



# Framework for Performance-Based Earthquake Engineering

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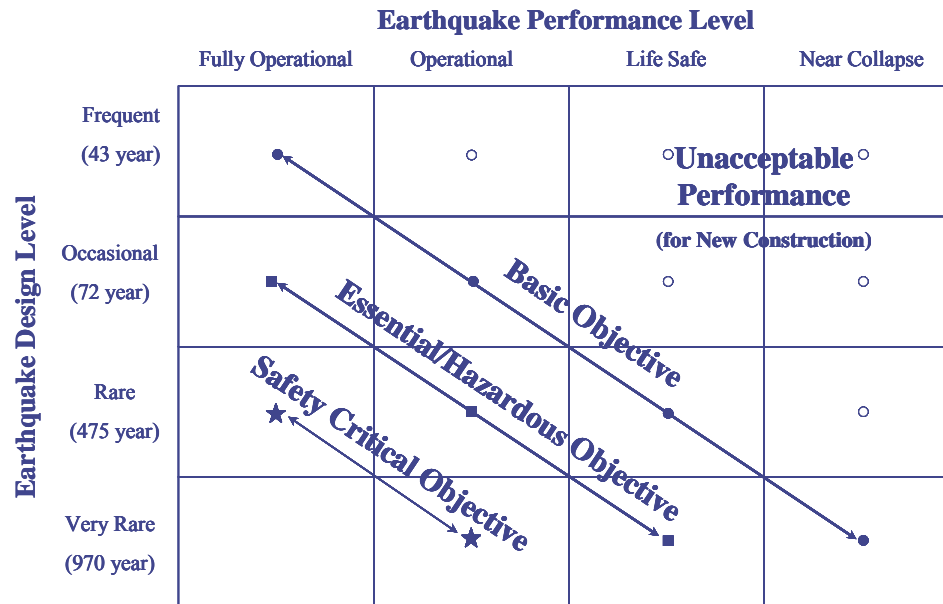
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**PEER Summative Meeting – June 13, 2007**

# Where were we 10 years ago?

## SEAOC Vision 2000, FEMA 273, ATC-40

- Descriptive performance levels (IO, LS, CP, etc.)
- Associated with specific hazard levels → Performance Objectives
- Qualitative (and a few quantitative) damage measures
- Limited consideration of uncertainties
- Implementation in terms of **FORCES and DEFORMATIONS**



# Measures of Performance - PBEE

## ◆ Forces and deformation?

- Yes, but **only** for engineering calculations
- Intermediate variables
- Not for communication with clients and community

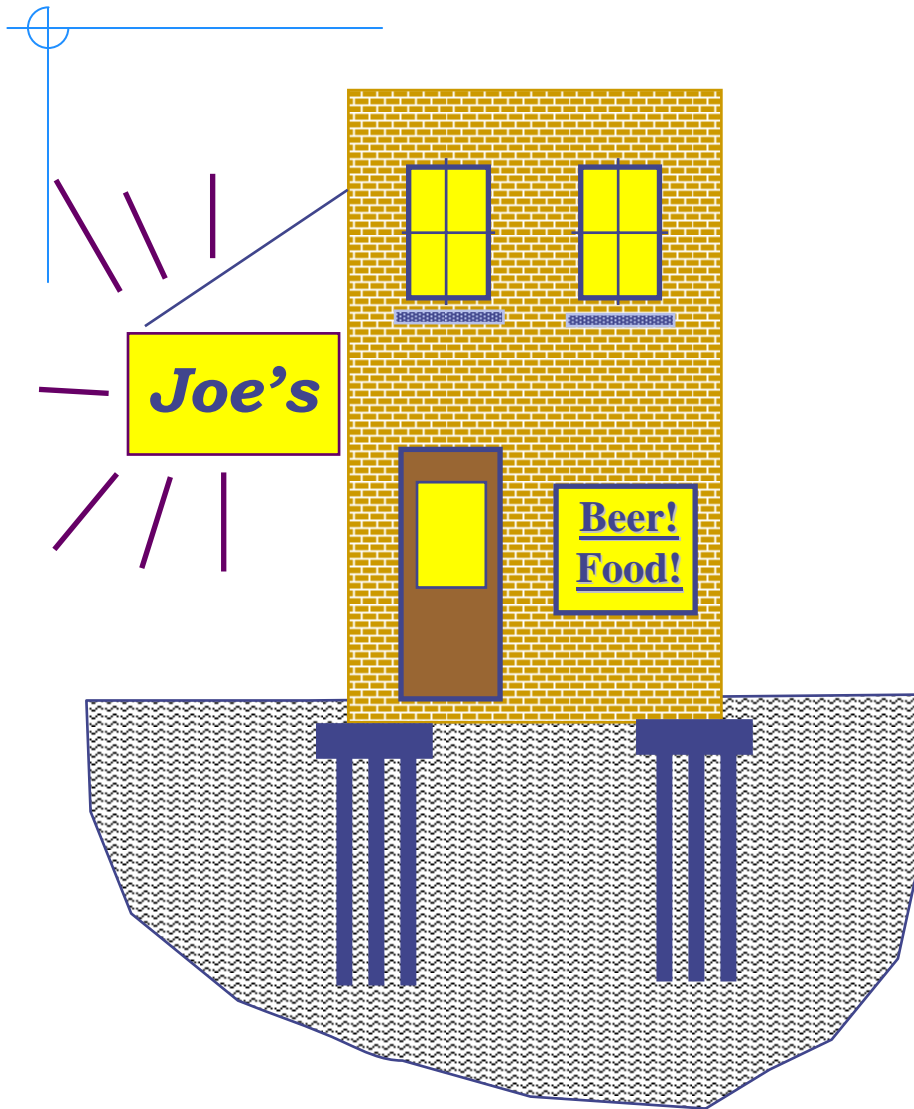
## ◆ Communication in terms of the three D's:

- Dollars (direct economic loss)
- Downtime (loss of operation/occupancy)
- Death (injuries, fatalities, collapse)

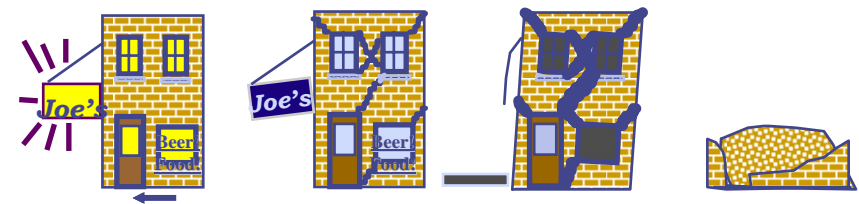
## ◆ Quantification

- Losses for a given shaking intensity
- Losses for a specific scenario (M & R)
- Annualized losses
- With or without rigorous consideration of uncertainties

# Vision of PBEE



1. Complete simulation
2. Defined performance objectives
  - Quantifiable performance targets
  - Annual probabilities of achieving them
3. Informed owners



Sources: G. Deierlein, R. Hamburger

# The Peer Framework Equation - 1999

$$v(DV) = \iiint G\langle DV | DM \rangle | dG\langle DM | EDP \rangle | dG\langle EDP | IM \rangle | d\lambda(IM)$$

Impact

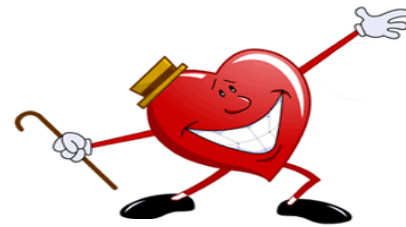
Performance (Loss) Models and Simulation

Hazard

Curse?



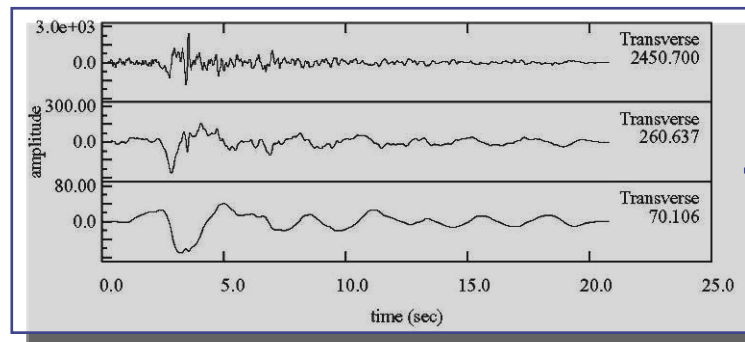
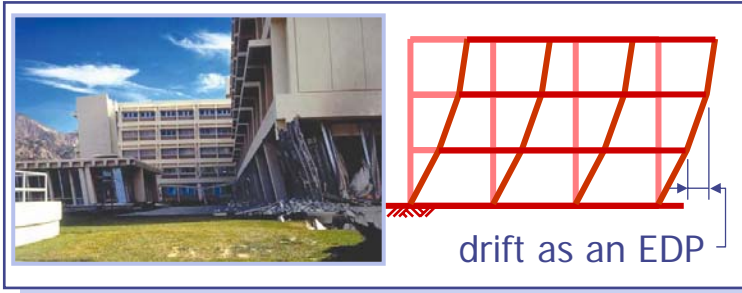
Blessing



# Performance-Based Methodology – Bldgs.

## Measures of Performance

- Collapse & Casualties
- Direct Financial Loss
- Downtime



Decision Variable

Damage Measure

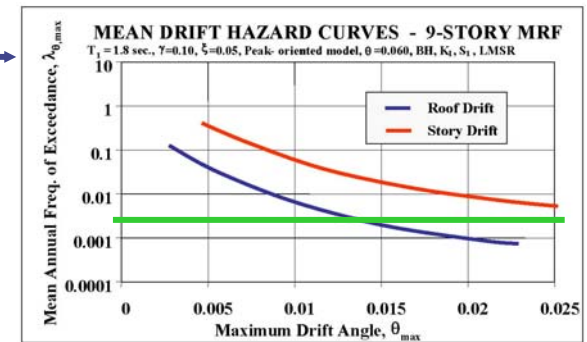
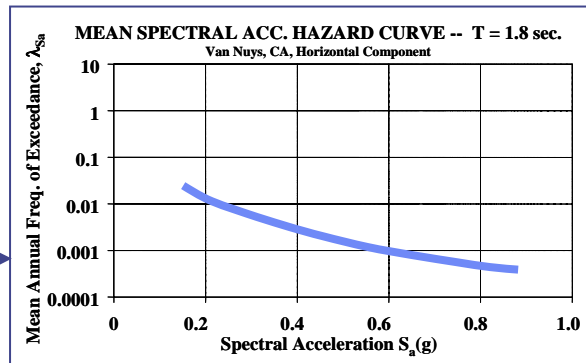
Engineering Demand Parameter

Intensity Measure

# Performance-Based Methodology

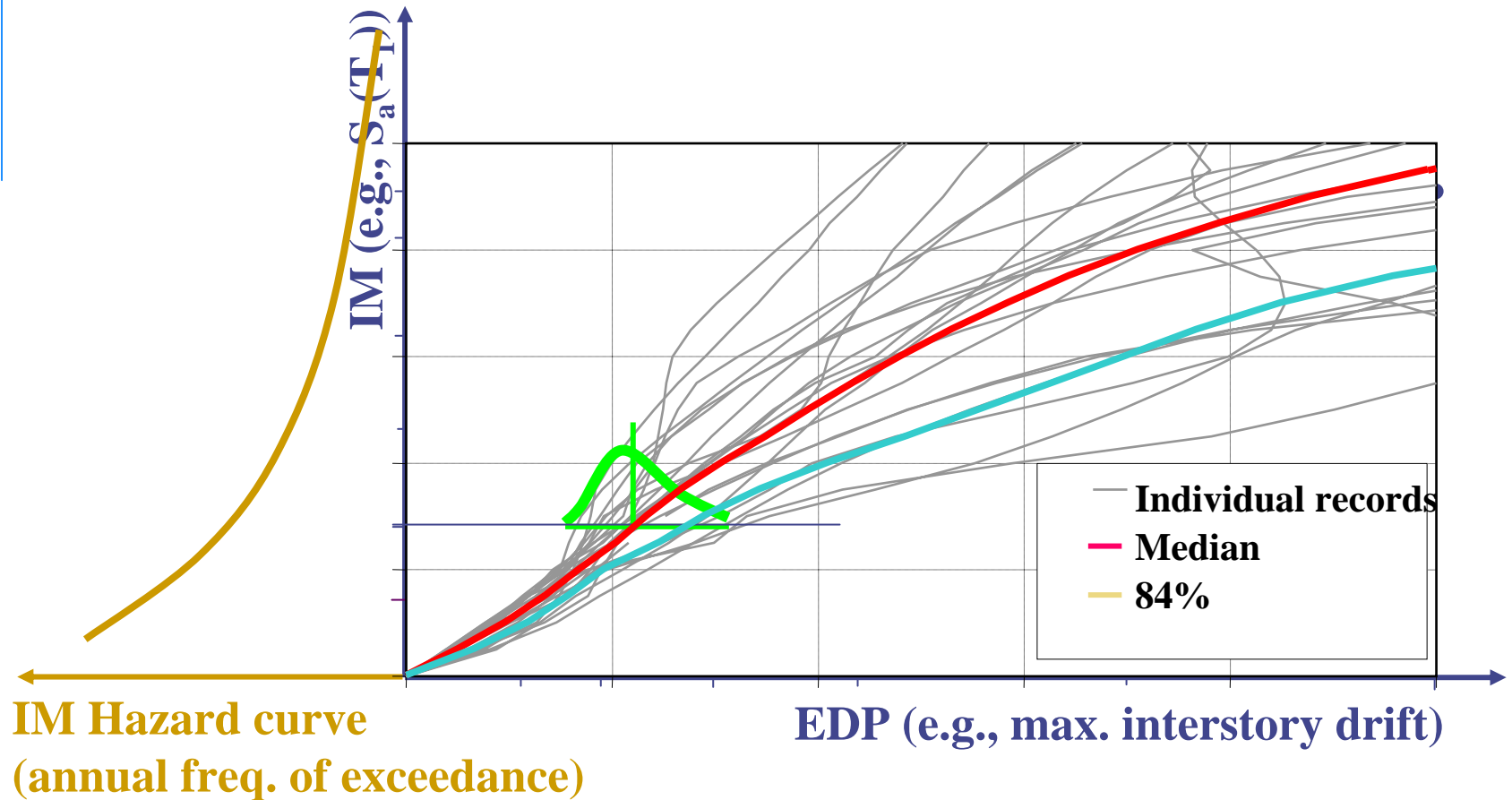
Engineering Demand Parameter

Intensity Measure



Medina & Krawinkler

# Incremental Dynamic Analysis



$$\lambda_{\text{EDP}}(y) = \int P[\text{EDP} \geq y \mid \text{IM} = x] \, d\lambda_{\text{IM}}(x)$$



# Performance-Based Methodology

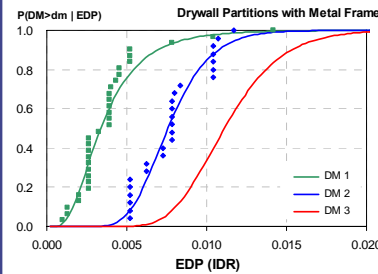
**Decision Variable**

## Performance Assessment types (ATC-58 definitions):

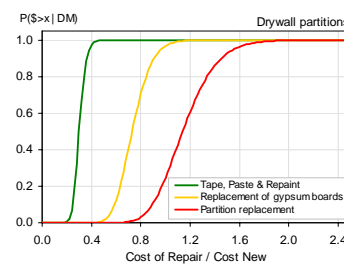
- Intensity-based:** Prob. facility perf., given intensity of ground motion
- Scenario-based:** Prob. facility perf., given a specific earthquake scenario
- Time-based:** Prob. facility perf. In a specific period of time

**Damage Measure**

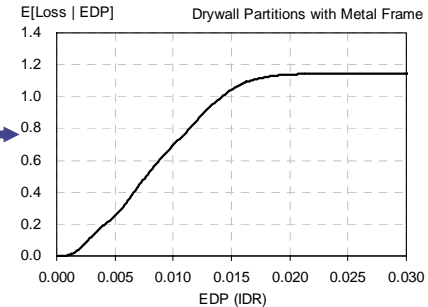
## Damage Fragility Curves:



## Cost Functions:

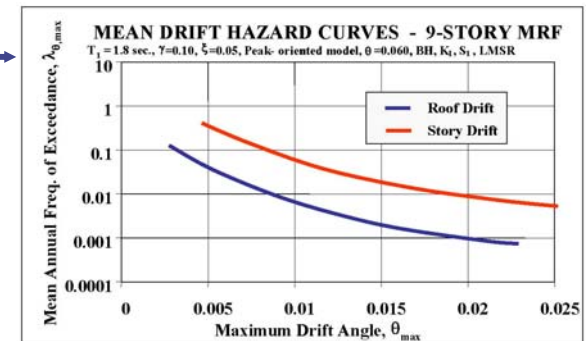
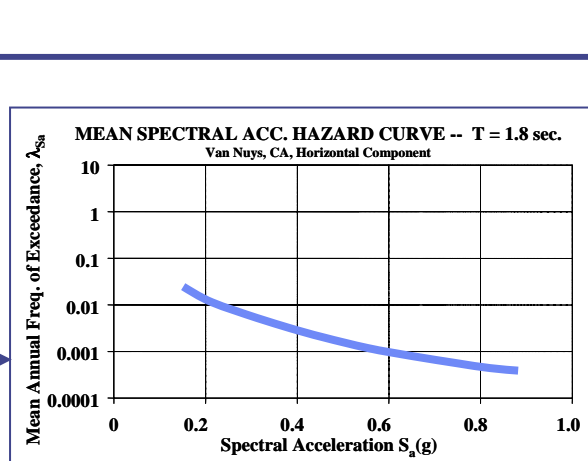


## Mean Loss Curve:



Aslani & Miranda

**Engineering Demand Parameter**

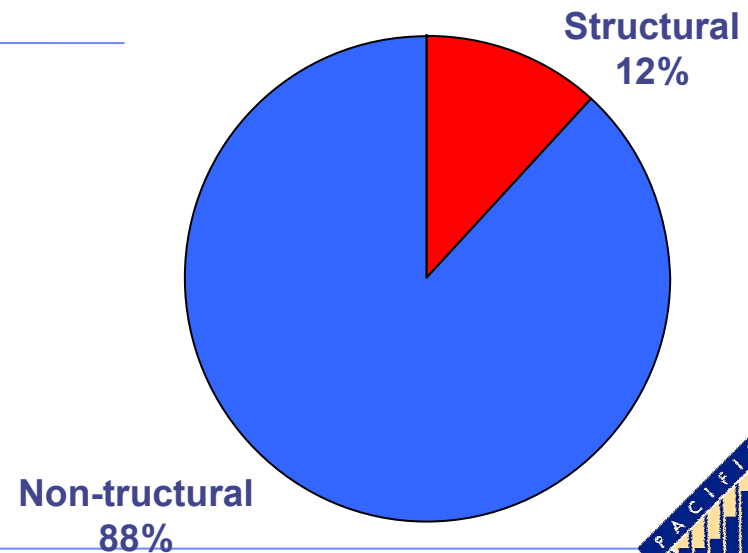
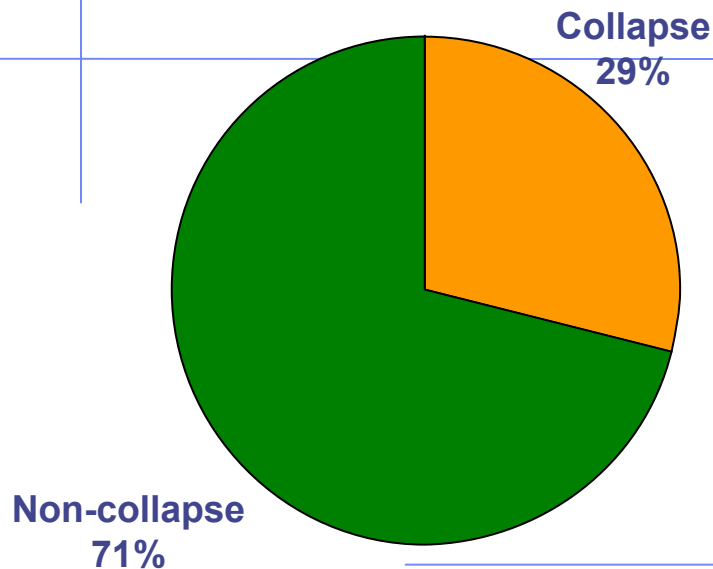


Medina & Krawinkler

**Intensity Measure**

# Deaggregation of Expected Annual Loss

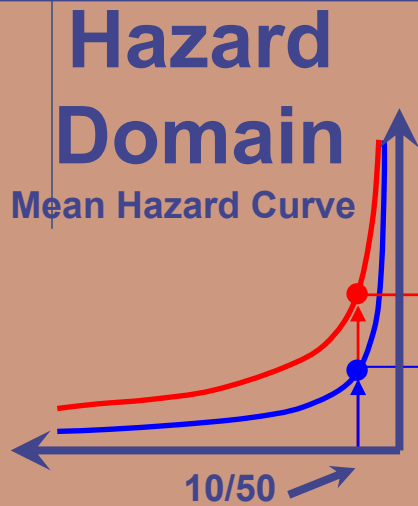
## Example: Van Nuys Testbed Building



Source: E. Miranda

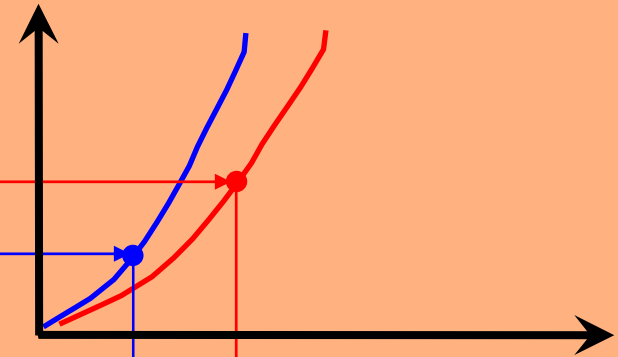
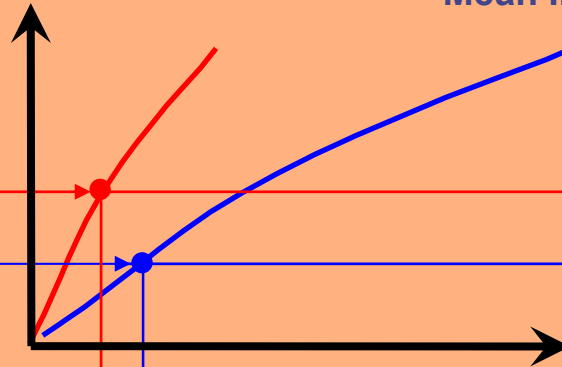


# Design Decision Support



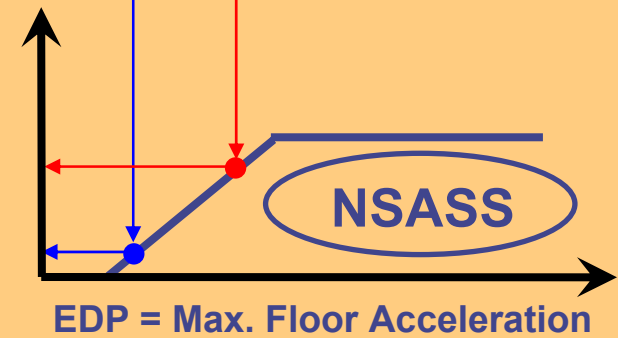
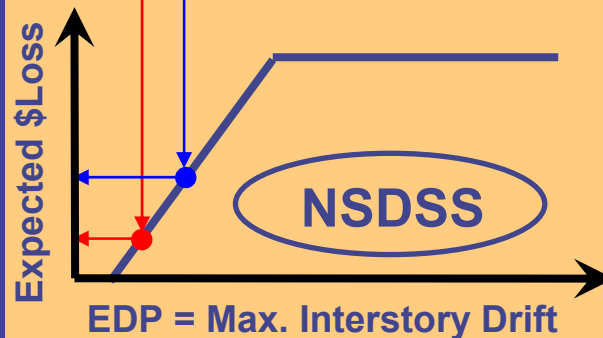
## Structural System Domain

Mean IM-EDP Curves



## Loss Domain

Mean Subsystem Loss Curves



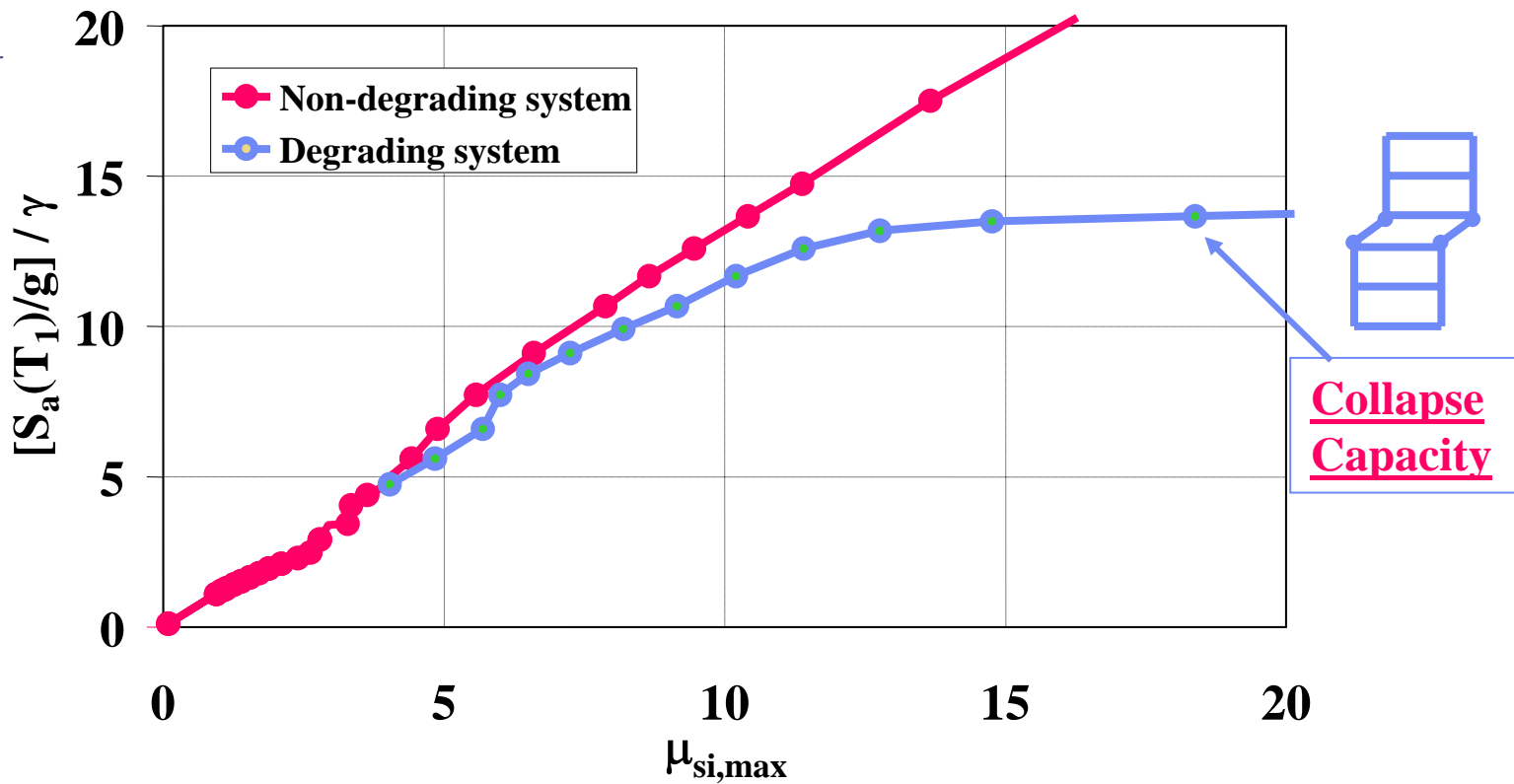
Zareian & Krawinkler (2005)

# Assessment of Collapse Potential

$$\gamma = \frac{V_y}{W}$$

## NORM. STRENGTH VS. MAX. STORY DUCT.

$N=9, T_1=0.9, \xi=0.05, \alpha=0.03, \theta=0.015, H_3, BH, K_1, S_1, NR94nya$

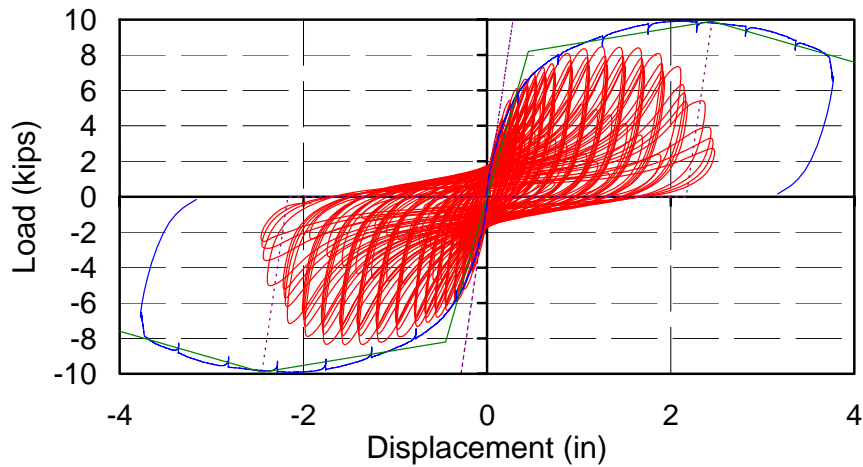


Collapse Capacity

# Modeling of Deterioration

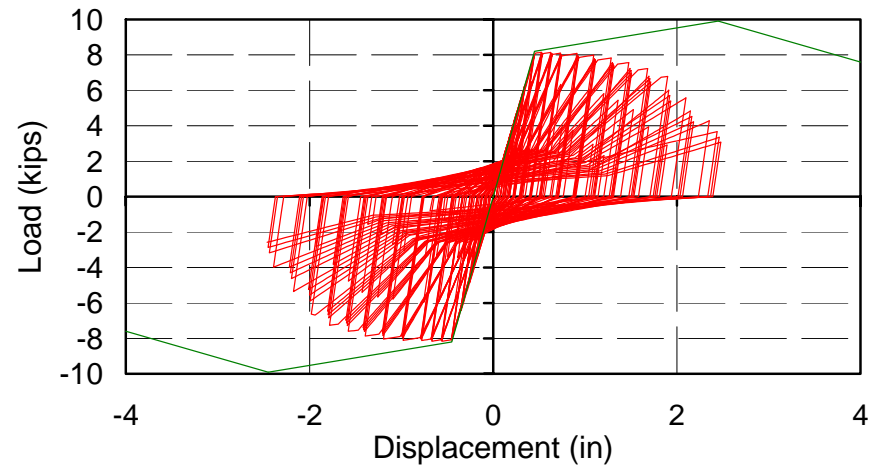
## UCI G12 OSB

$F_y=8.2$  kips,  $\delta_y=0.45$  in,  $\alpha_s=0.047$ ,  $\alpha_c=-0.081$ ,  $\alpha_u=1.94$ ,  $\delta_c/\delta_y=5.44$



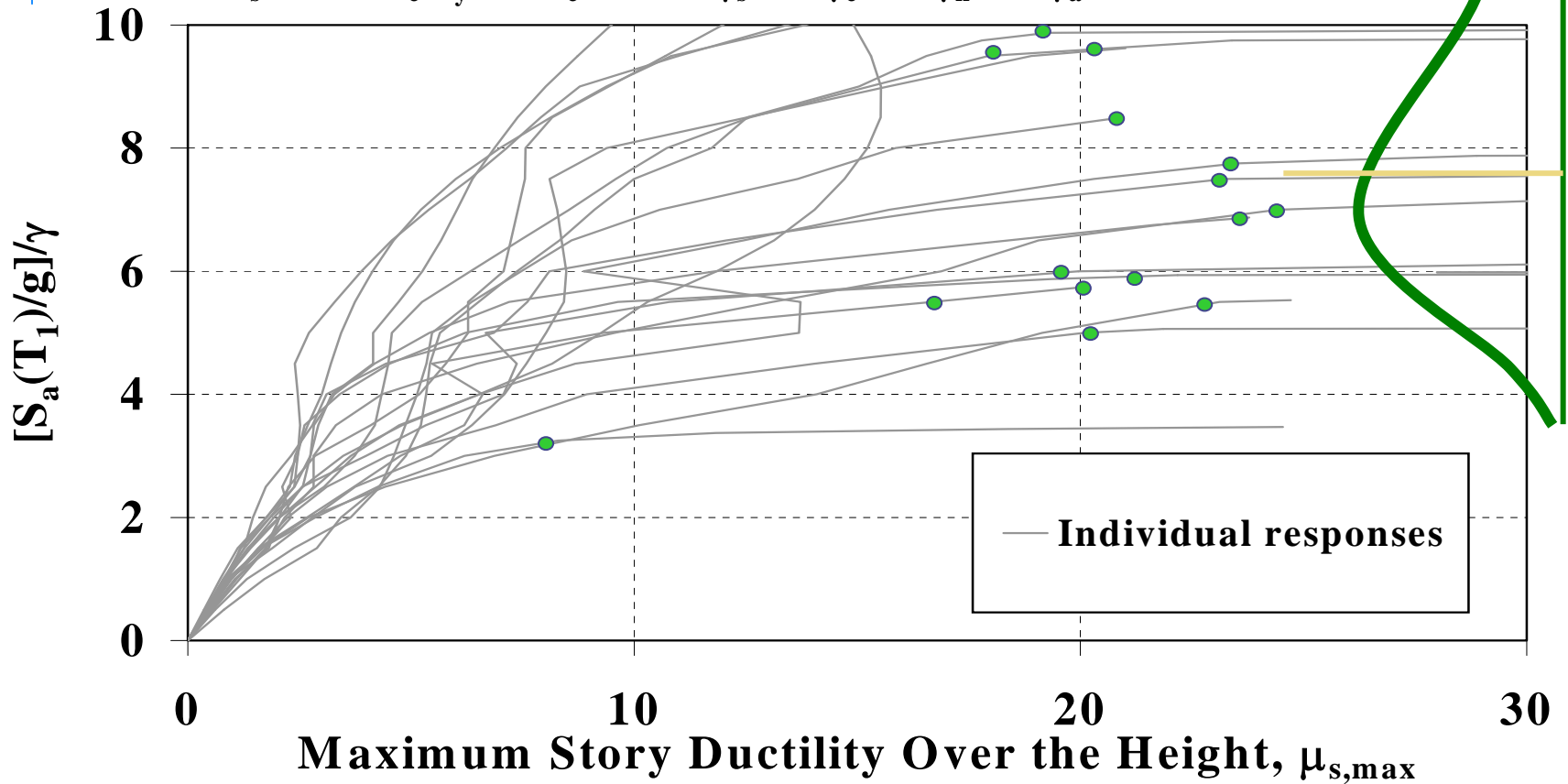
## UCI G12 OSB

Pinching Model,  $\kappa=0.5$ ,  $F_y=8.2$  kips,  $\delta_y=0.45$  in  
 $\alpha_s=0.047$ ,  $\alpha_c=-0.081$ ,  $\alpha_c=1.94$ ,  $\delta_c/\delta_y=5.44$ ,  $\gamma_s=270$ ,  $\gamma_c=270$ ,  $\gamma_k=\infty$ ,  $\gamma_a=270$



# Collapse Capacity for a Set of Ground Motions

**MAX. STORY DUCTILITY vs. NORM. STRENGTH**  
**N=9,  $T_1=0.9$ ,  $\xi=0.05$ ,  $K_1$ ,  $S_1$ , BH,  $\theta=0.015$ , Peak-Oriented Model,**  
 **$\alpha_s=0.05$ ,  $\delta_c/\delta_y=4$ ,  $\alpha_c=-0.10$ ,  $\gamma_s=8$ ,  $\gamma_c=8$ ,  $\gamma_k=8$ ,  $\gamma_a=8$ ,  $\lambda=0$ , LMSR**

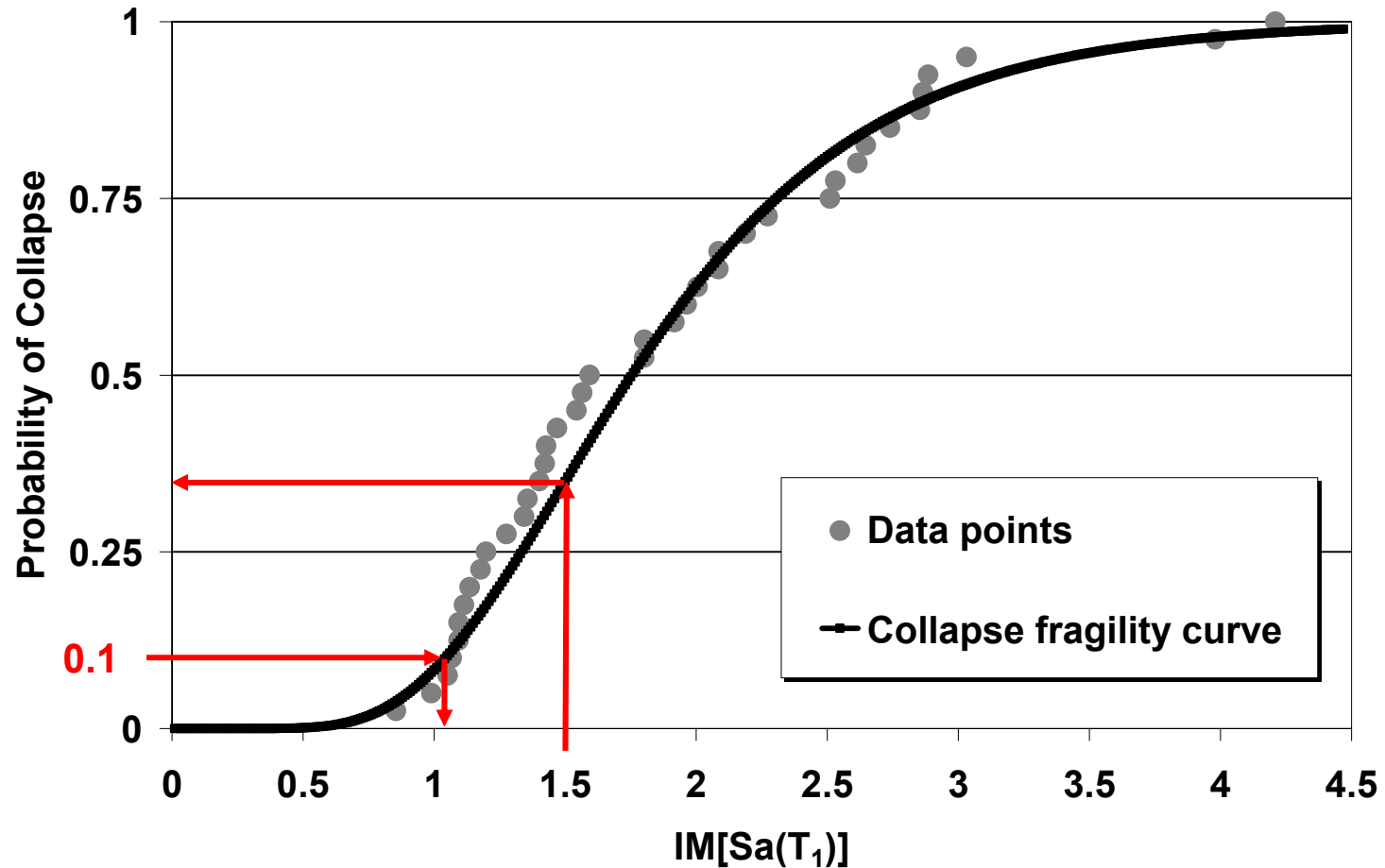


# Collapse Fragility Curve

## Obtaining the collapse fragility curve (MRF)

$N = 8$ ,  $T_1 = 1.2$ ,  $\gamma = 0.17$ , Stiff & Str = Shear, SCB = 2.4-2.4,  $\xi = 0.05$

$\theta_p = 0.03$ ,  $\theta_{pc}/\theta_p = 5$ ,  $\lambda = 20$ ,  $M_c/M_y = 1.1$

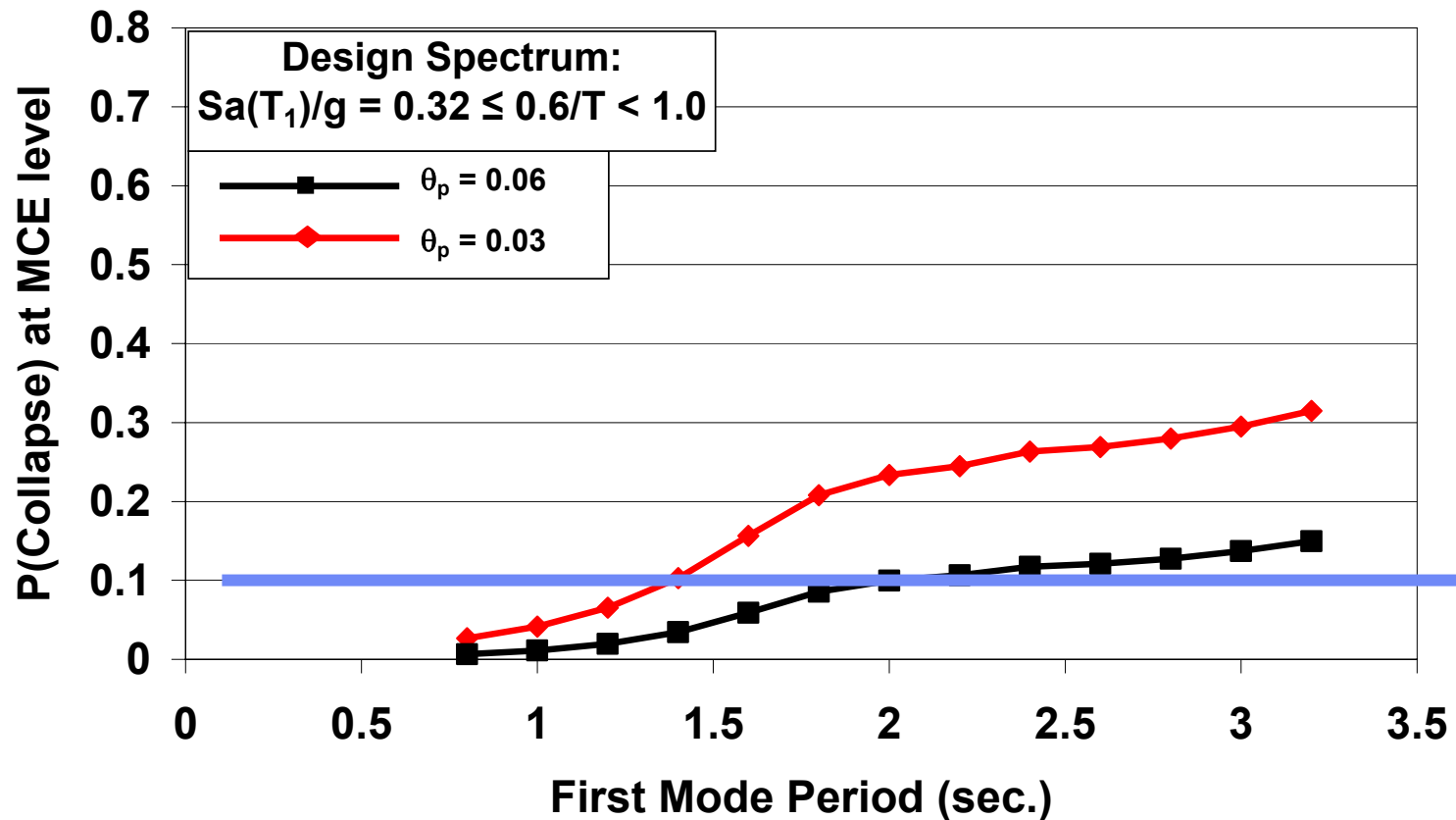


Zareian & Krawinkler (2004)

# Probability of Collapse at MCE, for MRFs with $R = 8$

## P(Collapse) at MCE given $R = 8$ & $\Omega = 2.5$ (MRF)

Siff. & Str. = Shear, SCB = 2.4-1.2,  $\xi = 0.05$ ,  $\theta_{pc}/\theta_p = 15.0$ ,  $\lambda = 50$ ,  $M_c/M_y = 1.1$



Zareian & Krawinkler (2007)



# Implementation of Framework

- ◆ ***ATC-58*** – Guidelines for Seismic Performance Assessment of Buildings
- ◆ ***ATC-63*** – Recommended Methodology for Quantification of Building System Performance
- ◆ ***TBI*** – Tall Building Initiative
- ◆ ***LRFD*** for bridge design

# Concluding Remarks - 1999

- **Performance based engineering is here to stay**
- **It enforces a transparent design/assessment approach**
- **Much more emphasis must be placed on \$ losses and loss of function (downtime)**
- **Performance based design should be reliability based**
- **We have a long road ahead of us**

