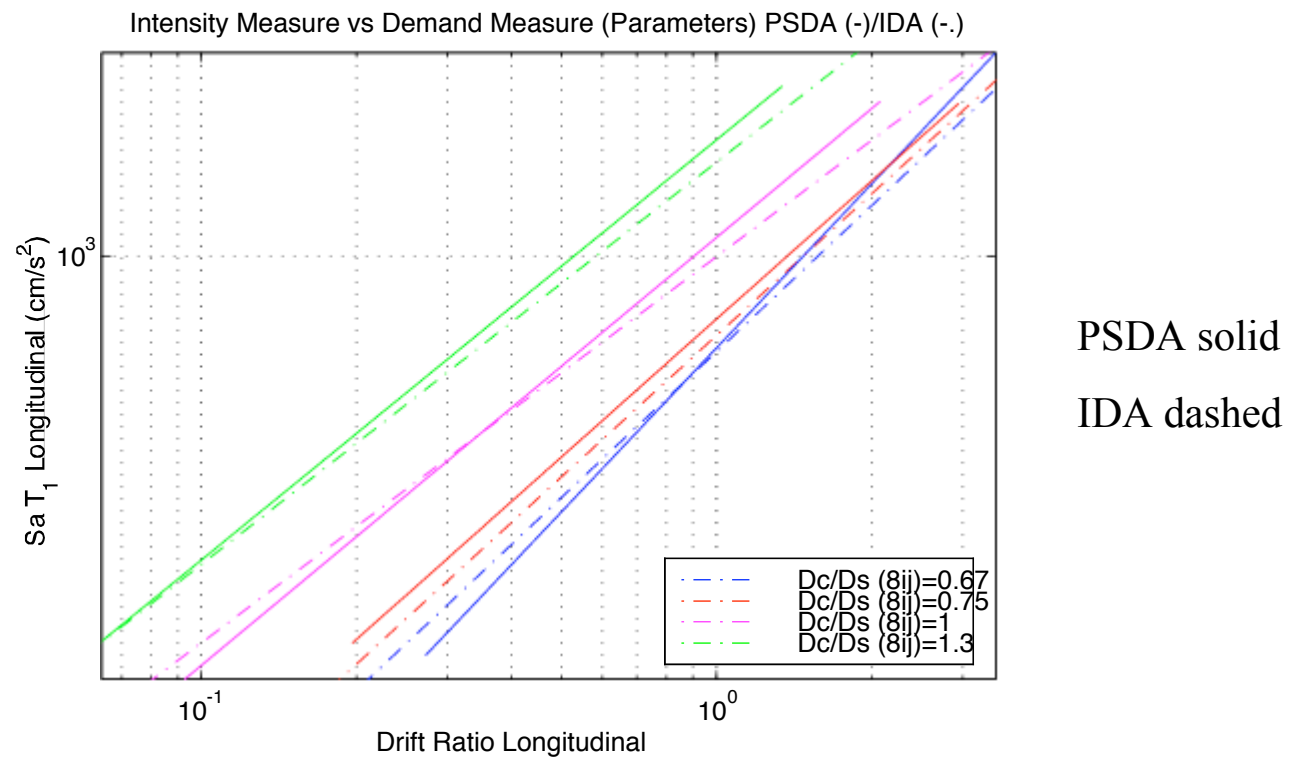


IDA



IDA-PSDA Median Comparison

Sa_{T_1} -Drift ratio (linear), 8x10 IDA matrix



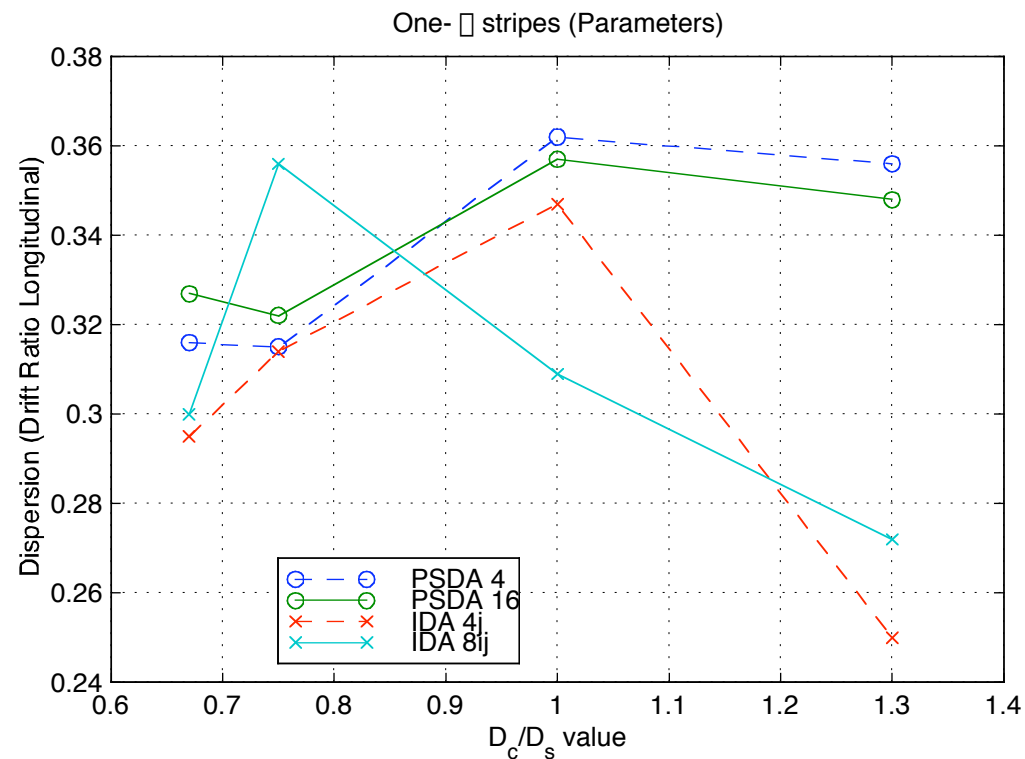
Superstructure depth to column diameter ratio (Dc/Ds)

1-bent bridge with abutments



IDA-PSDA Dispersion

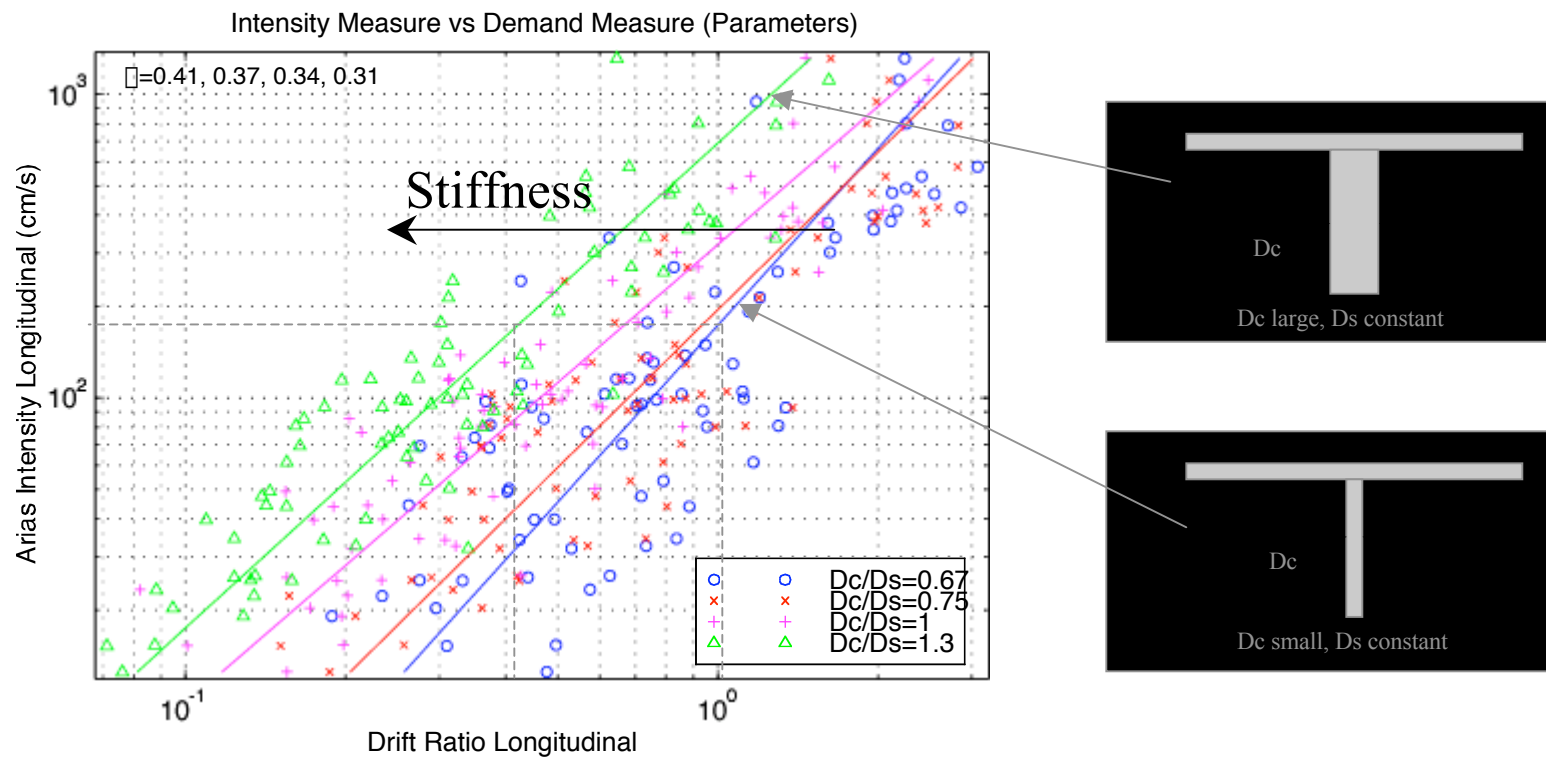
Sa-Drift ratio, 8x10 IDA matrix



- IDA 4x20 under-predicts dispersion
- IDA 8x10 approaches real dispersion

Design Parameter Sensitivity

Superstructure depth to column diameter ratio (D_c/D_s)



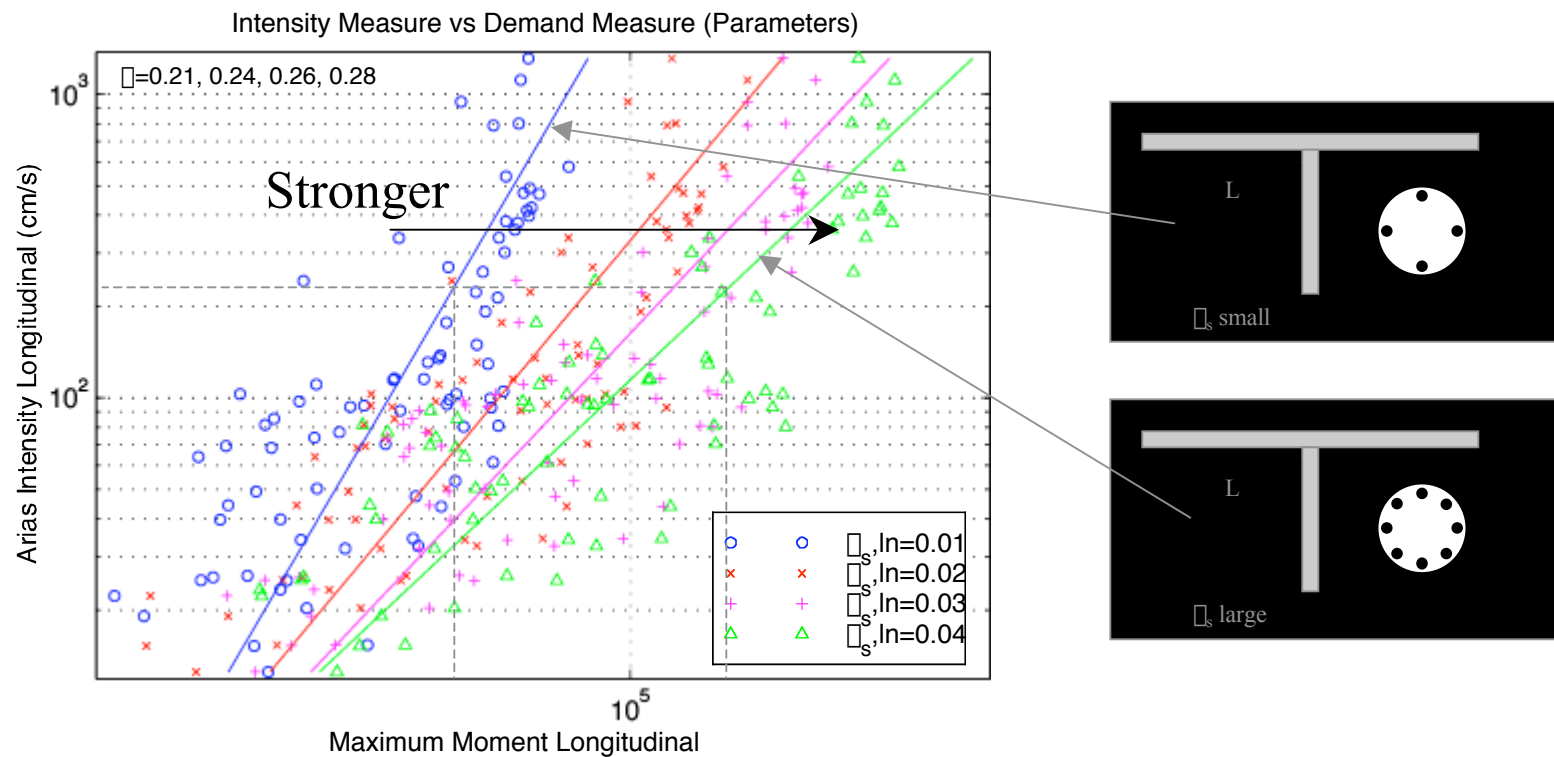
Decreasing column height increases drift (decreases max. displ.)

1-bent bridge with abutments



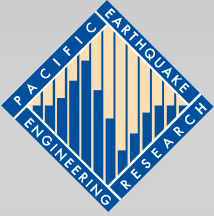
Design Parameter Sensitivity

Column longitudinal steel ratio (ρ_s)



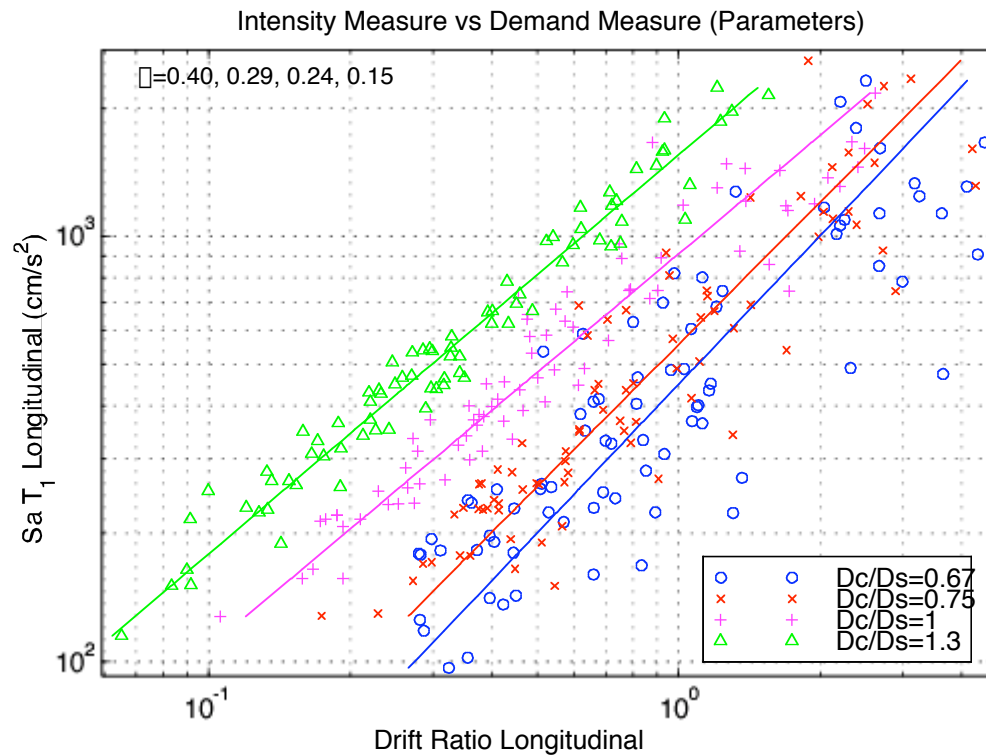
Increasing steel increases column moment demand

1-bent bridge with abutments

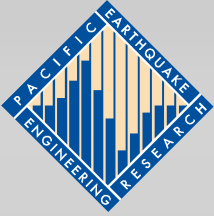


Sensitivity with Optimal IMs

SaT1-Drift sensitivity to Dc/Ds design parameter

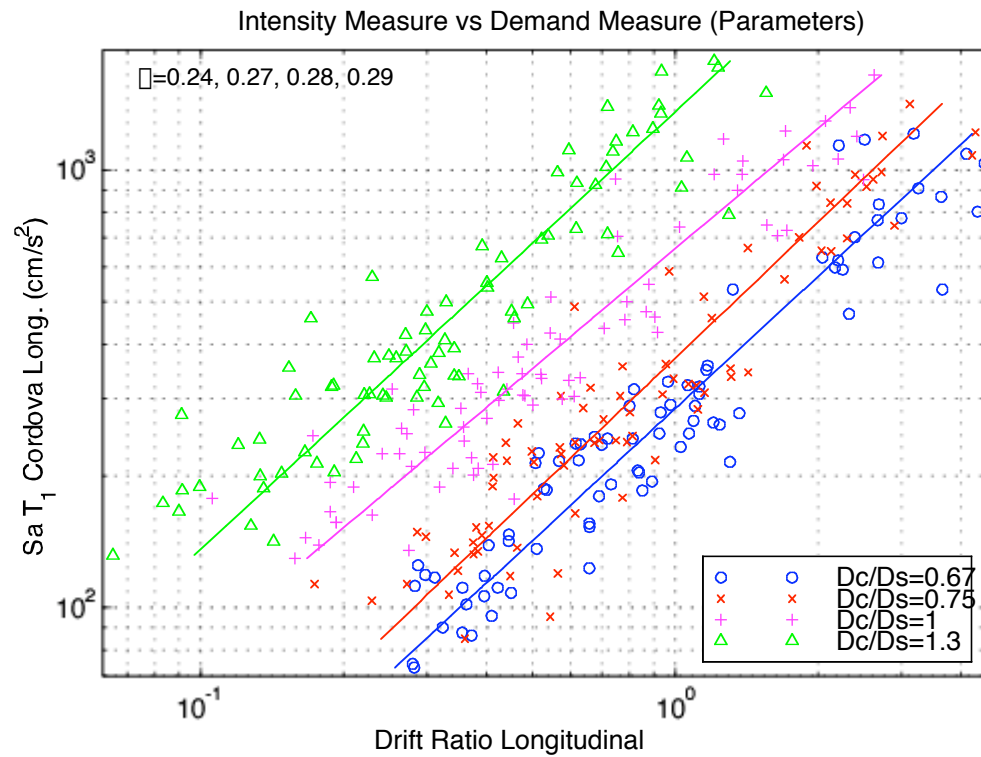


Arias	0.41	0.37	0.34	0.31
Sa(T1)	0.40	0.29	0.24	0.15

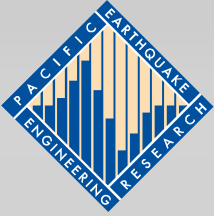


Sensitivity with Optimal IMs

Sa(Cordova)-Drift sensitivity to Dc/Ds design parameter

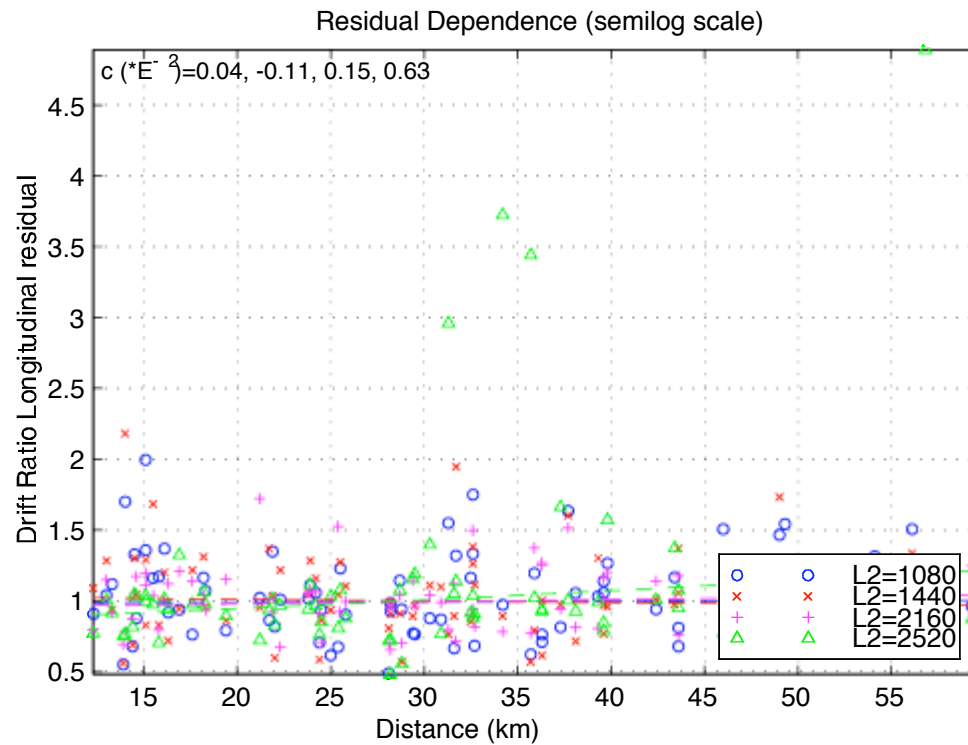


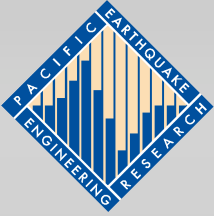
Arias	0.41	0.37	0.34	0.31
Sa(T1)	0.40	0.29	0.24	0.15
SaC	0.24	0.27	0.28	0.29



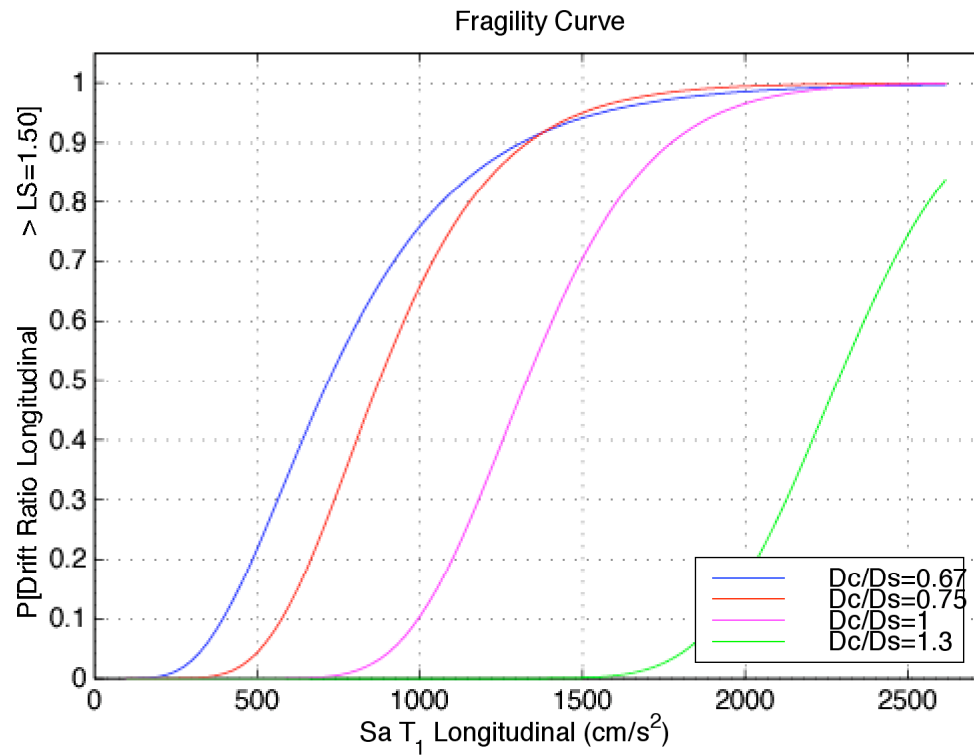
Sufficiency

Distance (R) dependence



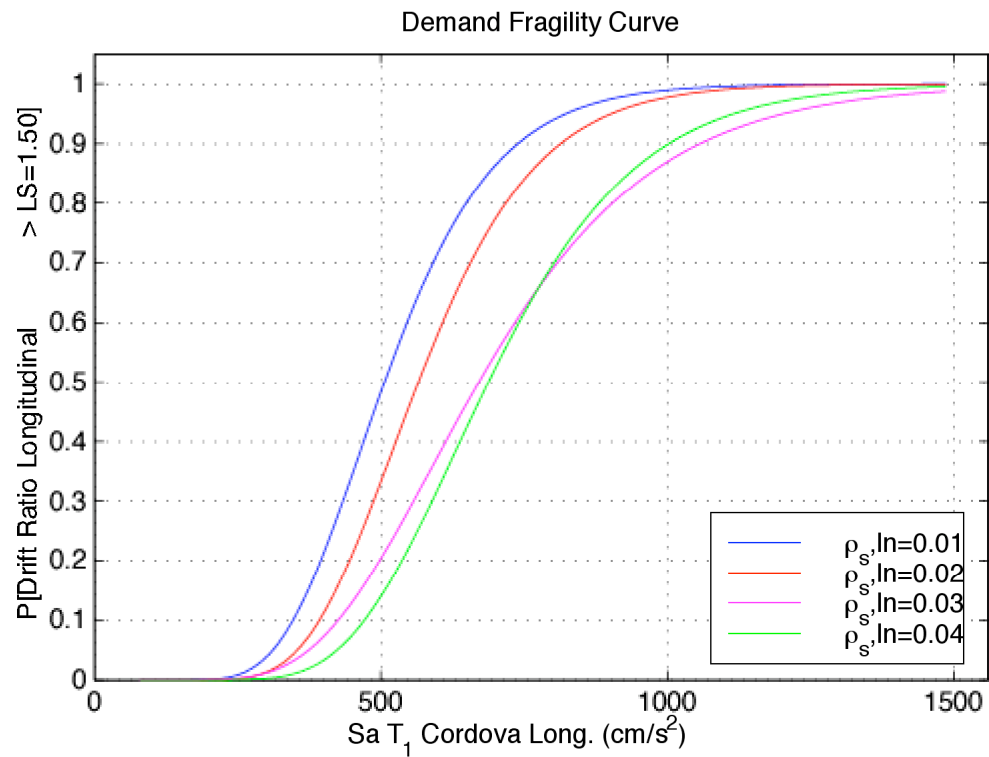


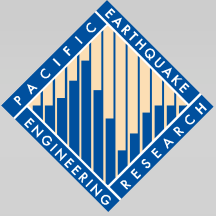
Demand fragility





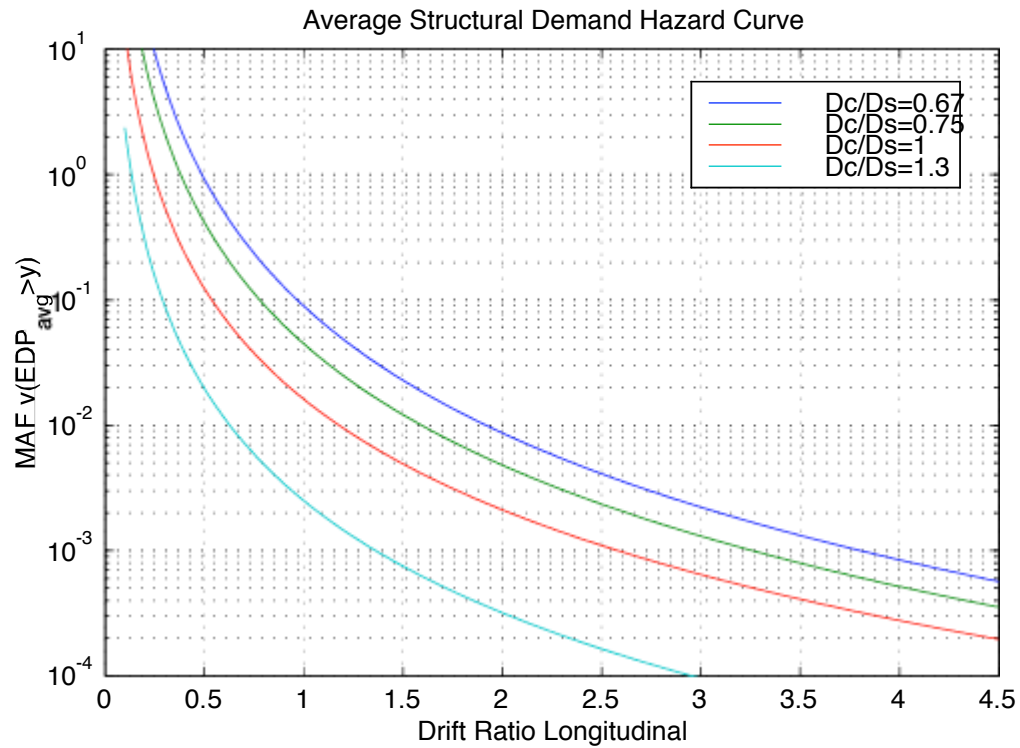
Demand Fragility

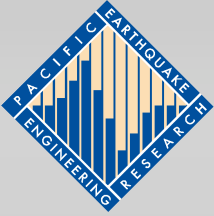




PSDM Extensions

Structural Demand Hazard Curves

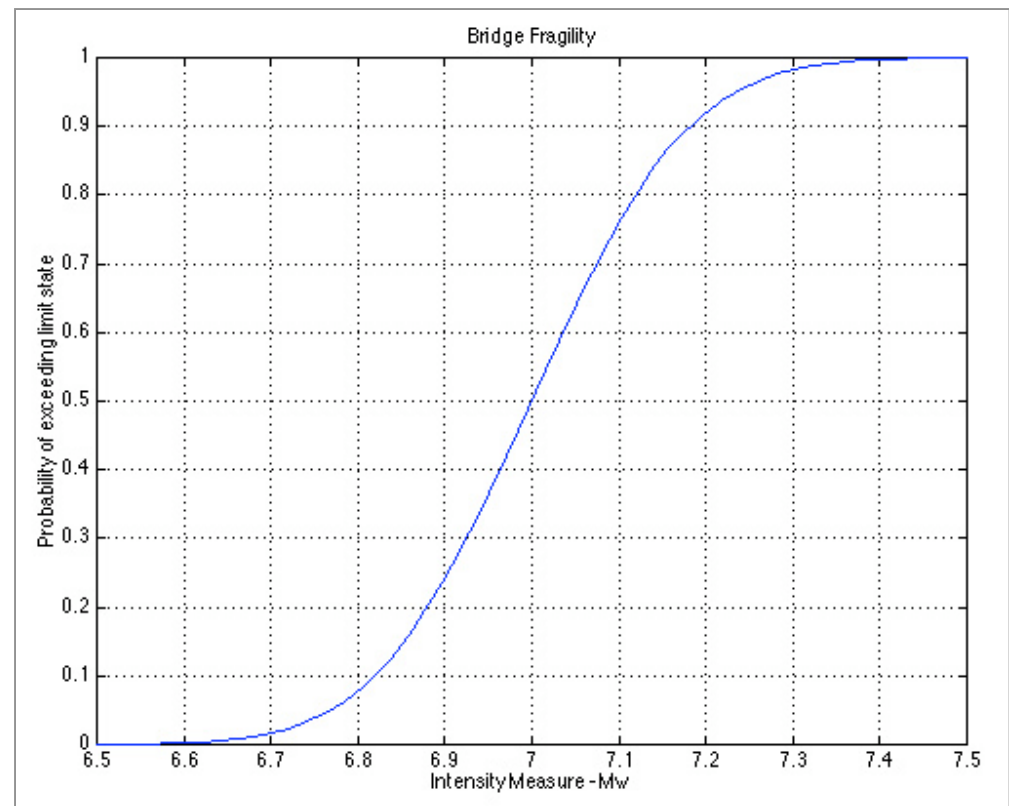




PSDM Extensions

Functional EDPs

- Maximum post-earthquake functionality (traffic load, eg)
- Fragility functions: Probability of exceeding a given damage state





Conclusions

- PEER PBD framework is completely general with a probabilistic basis
- Demand models can be represented by:
 - PSDA
 - IDA
 - Demand fragility
- Criteria for acceptance of a demand model
 - Practical, efficient, effective, sufficient
- Damage fragility easily incorporated using capacity models



Thank You!

- Questions?
- For more information contact:
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- Visit <http://millerbird.ce.berkeley.edu>



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