

Seismic Demands for Performance-Based Design of Bridges

Project #: 3122000

Kevin Mackie

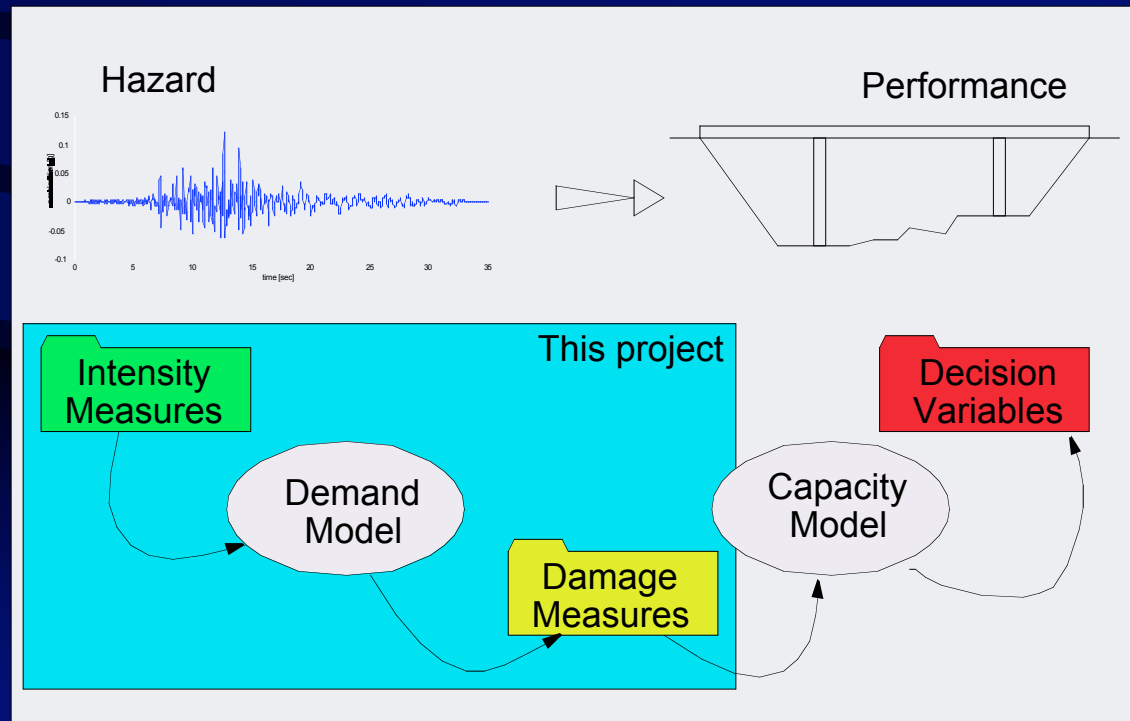
Bozidar Stojadinovic

University of California, Berkeley
Department of Civil and Environmental Engineering

PEER Annual Meeting
January 25-26th, 2001

Project Objective

Develop a probabilistic demand model for typical new bridges in California



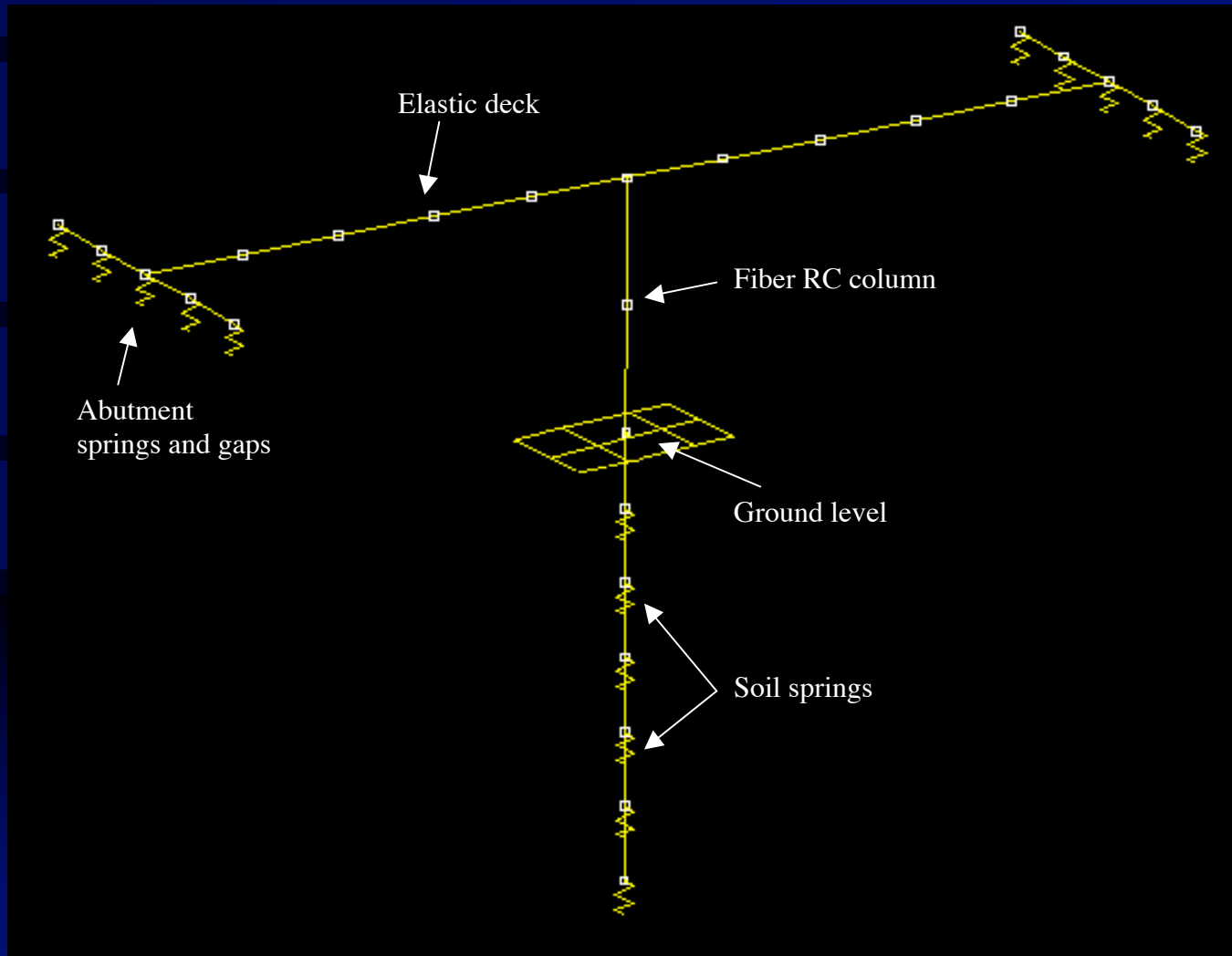
Seismicity: Intensity Measures

- Structure Independent Measures
 - Magnitude
 - Distance
 - Arias intensity (acceleration & velocity)
 - Cumulative absolute velocity
 - Cumulative absolute displacement
 - Frequency ratios
 - Strong motion duration
 - RMS acceleration
 - Characteristic intensity
 - PGA, PGV, PGD
- Structure Dependent Measures
 - S_a , S_v , S_d
 - $S_{d,inelastic}$

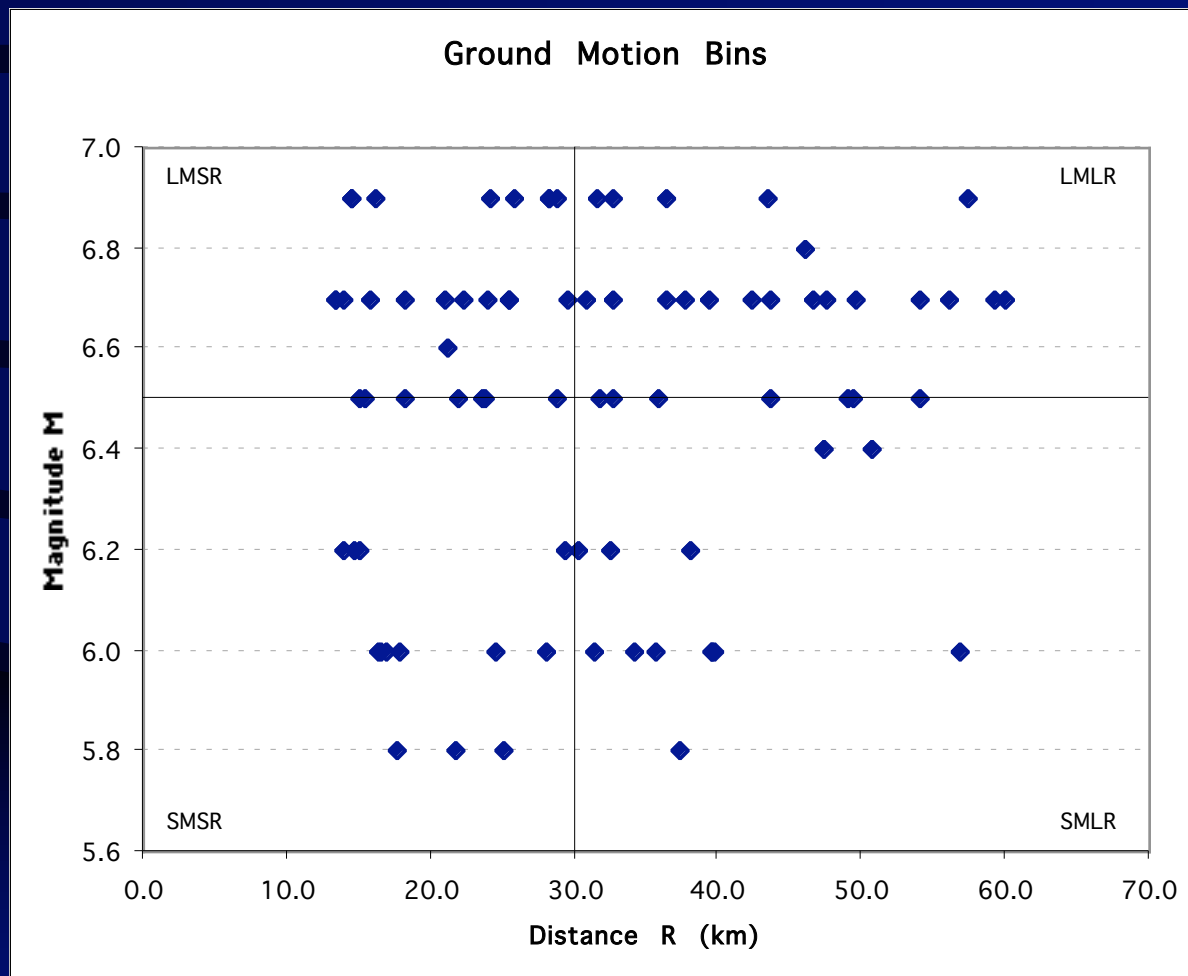
Demand: Damage Measures

- Steel strain ϵ_s
- Concrete strain ϵ_c
- Curvature ductility
- Displacement ductility
- Drift ratio
- Residual displacement index
- Plastic rotation
- Hysteretic energy
- Normalized hysteretic energy
- Maximum column curvature

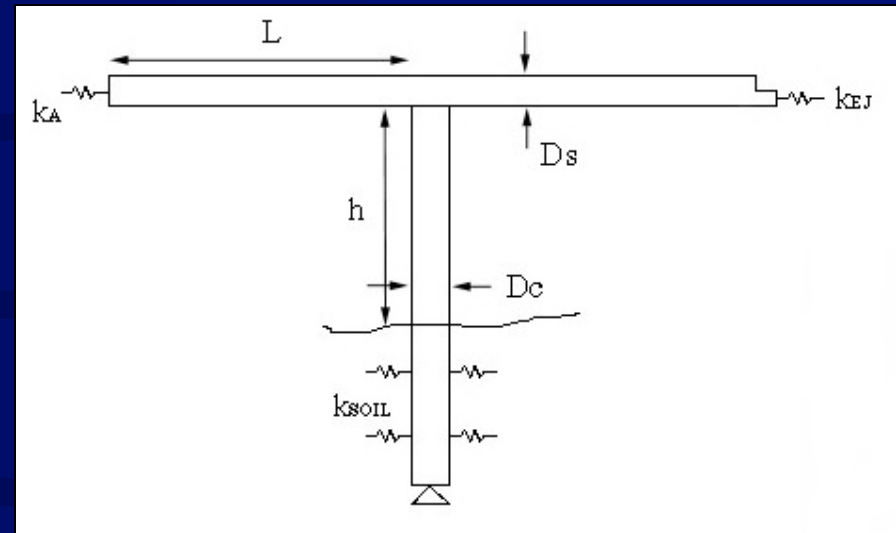
OpenSees Bridge Model



Ground Motion Portfolio

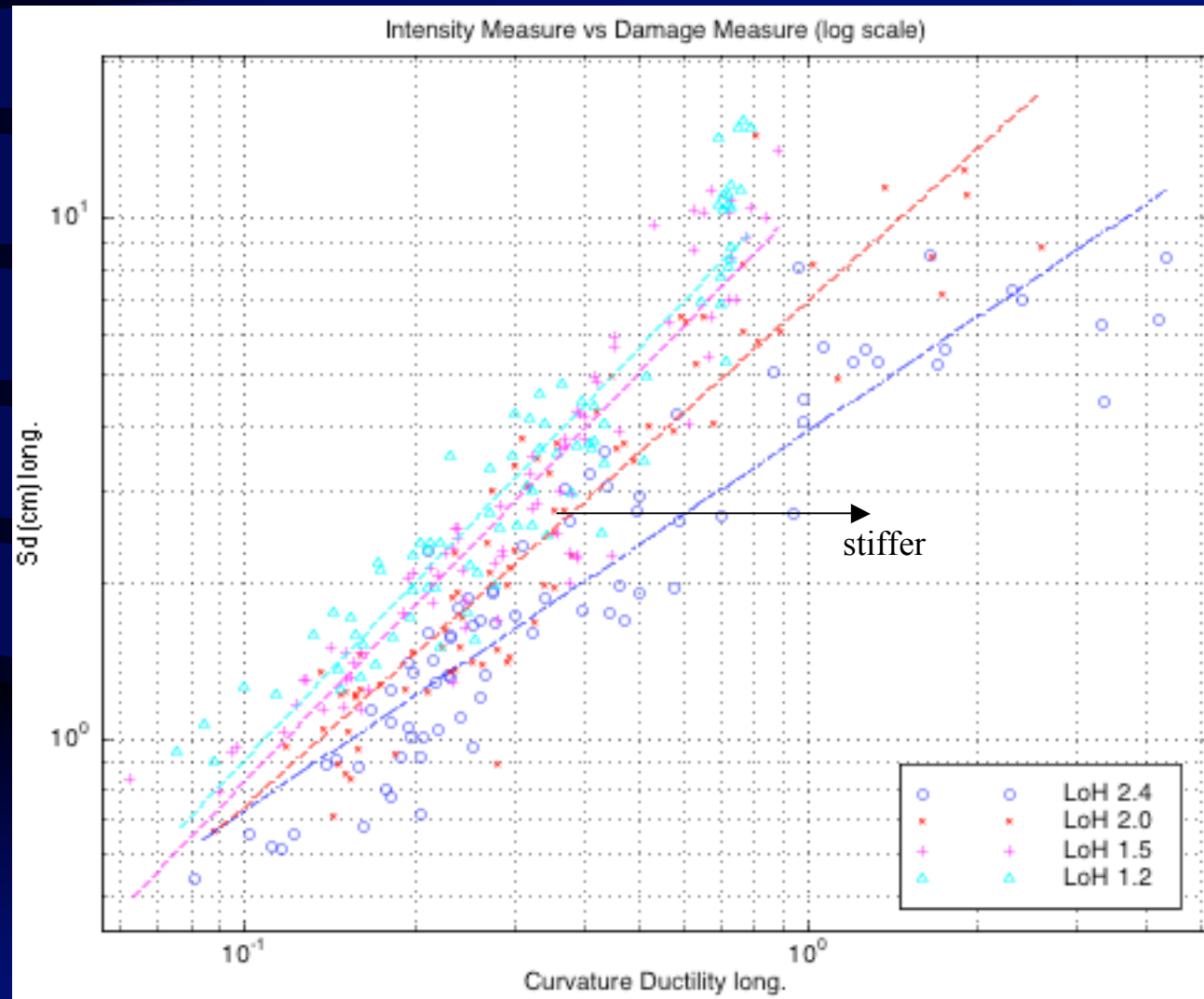


Bridge Portfolio



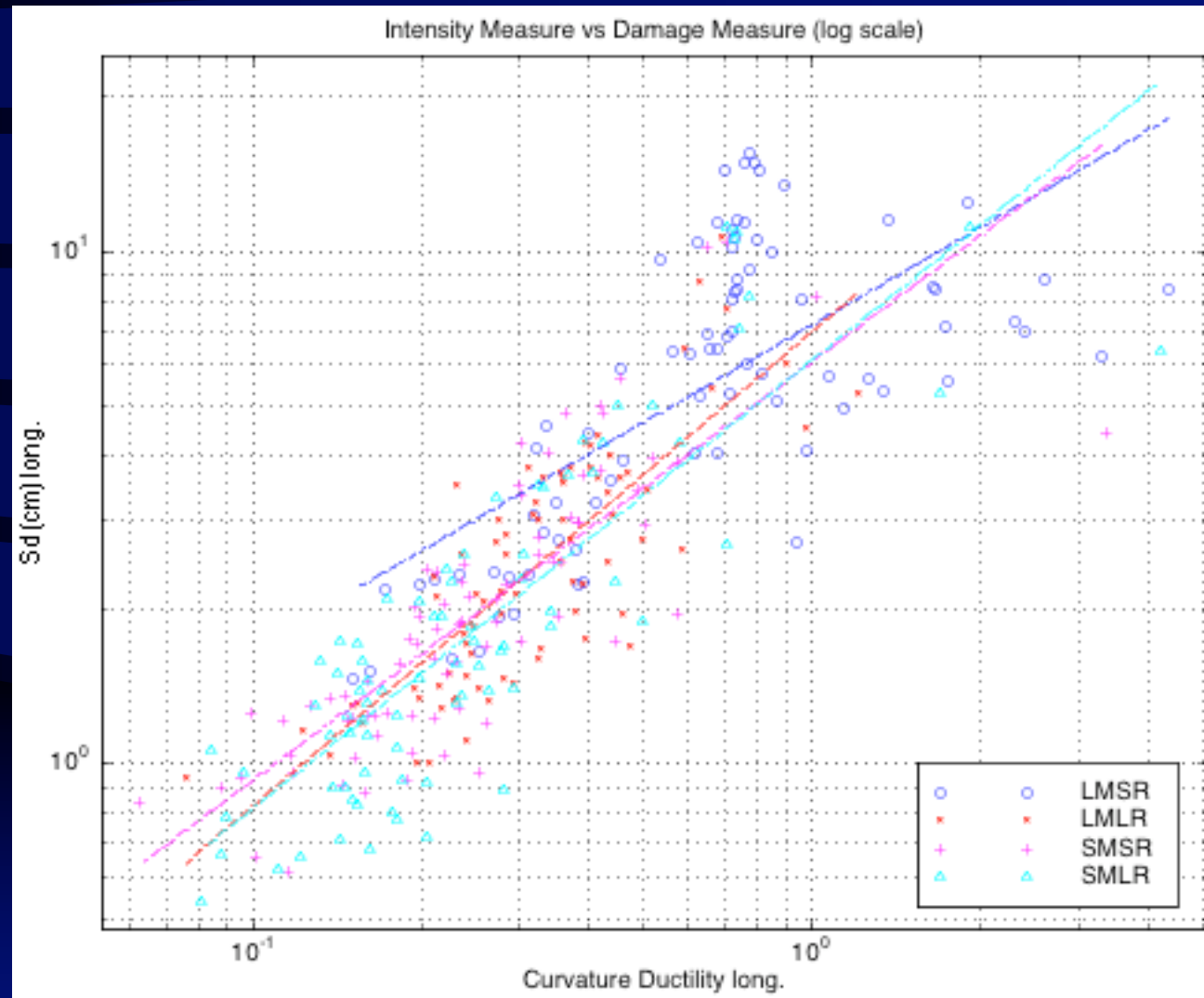
- Skew degree of skewness 0-50°
- L span length 60-180 ft
- L/h span to column height ratio 1.2-2.0
- f_y steel strength 68-95 ksi
- f'_c concrete strength 3-8 ksi
- $\rho_{s, long}$ column longitudinal reinforcement 1-4%
- D_c/D_s column to superstructure dimensions 0.67-1.33
- K_{soil} NEHRP soil group B,C,D
- W_t additional superstructure weight 0.1-50%
- $\rho_{s, trans}$ column transverse reinforcement 0.4-1.0%

Design Parameters - S_d vs μ_{θ}



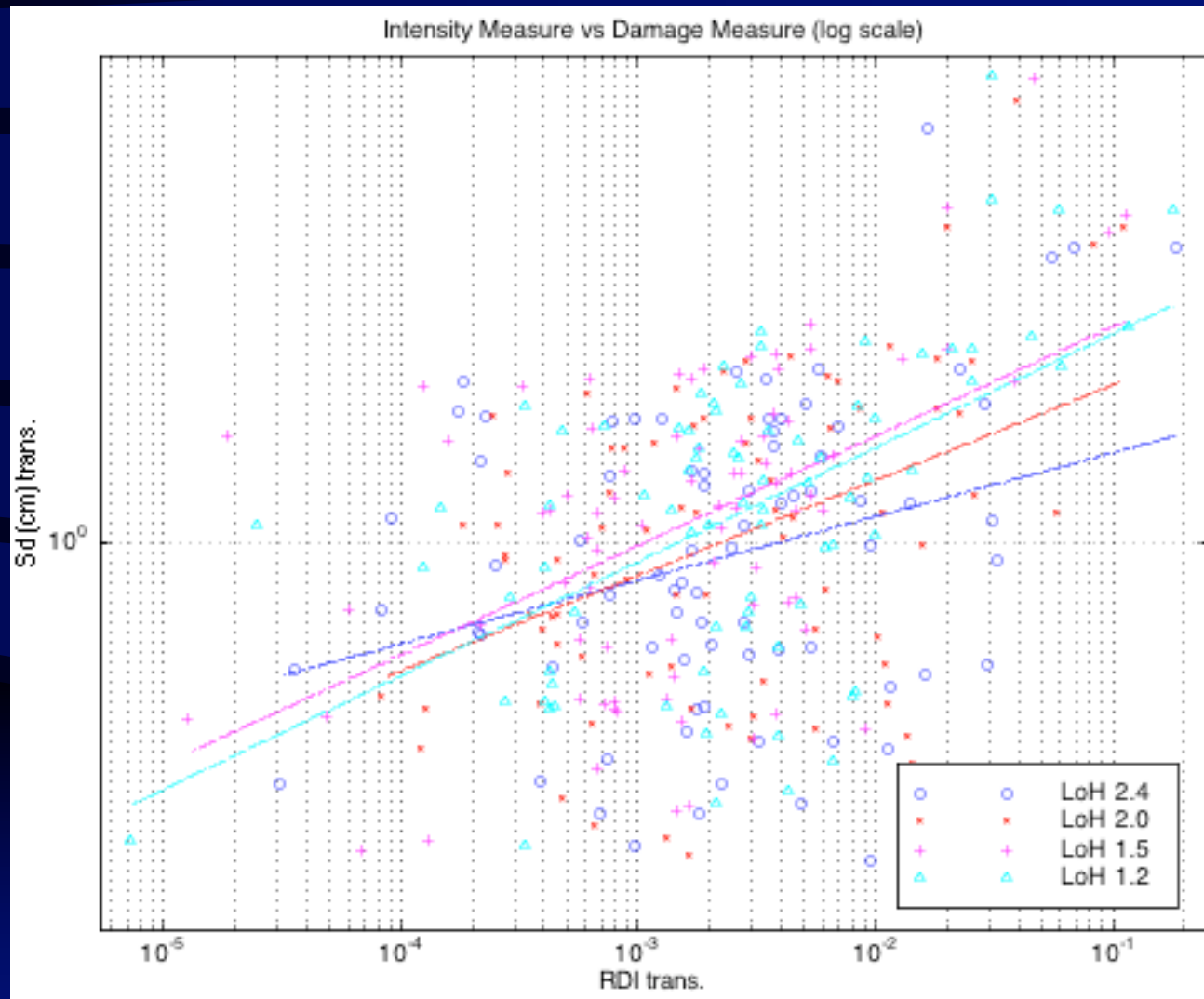
Increasing strength has negligible effect on performance.

Seismicity - S_d vs μ_{\square}



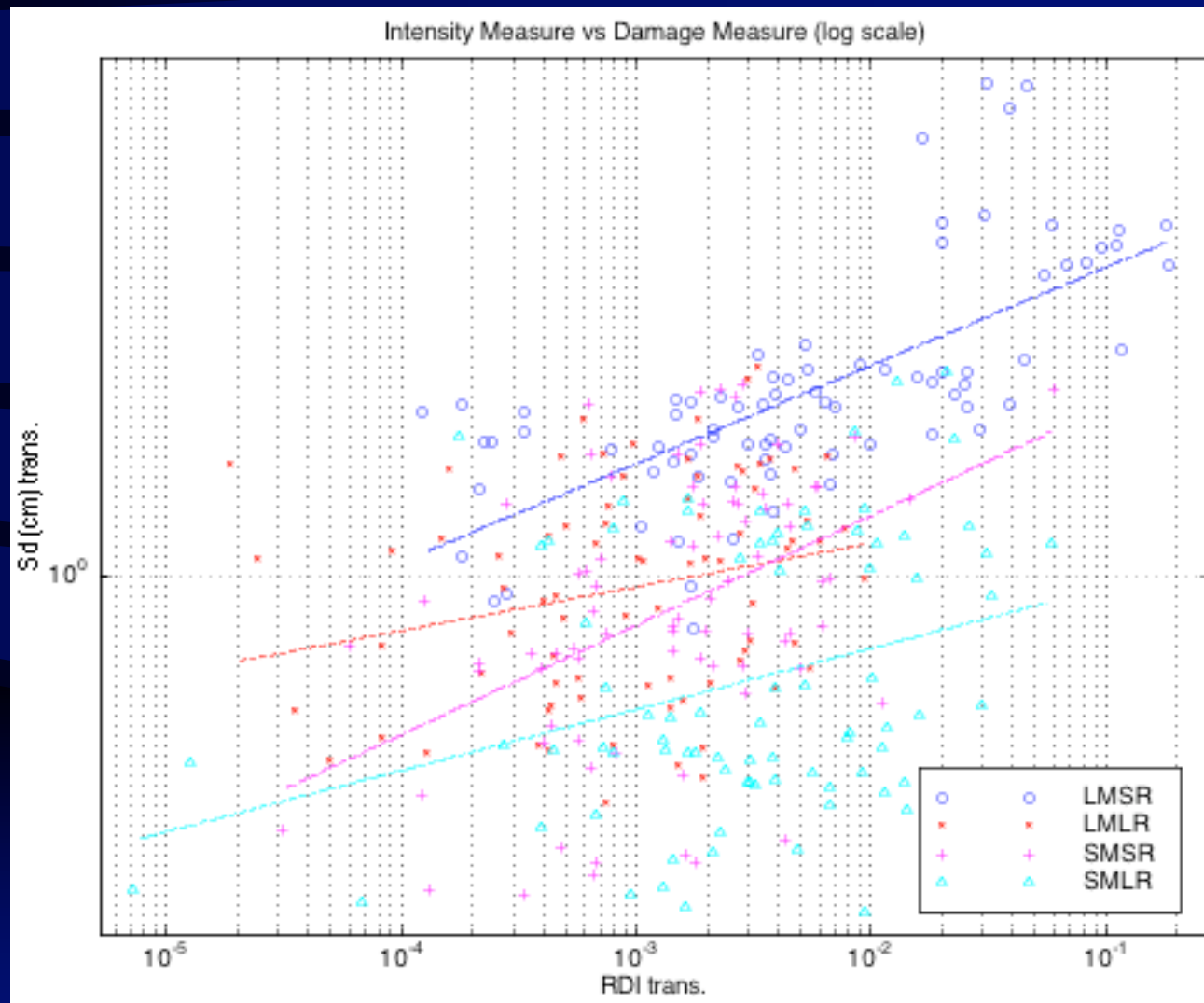
LoH

Design Parameters - Sd vs RDI



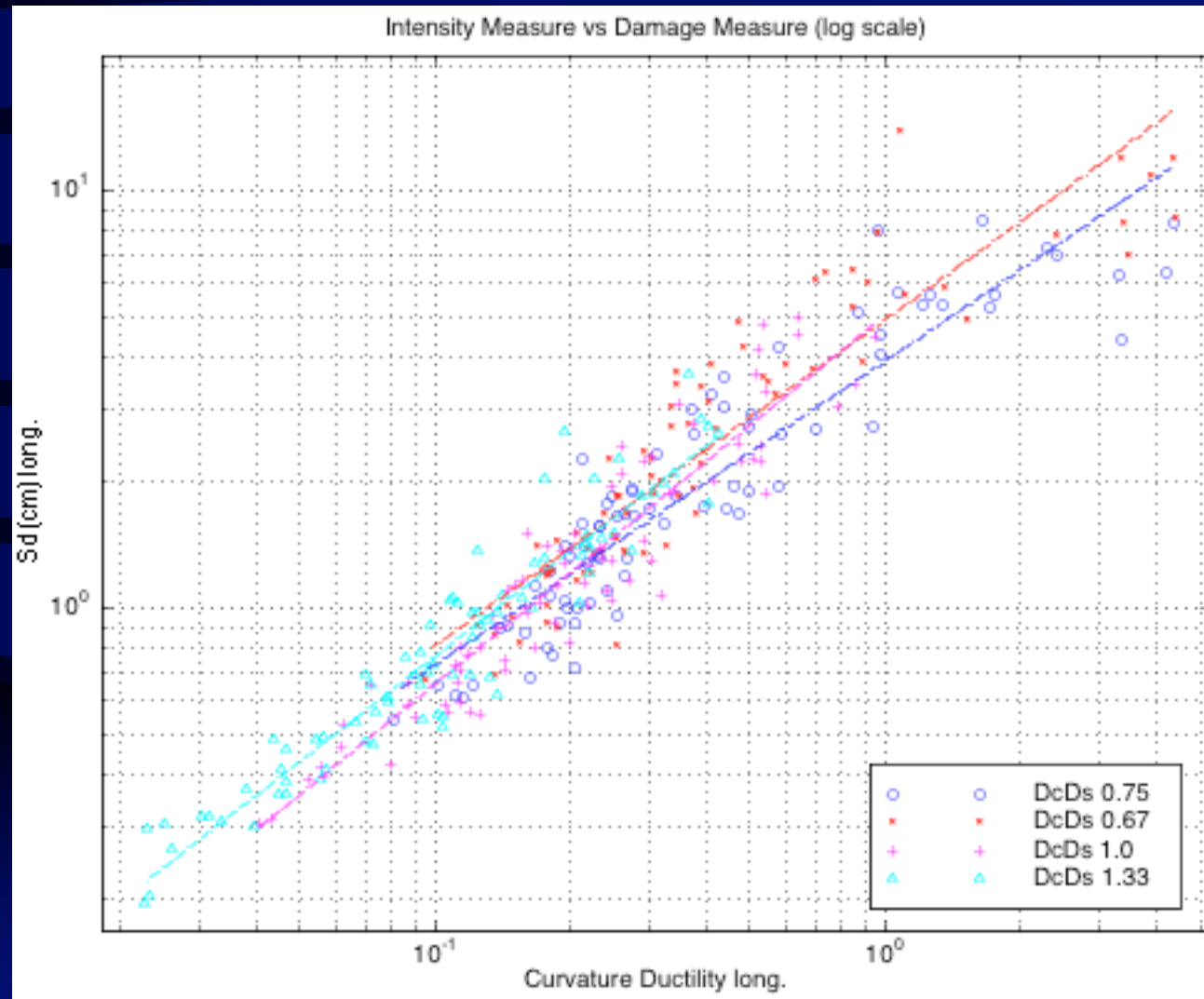
Poor DM choice

Seismicity - Sd vs RDI



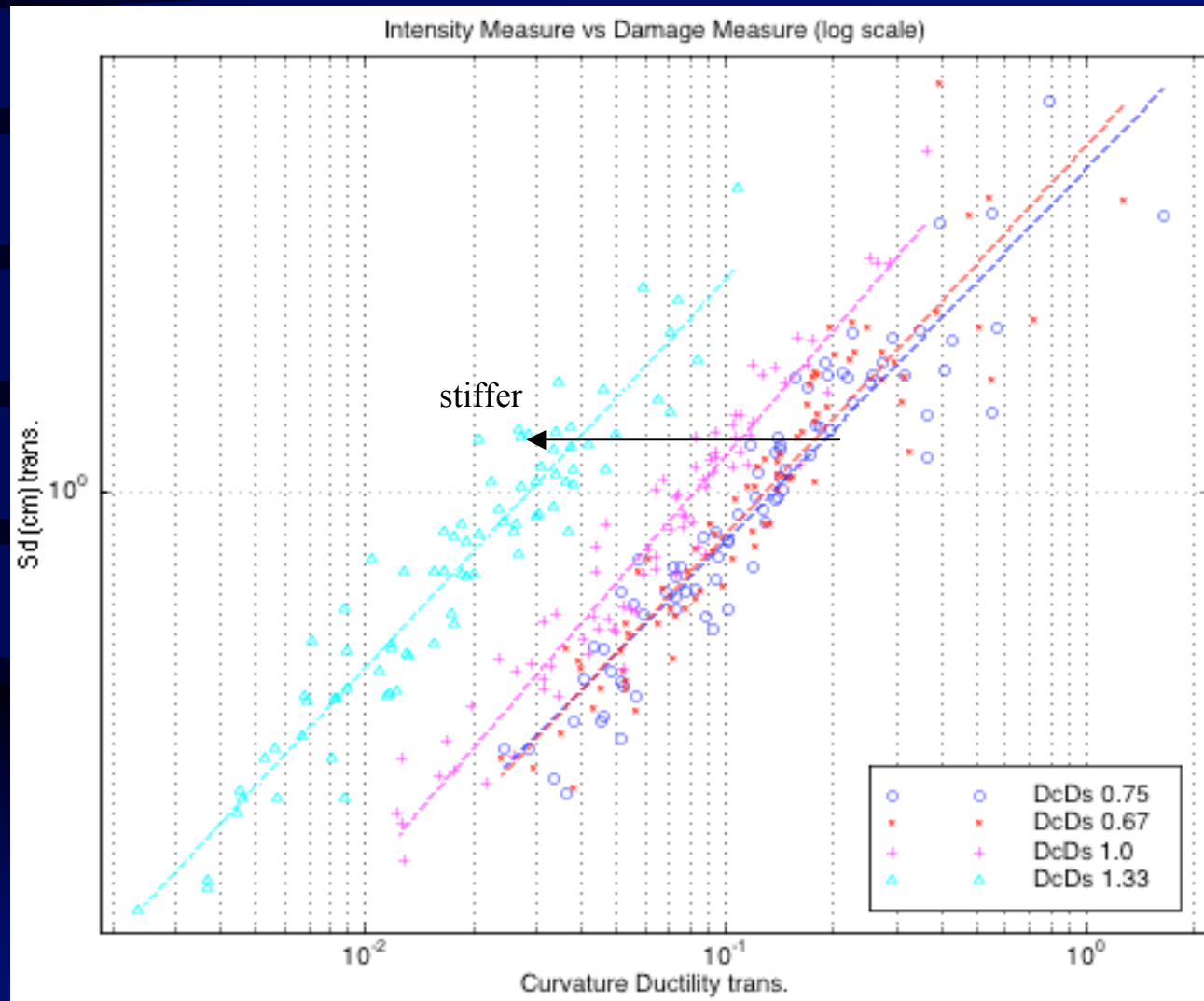
LoH - Poor DM choice

Design Parameters - S_d vs μ_{θ}



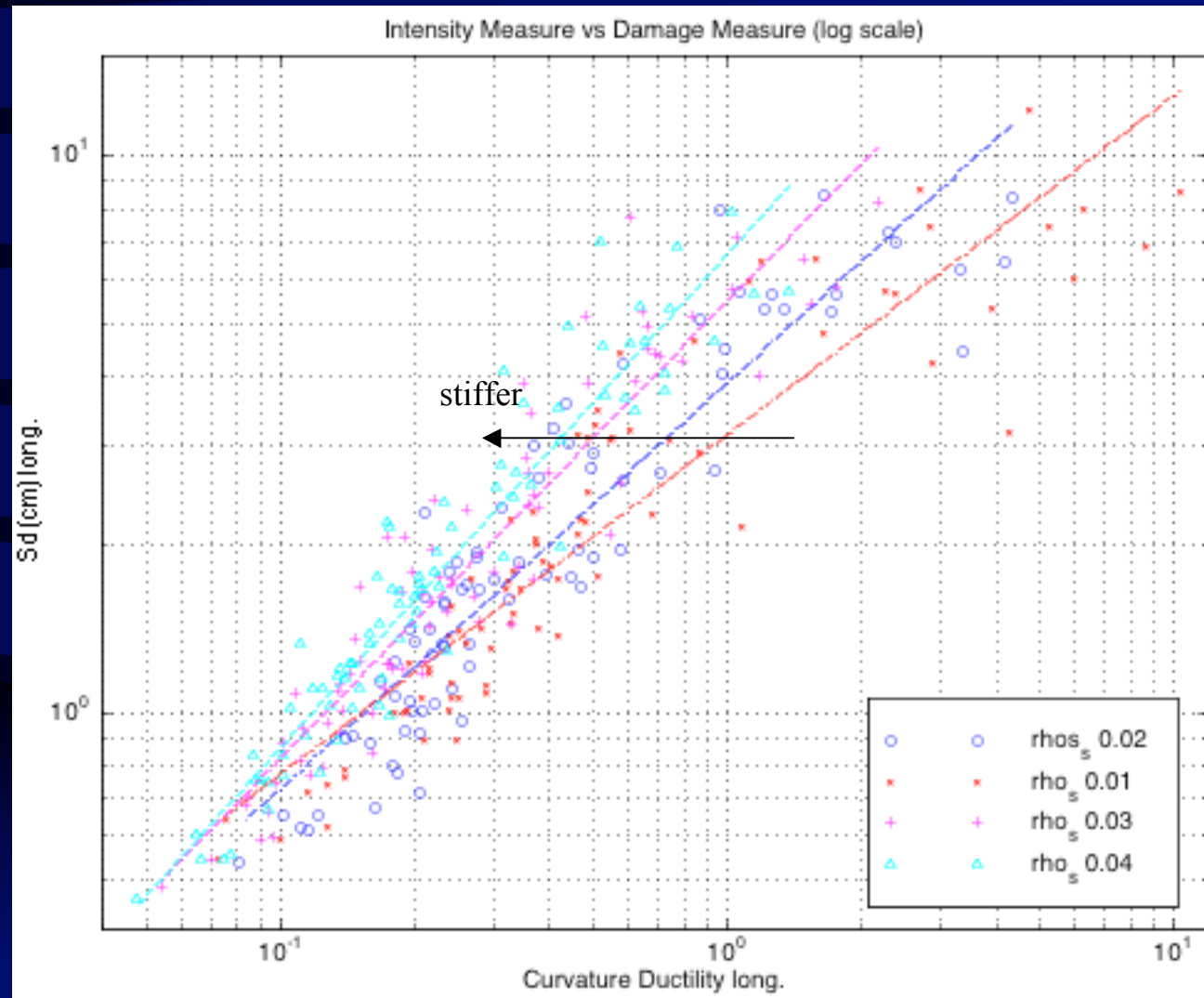
Design parameter does not affect performance

Design Parameters - S_d vs μ



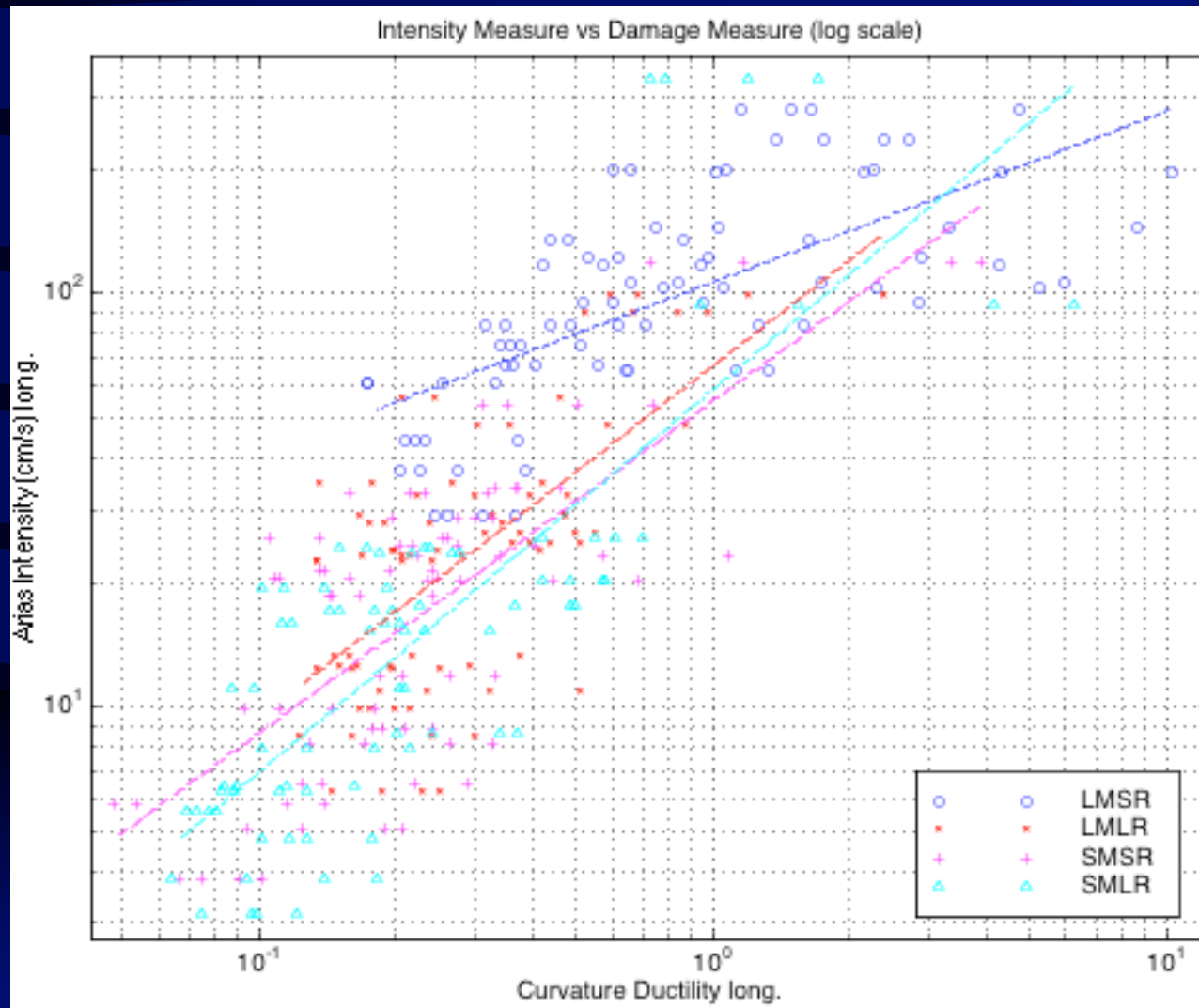
Increasing strength lowers demand. Slope similar for linear case

Design Parameters - S_d vs μ_{θ}

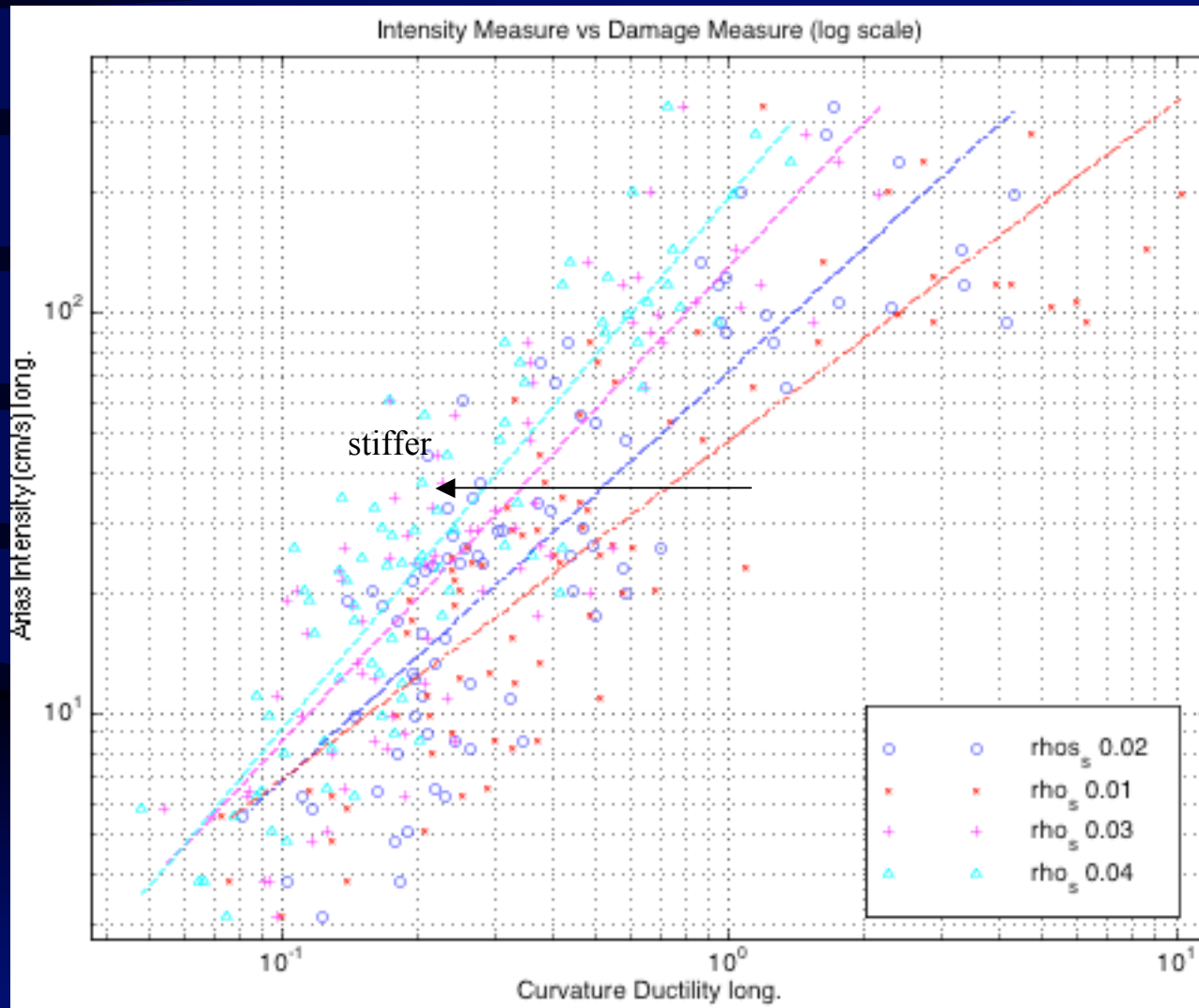


Increasing strength lowers demand. Slope increases with strength for nonlinear case

Seismicity - AI vs μ

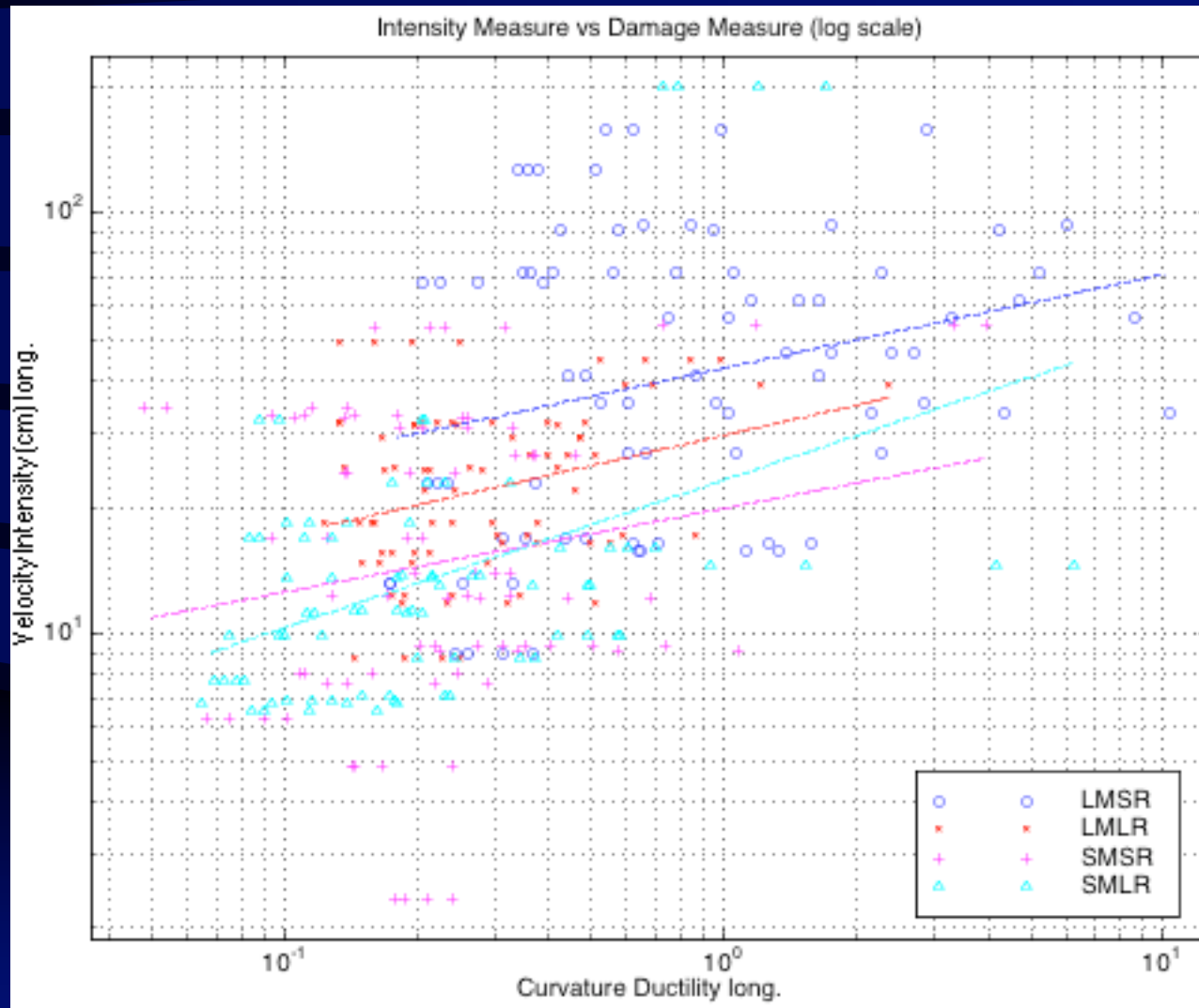


Design Parameters - AI vs ρ_s

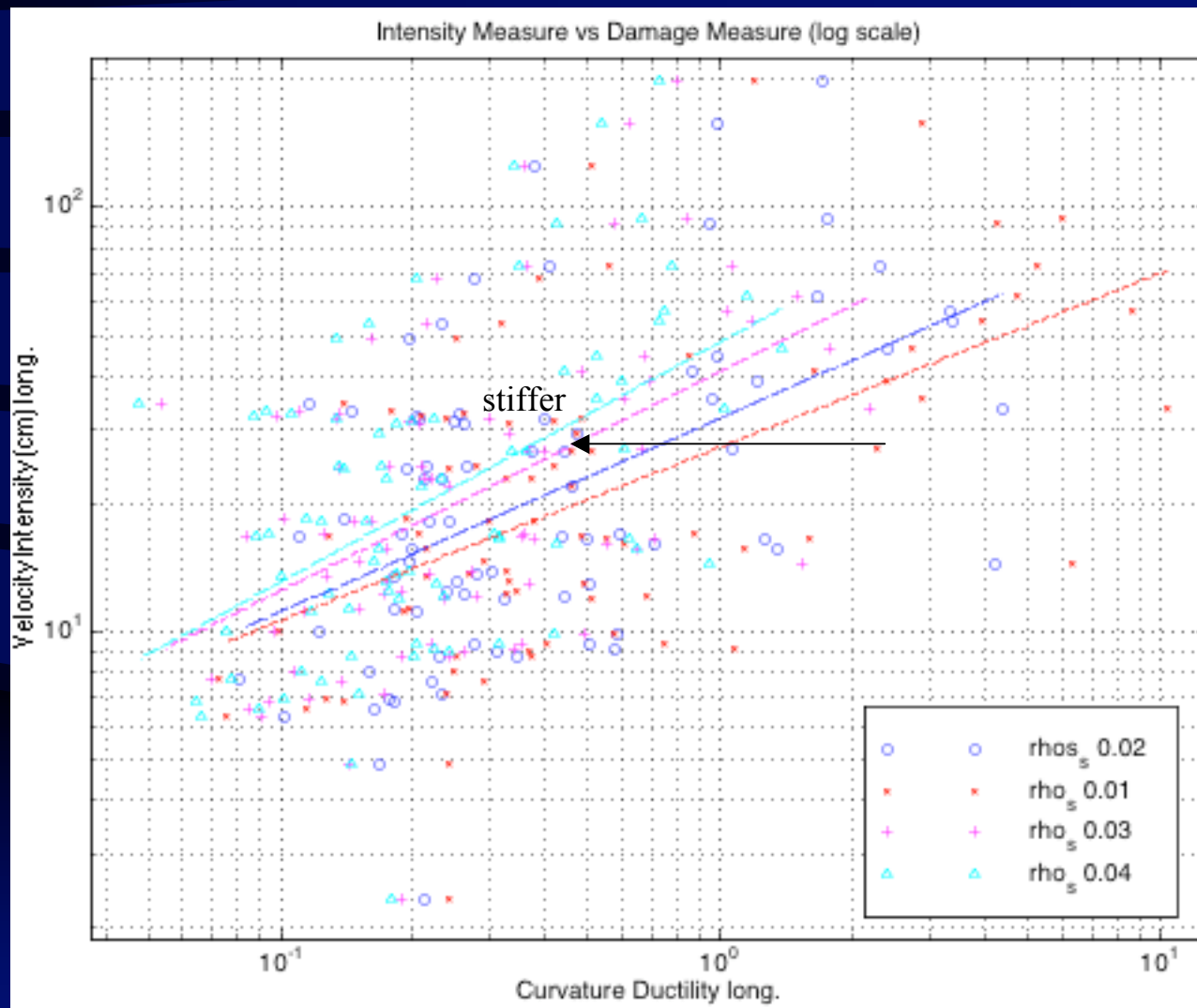


Increasing strength lowers demand

Seismicity - VI vs μ_{long}

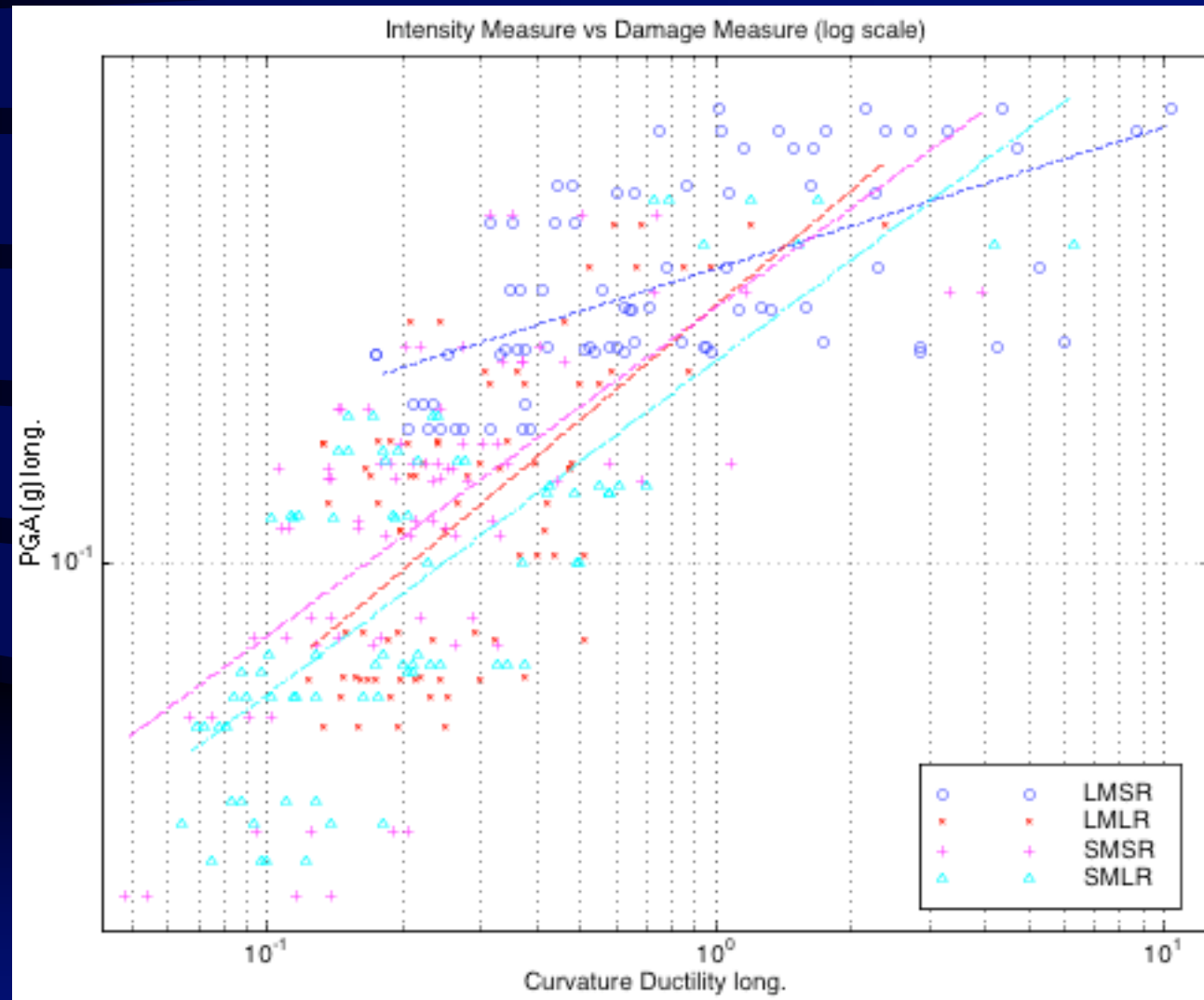


Design Parameters - VI vs ρ_s

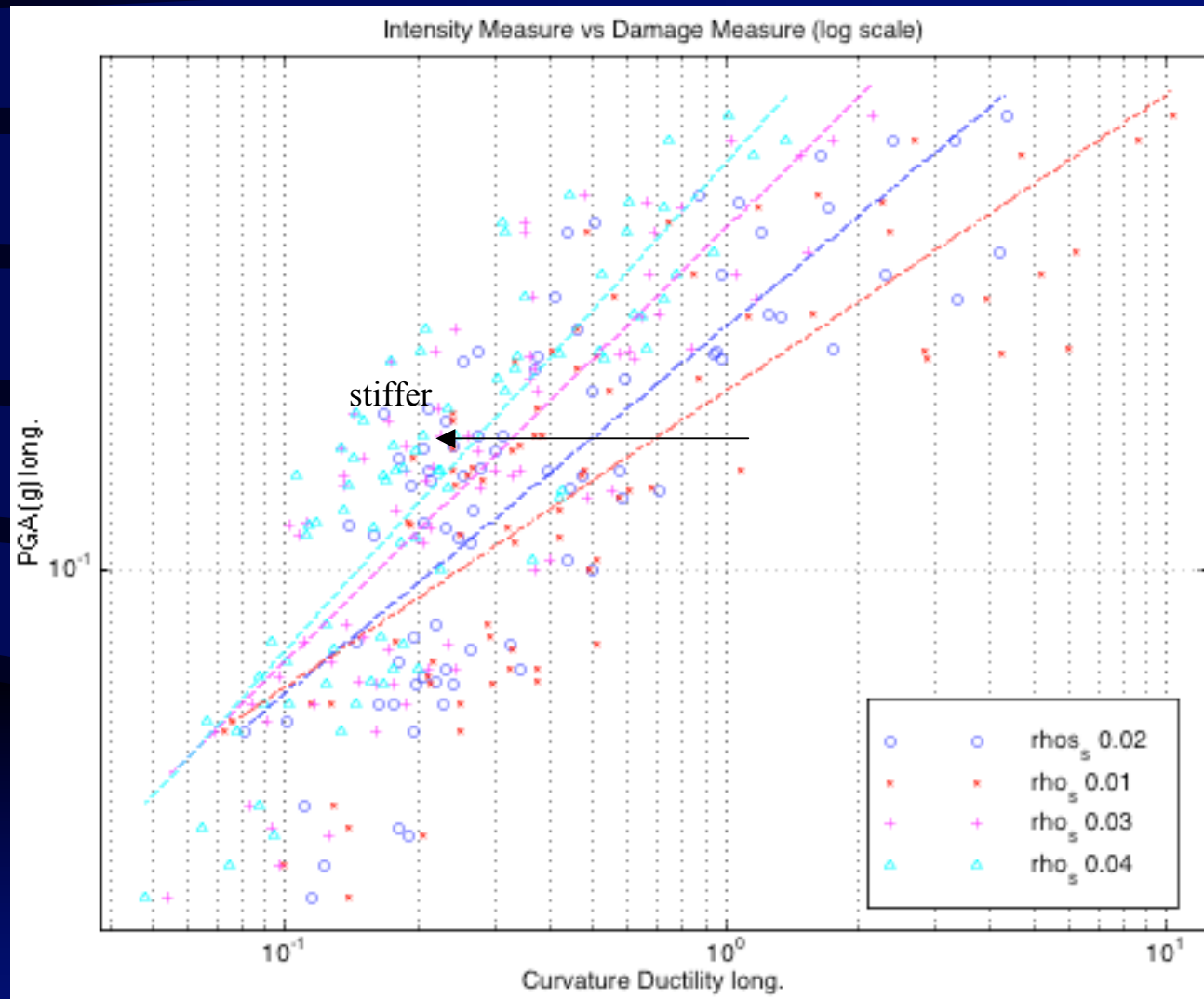


Increasing strength lowers demand, high scatter

Seismicity - PGA vs μ_s

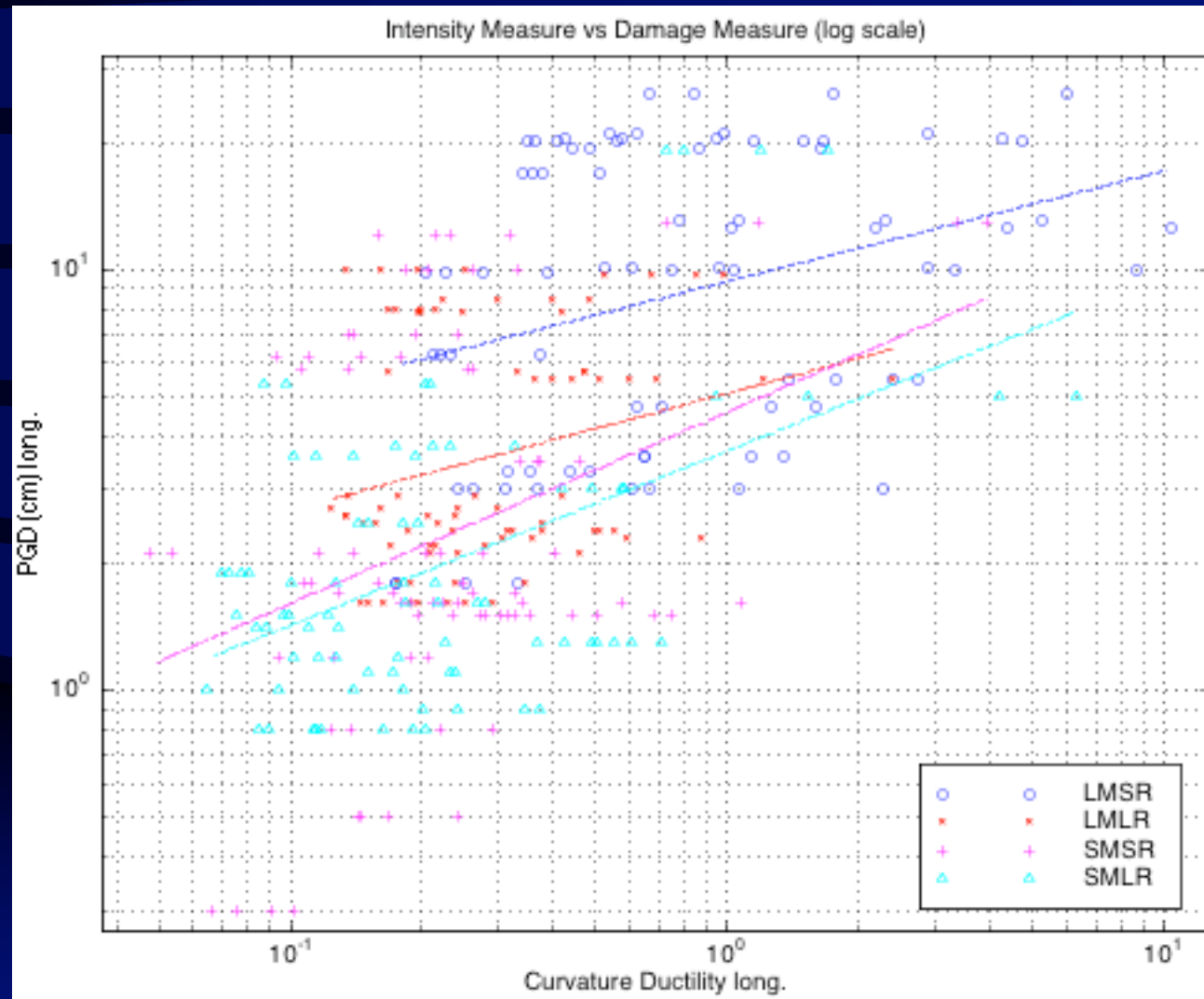


Design Parameters - PGA vs μ_{\square}



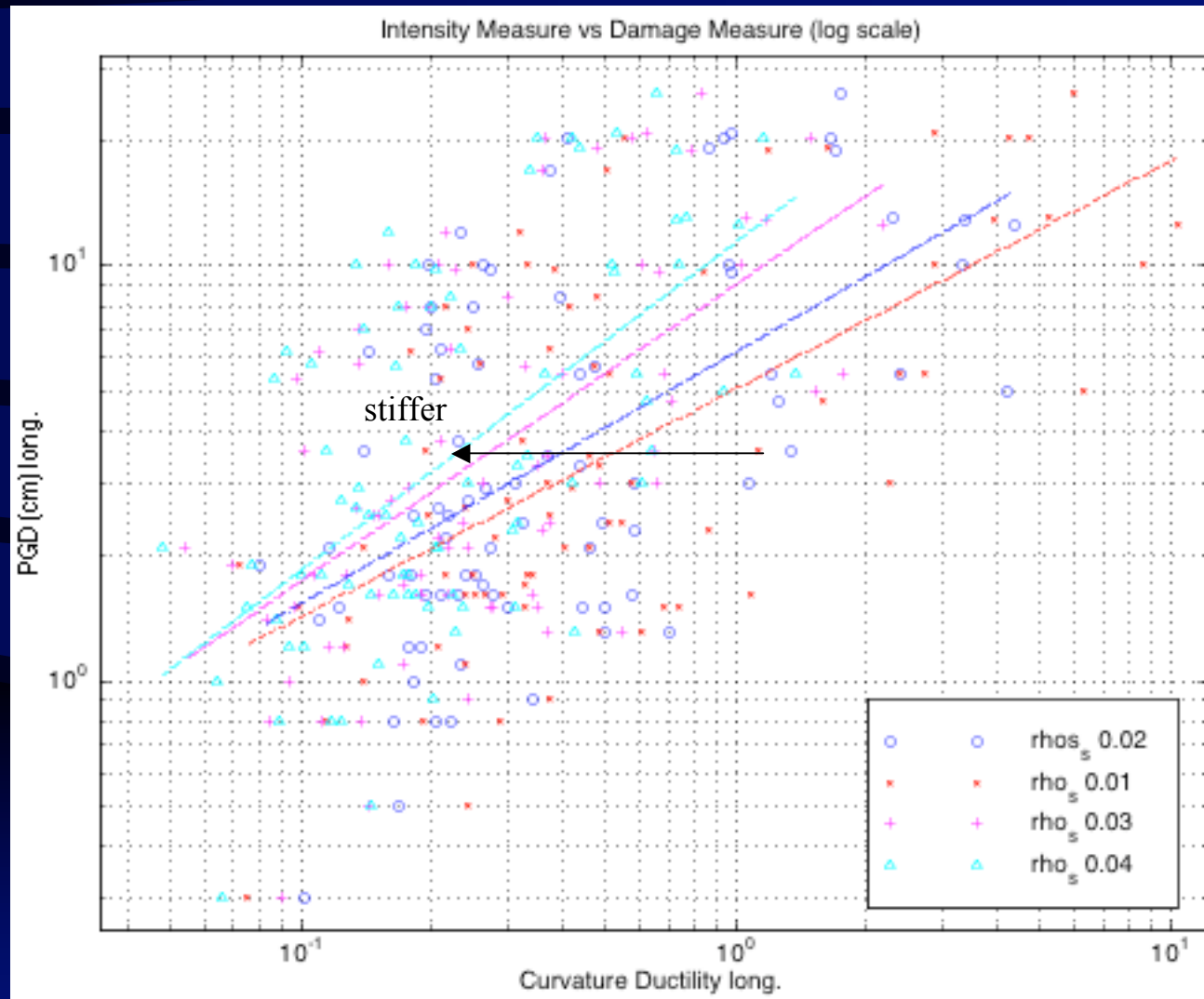
Increasing strength lowers demand

Seismicity - PGD vs μ_{\square}



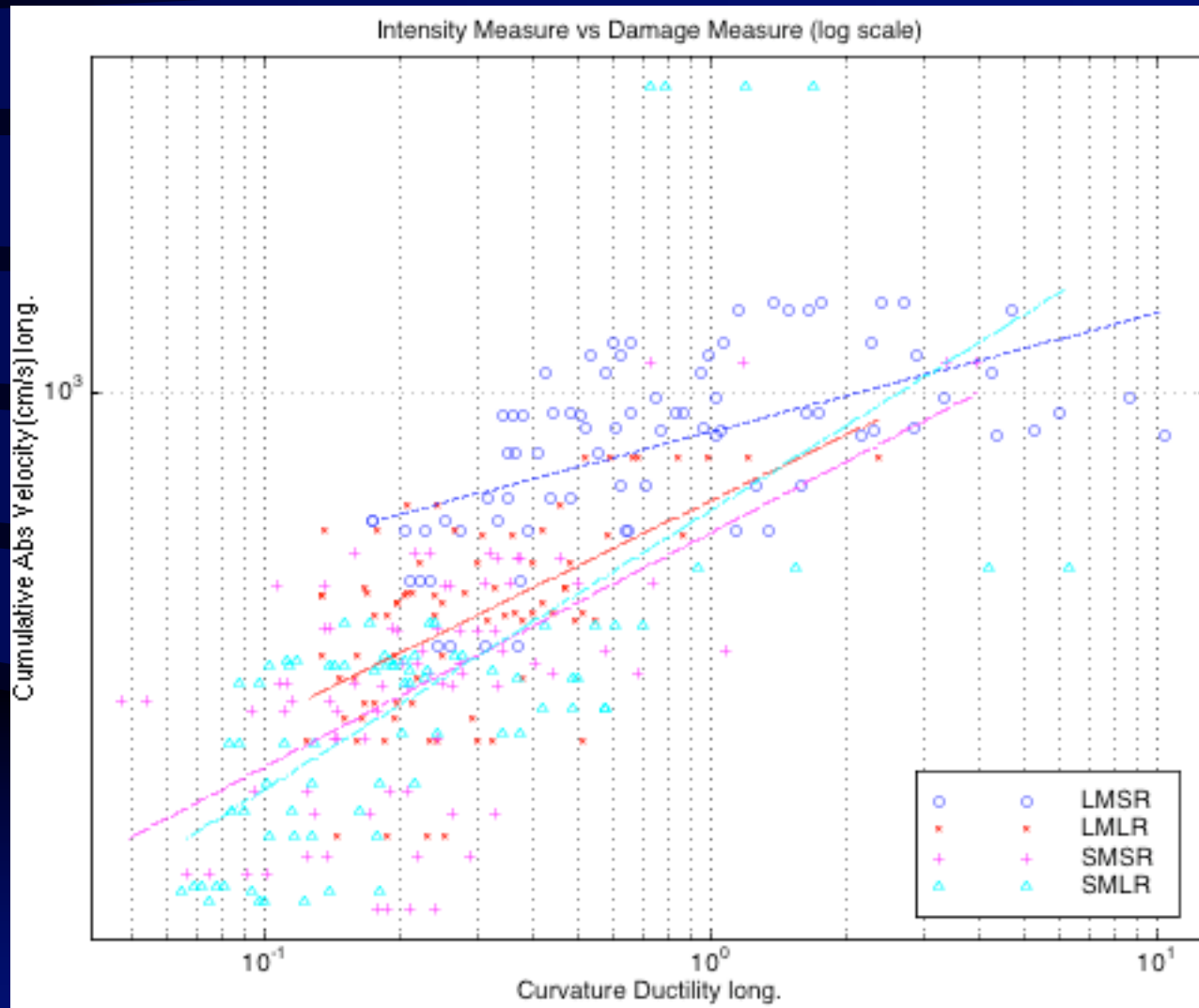
μ_{\square} s - poor IM choice

Design Parameters - PGD vs μ_{\square}

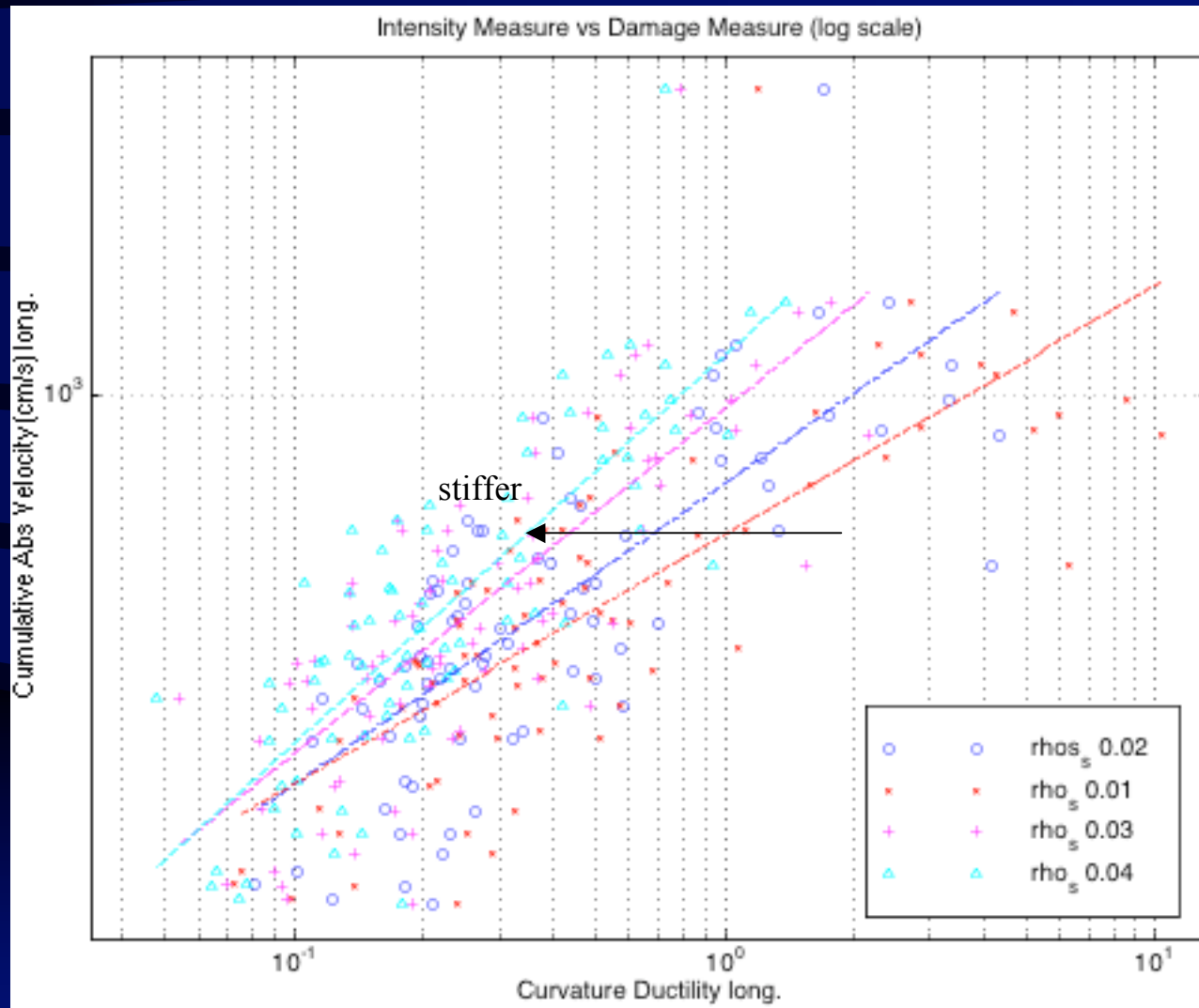


Poor IM choice

Seismicity - CAV vs μ_{long}

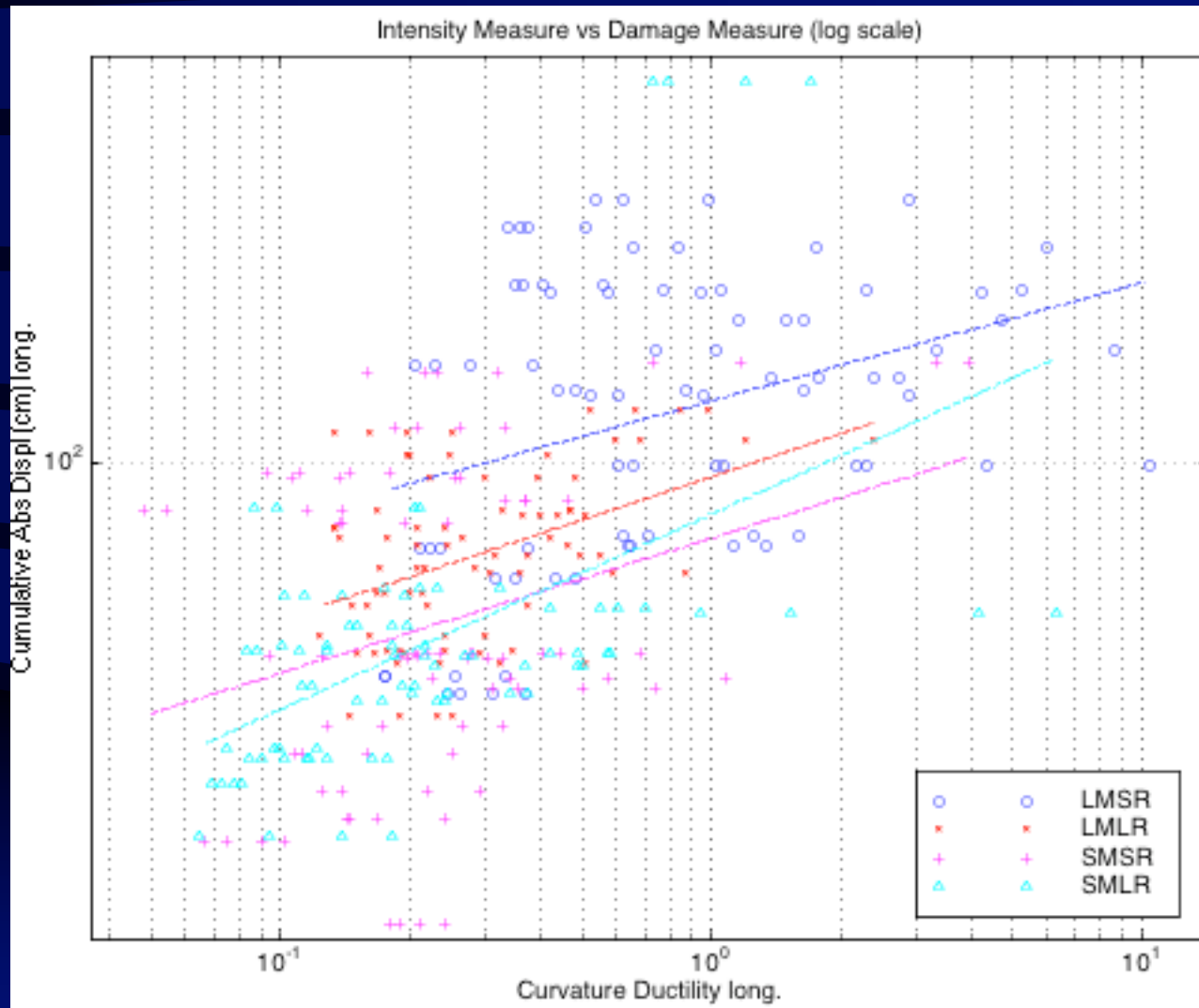


Design Parameters - CAV vs μ_{\square}



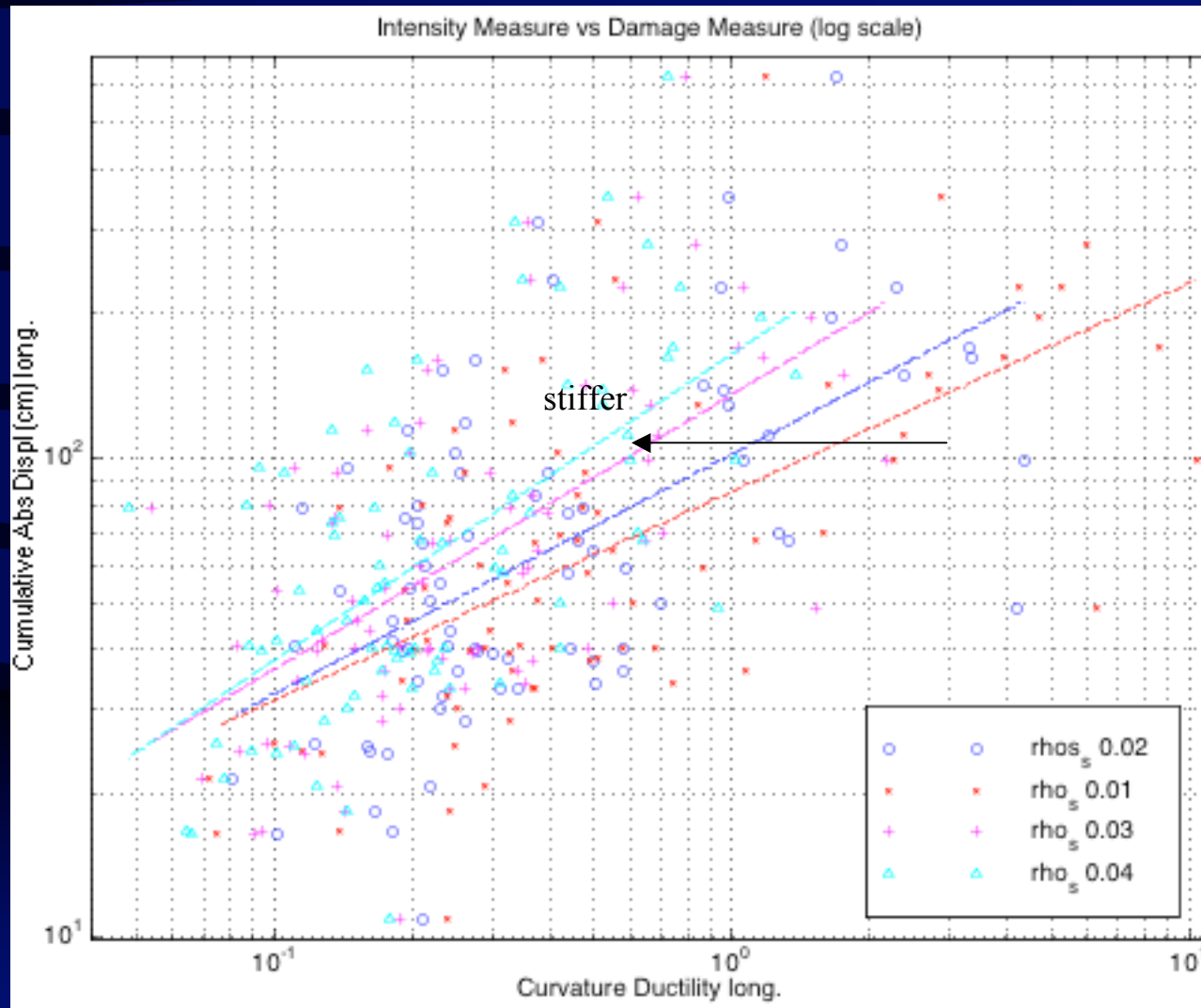
Increasing strength lowers demand

Seismicity - CAD vs \square_{\square}



\square_{\square} - poor IM choice

Design Parameters - CAD vs \square



Poor IM choice

Current Status

- All ground motions and all bridge parameters successfully run
- Specific bridge has complete database of all IM-DM combinations
- Abutment dominated performance for short bridge creates long/trans data discrepancy

Immediate Future

- Evaluate all DM-IM combinations and trends
- Refine abutment model
- Address other bridge configurations with select few DM-IM pairs
- M, R dependence of DM
- Comparison with SDOF and simpler analysis techniques

Conclusion

- Formulate a demand model that fits into PEER performance-based design framework
- Allow designers to see the effects of:
 - seismicity
 - design parameterson seismic performance of a bridge