

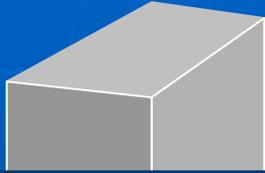
Caltrans-PEER Seismic Research Seminar
Sacramento, CA, USA

Rocking Seismic Isolation of Bridges Supported by Direct Foundations

June 8, 2009

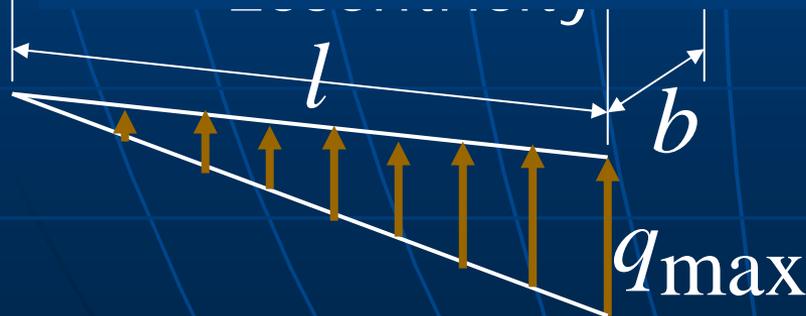
Kazuhiko Kawashima
Tokyo Institute of Technology

Requirements in the Overturning Based on Static Analysis



Eccentricity

- Size effect is not included in the conventional static analysis
- As long as the shape and mass density are the same, a foundation can overturn in the conventional static analysis no matter how the foundation is extremely large



Bearing Capacity ^{+ seismic}

$$q_{\max} = \frac{V}{lb} + \frac{6M_B}{lb^2} < q_a$$

Akashi Strait Bridge

The World Longest Bridge



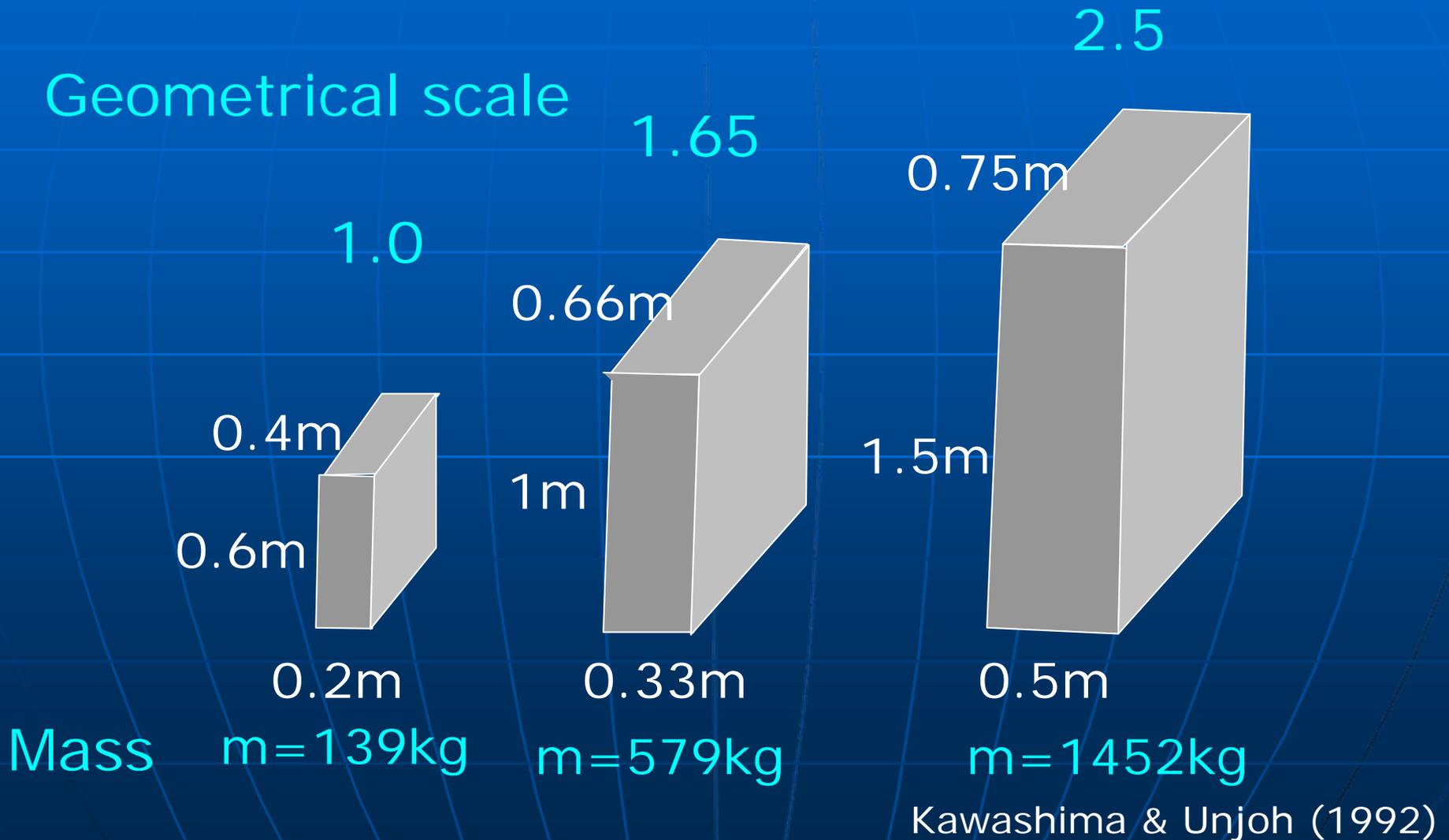
In the static design, overturning was the major factor for sizing of those foundations based on the conventional analysis



Does such a 100m tall foundation
overturn under seismic excitation??

- Mass
- Natural period
- Frequency content of a ground motion

Shake Table Experiment on the Effect of Size & Mass for Overturning of Rigid Foundations



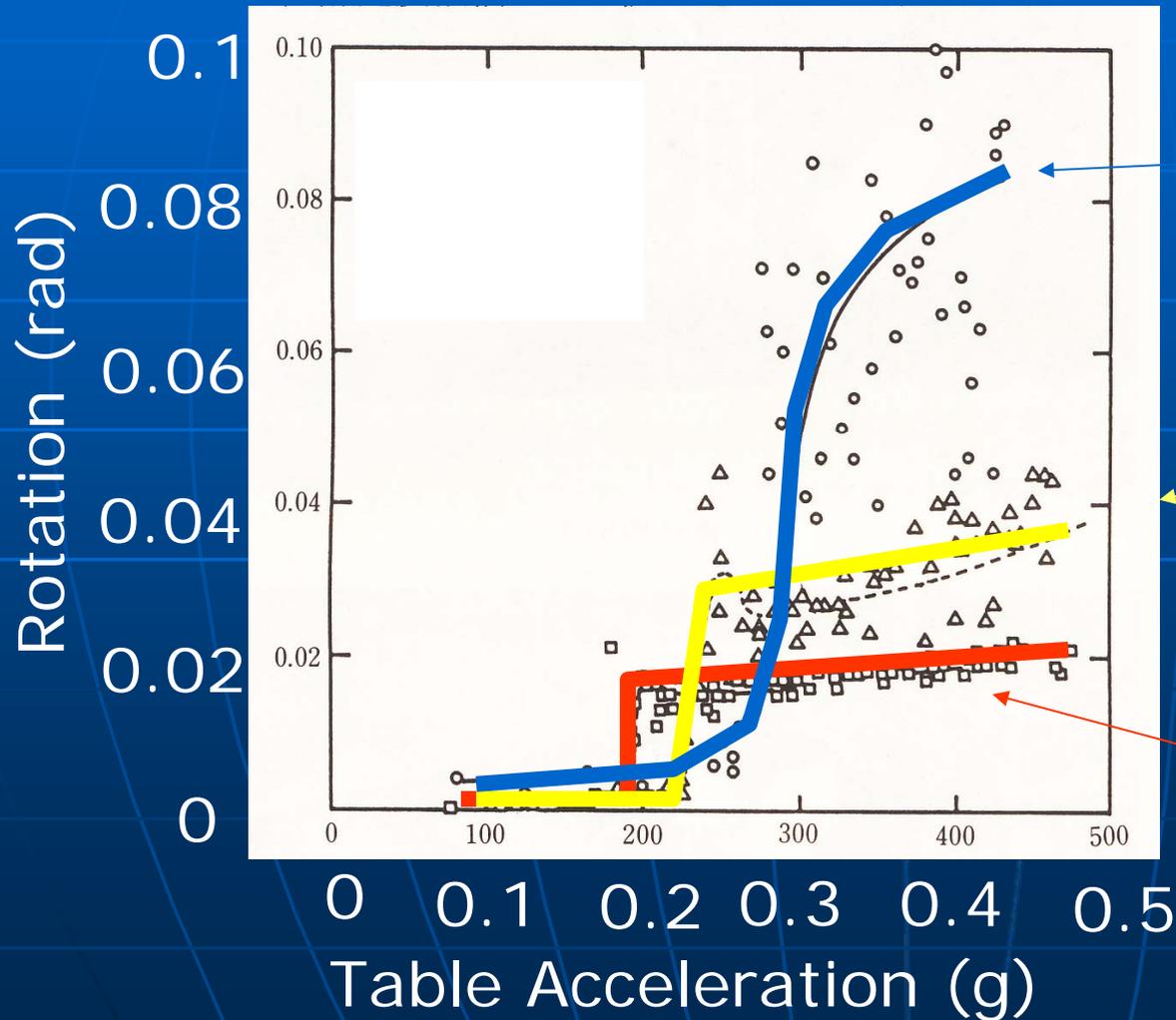
Shake Table Experiment on the Effect of Size & Mass for Overturning of Rigid Foundations

Public Works Research Institute



Kawashima & Unjoh (1992)

How does the rocking of rigid foundations depend on the size?

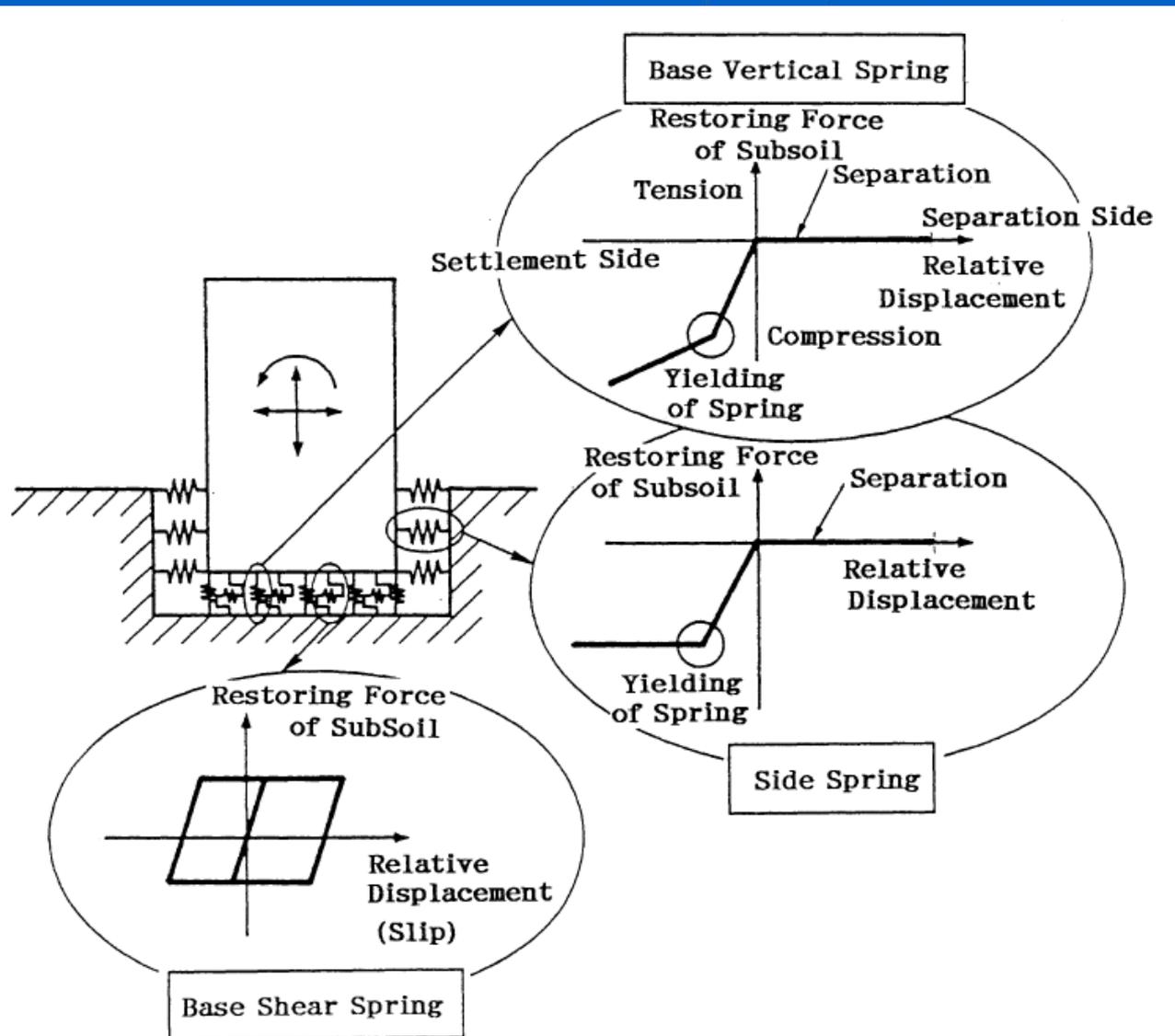


1.5m

1m

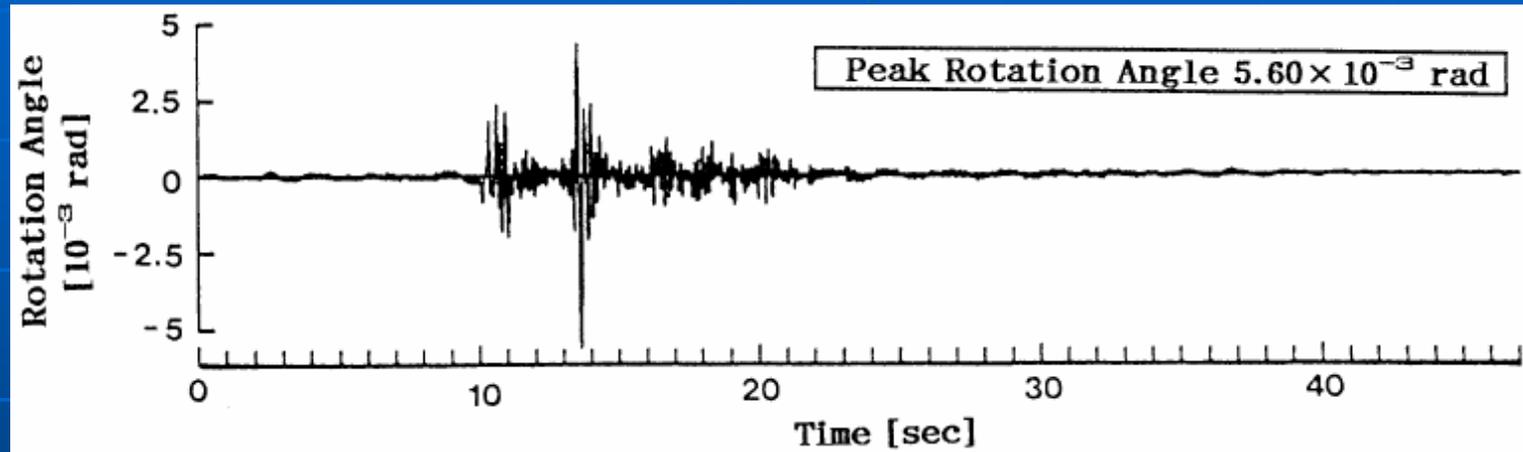
0.6m

Analytical Idealization for Rocking and Sliding Response of a Rigid Foundation

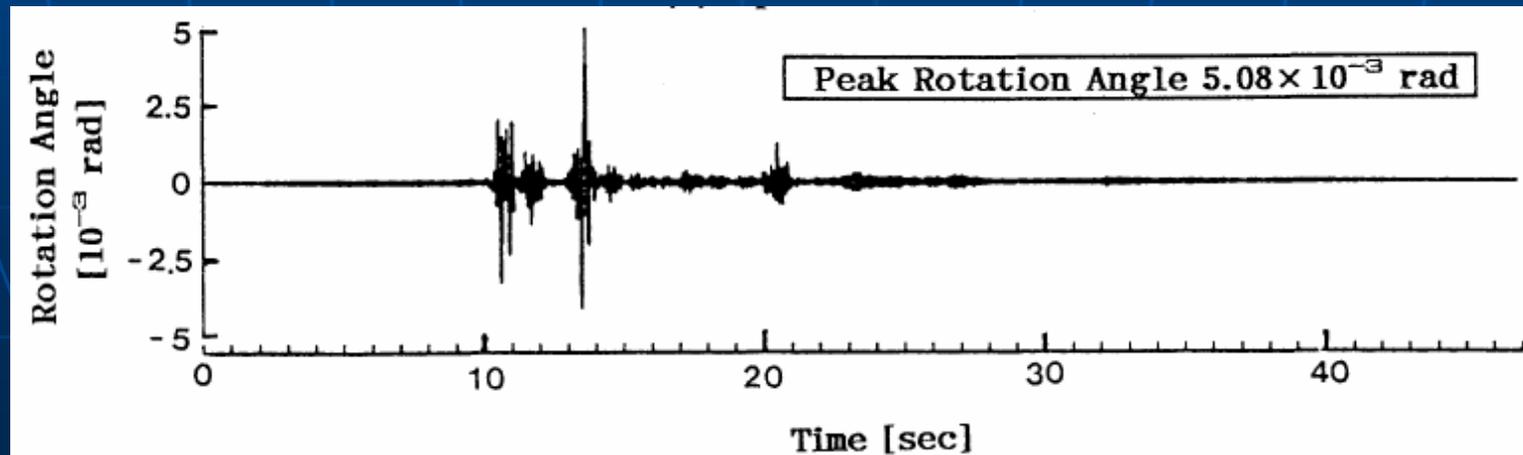


Analytical Correlation on the Rocking Response of a 1.5m Tall Rigid Foundation

Experimental



Analytical

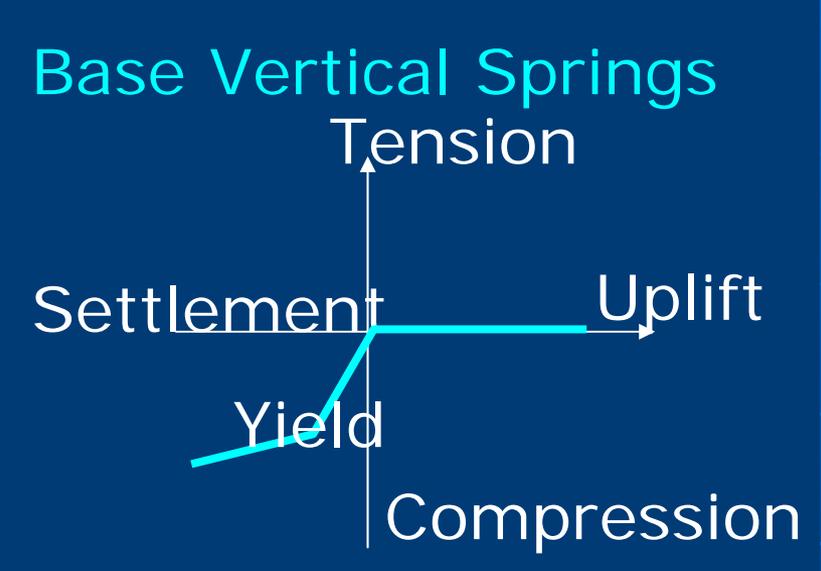
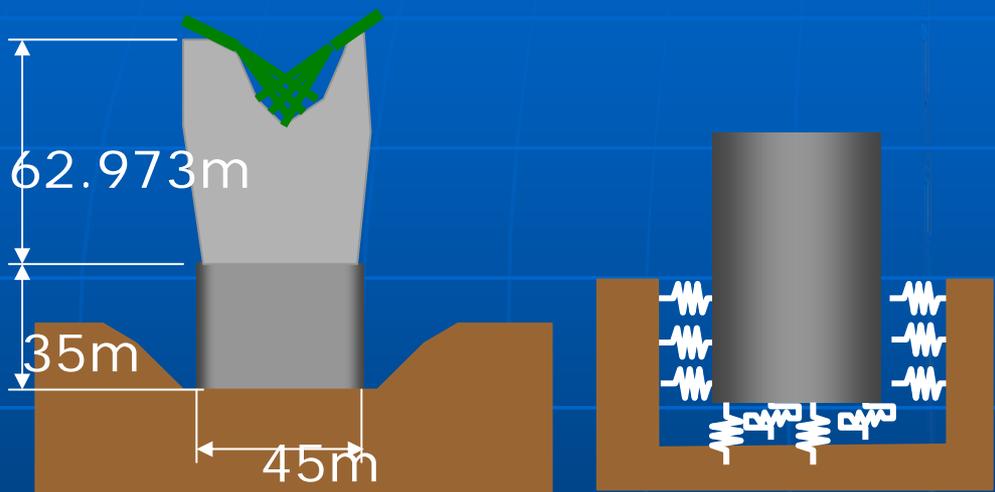


Seismic Response Analysis of Kurushima Straight Bridge

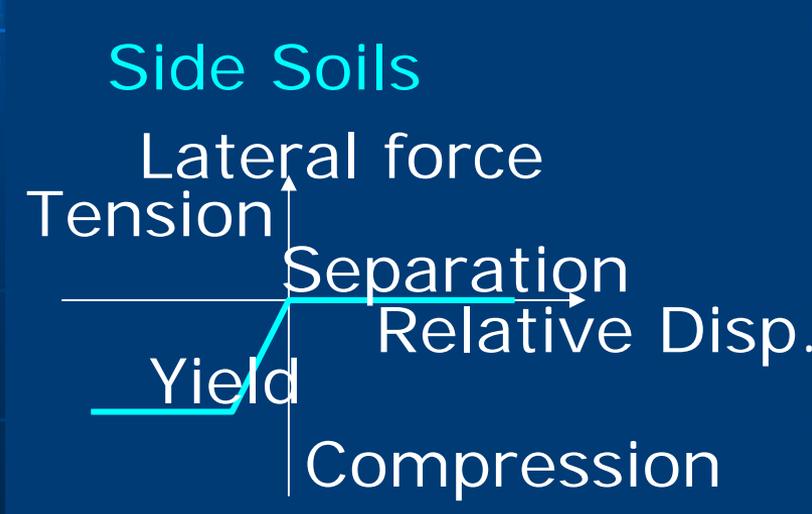
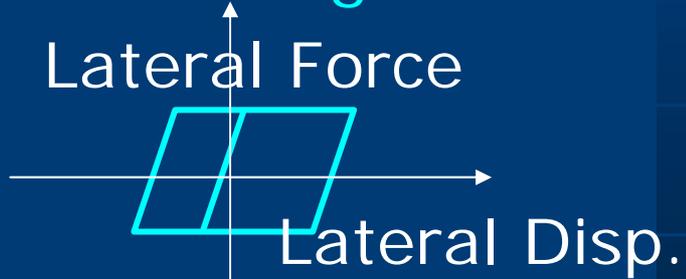


m | 48 | 23

Seismic Response Analysis of Anchorages, Towers and Superstructure System of Kurushima Straight Bridge



Lateral Sliding at Base

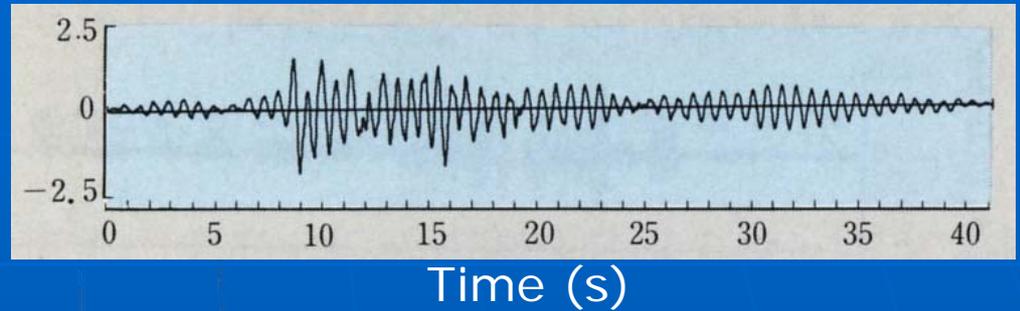


Peak Responses

Response displacement at the top of a tower

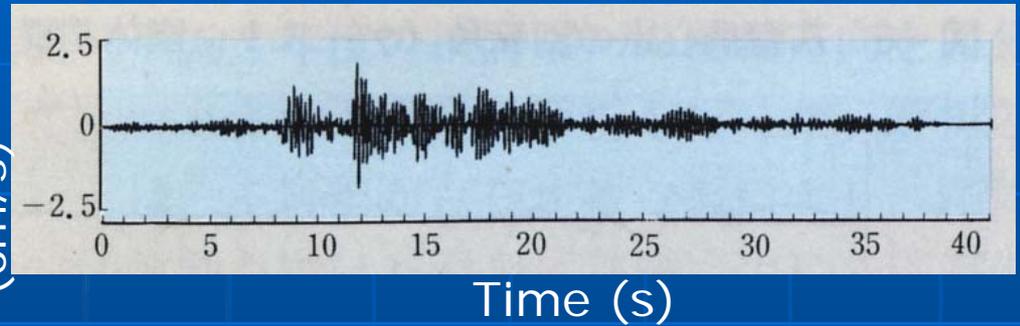
Displacement (cm)

Soft rock does not yield



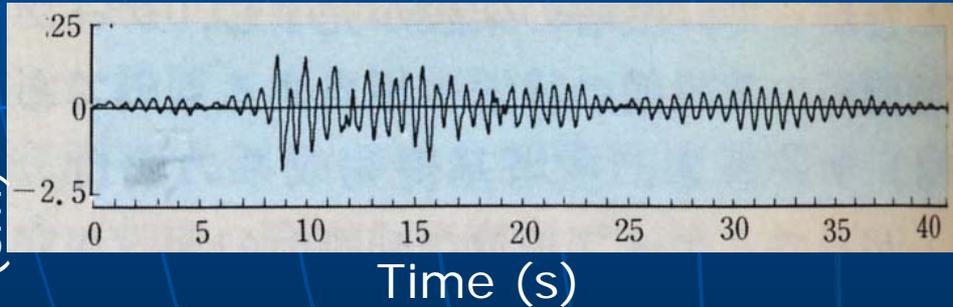
Response rotation of an anchorage

Acceleration (cm/s)



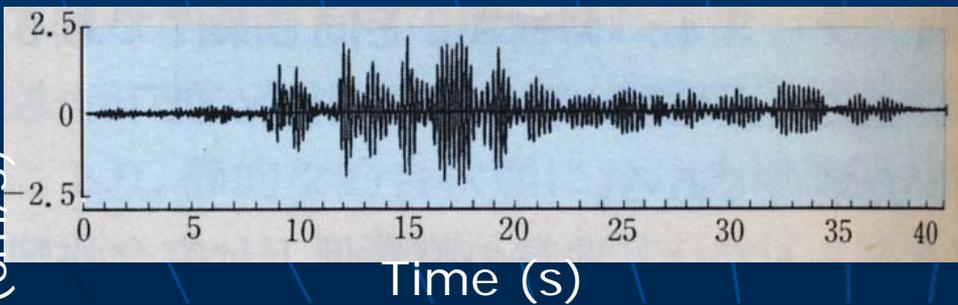
Soft rock yields

Displacement (cm)



Response displacement at the top of a tower

Acceleration (cm/s)



Response rotation of an anchorage

How frequently does the anchorage uplift?

Soft rock does not yield

Soft rock yields

Anchorage

Anchorage

Left edge

Right edge

Left edge

Right edge

Peak uplift
0.114m

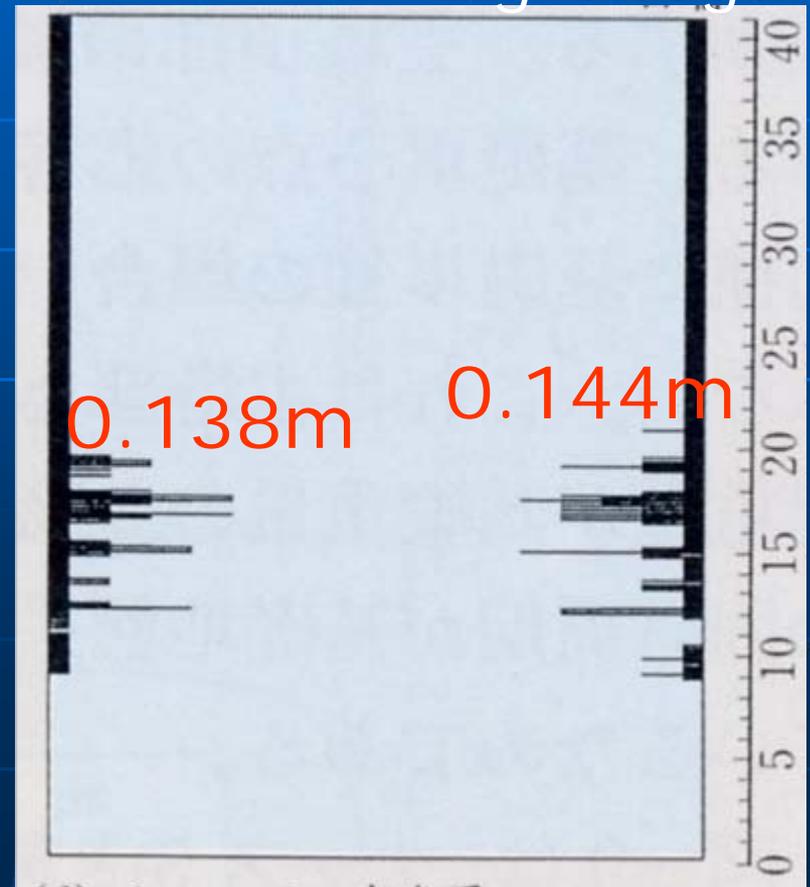
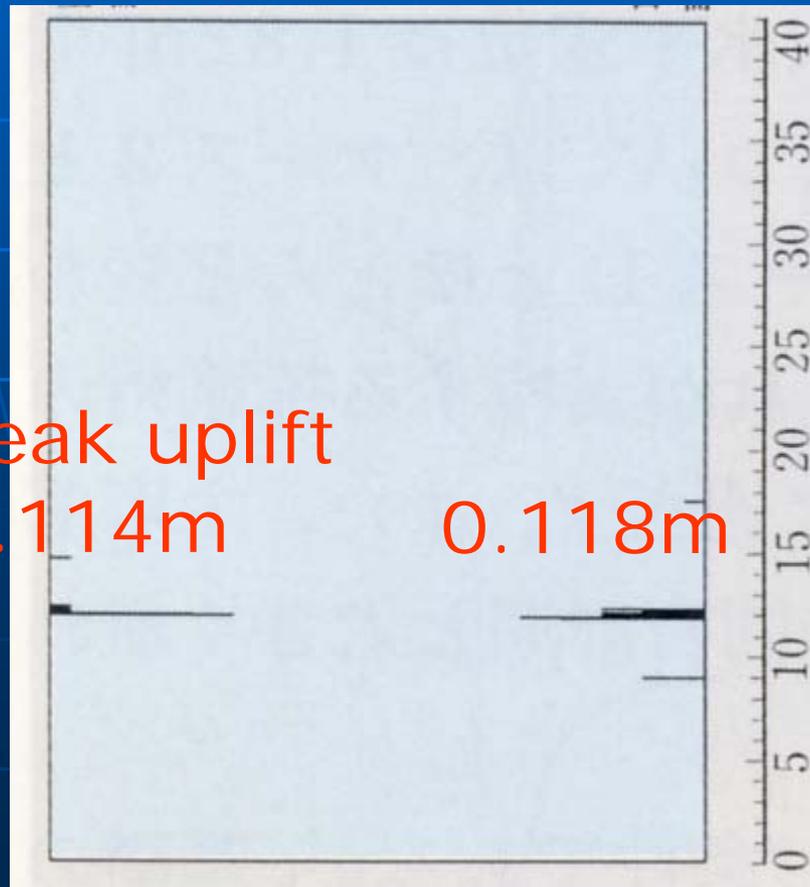
0.118m

Time (s)

0.138m

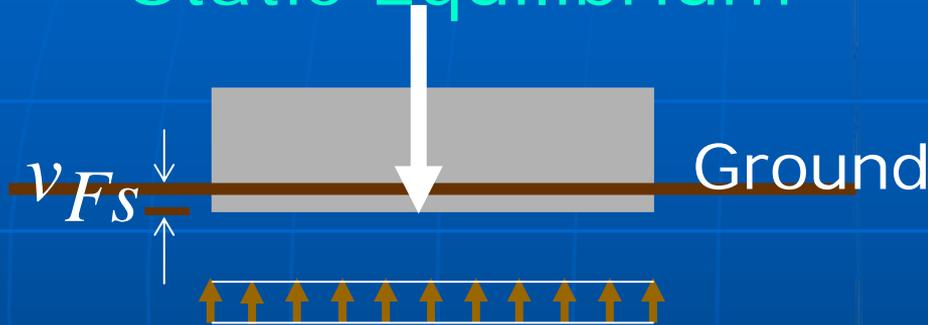
0.144m

Time (s)

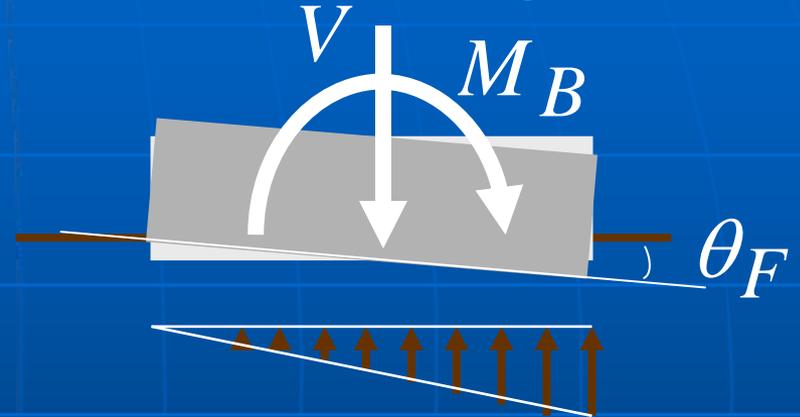


Uplift & separation and contact of a foundation with the underlying ground

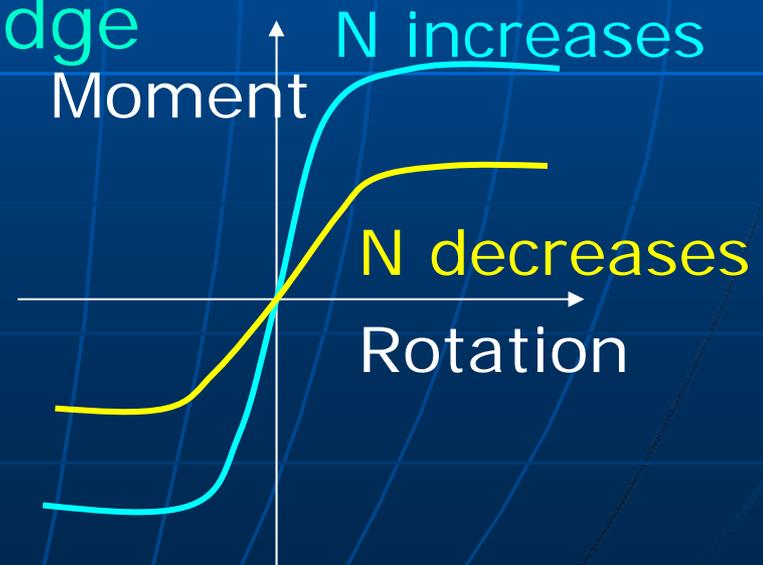
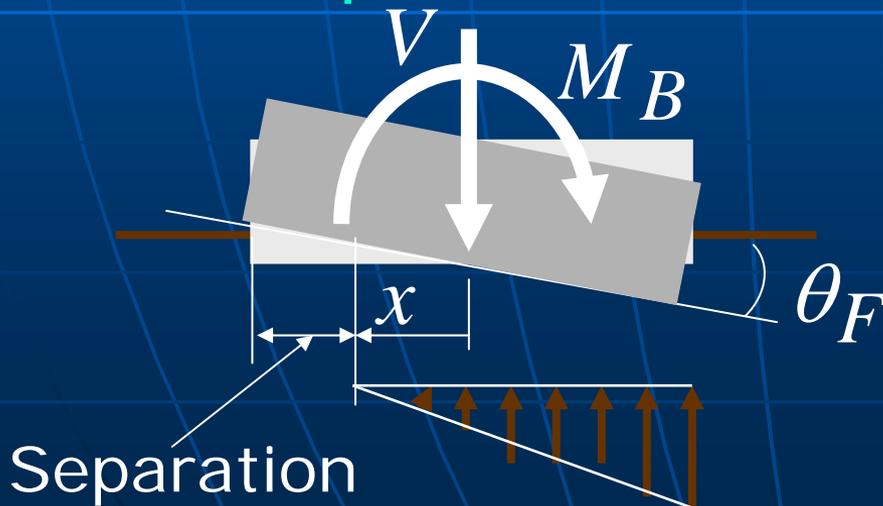
Static Equilibrium



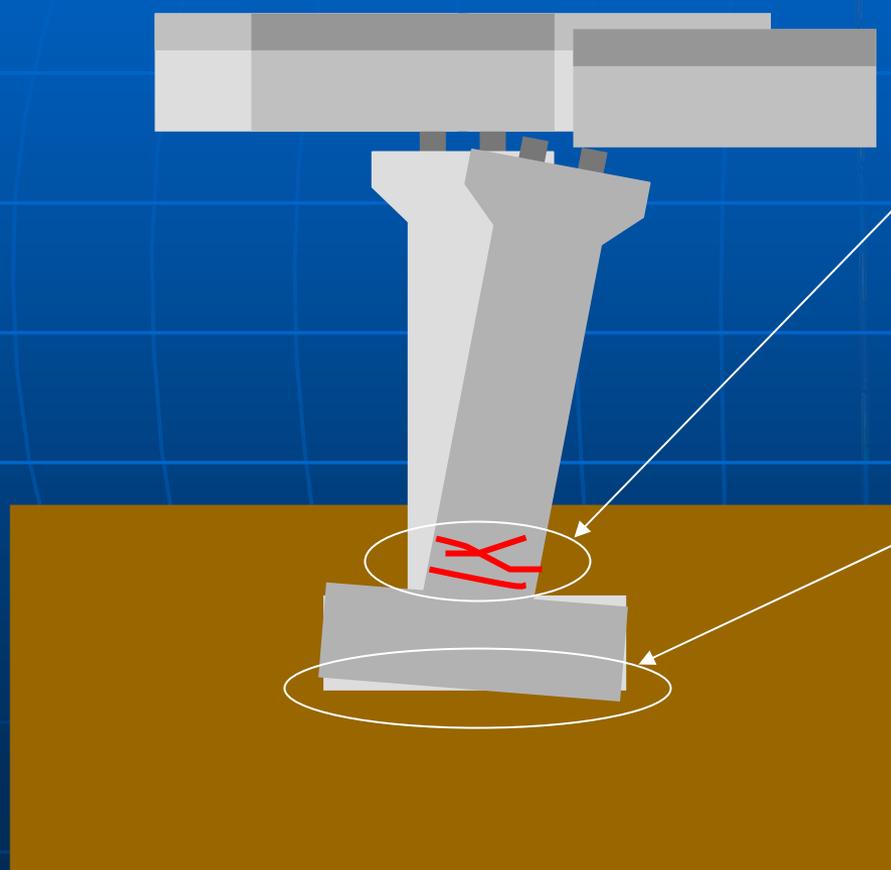
Start to Uplift



Uplifted at the Left Edge



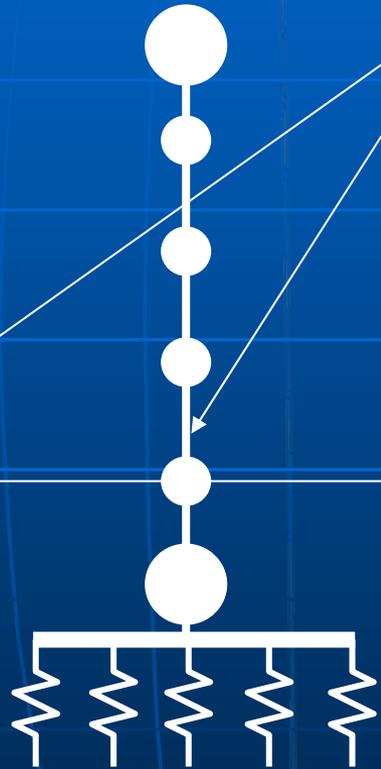
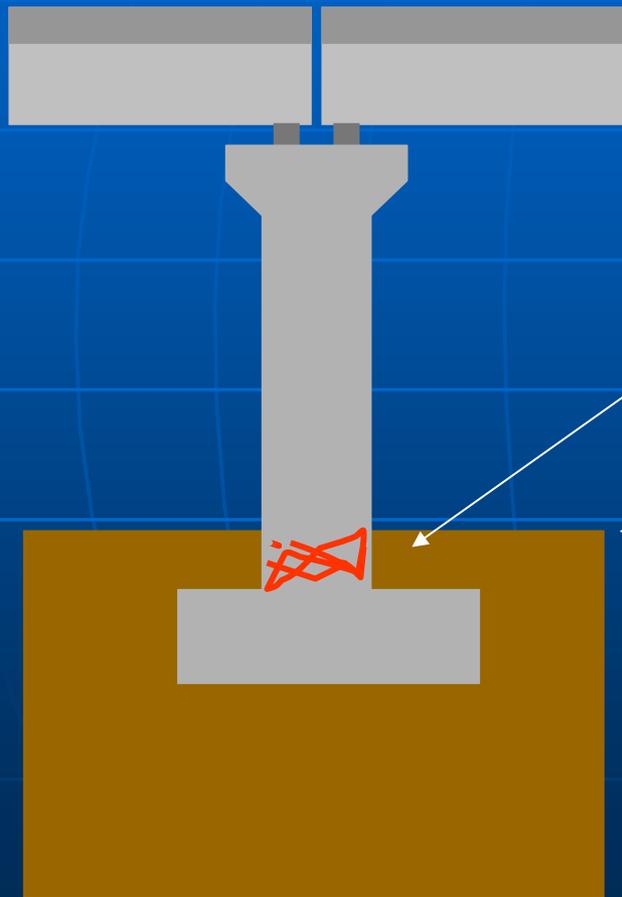
Nonlinear Interaction between 2 plastic hinges; (1) Column Plastic Hinge and (2) Foundation Nonlinear Behavior



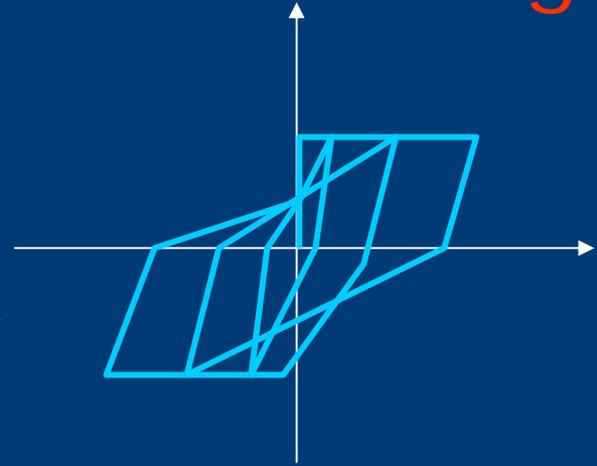
Plastic deformation
of a column

Rocking response
of a foundation

Analytical Idealization



Plastic Hinge



Uplift of Foundation

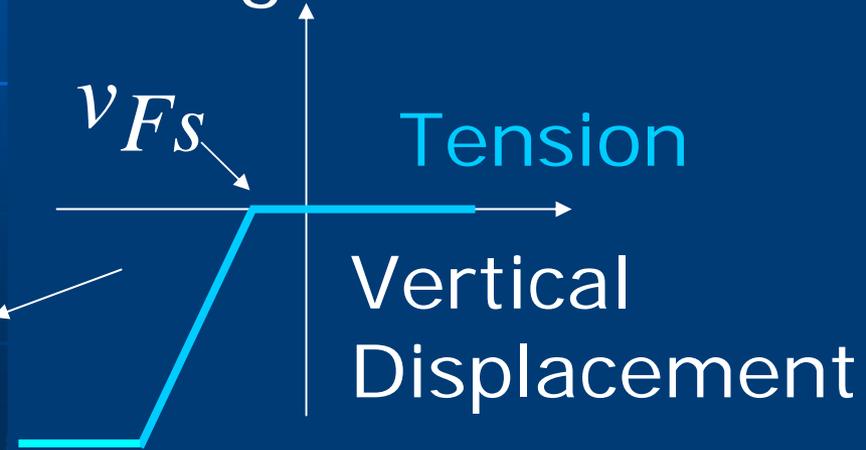
Subgrade Reaction

vFs

Tension

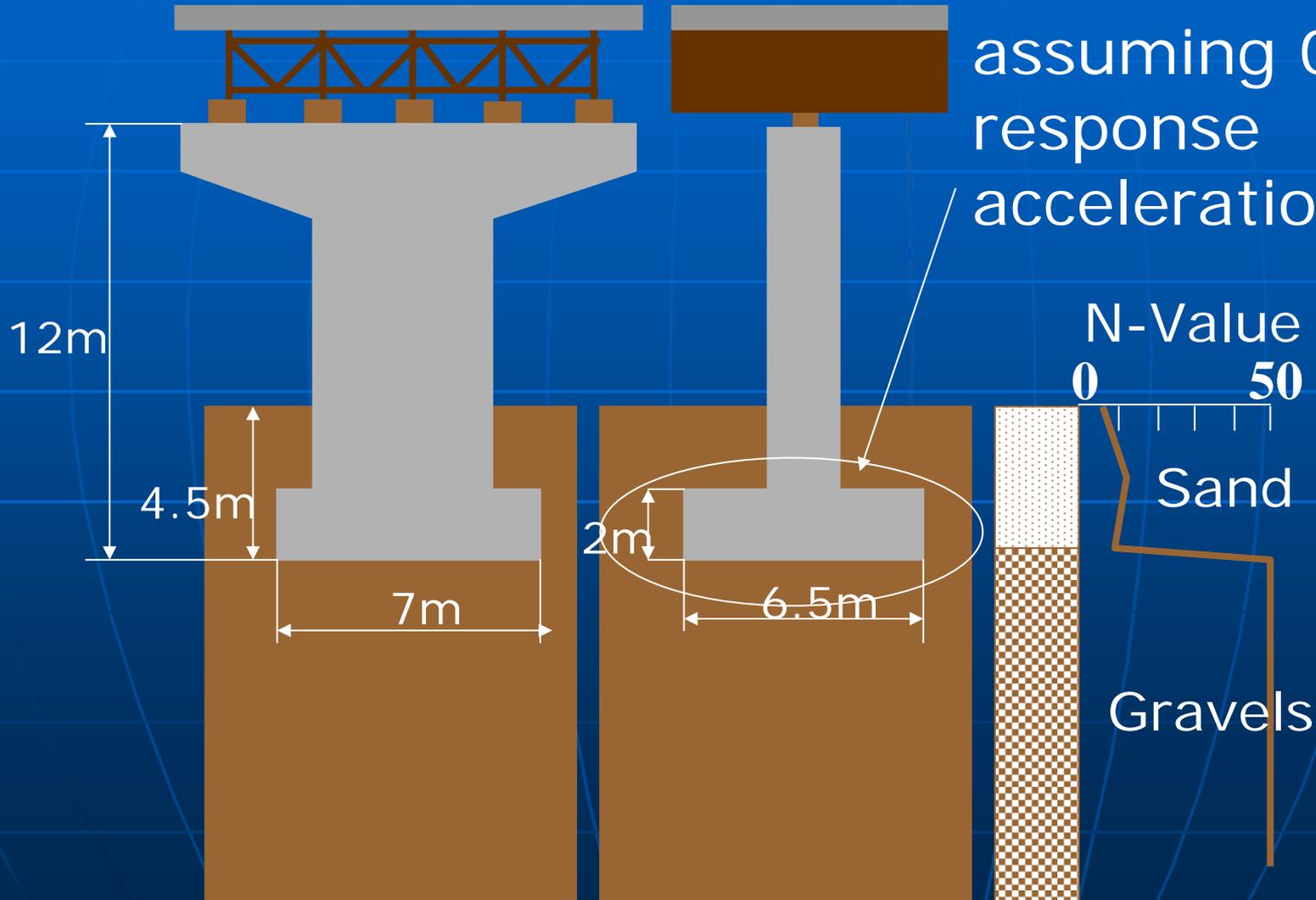
Vertical Displacement

Compression



Bridge Analyzed

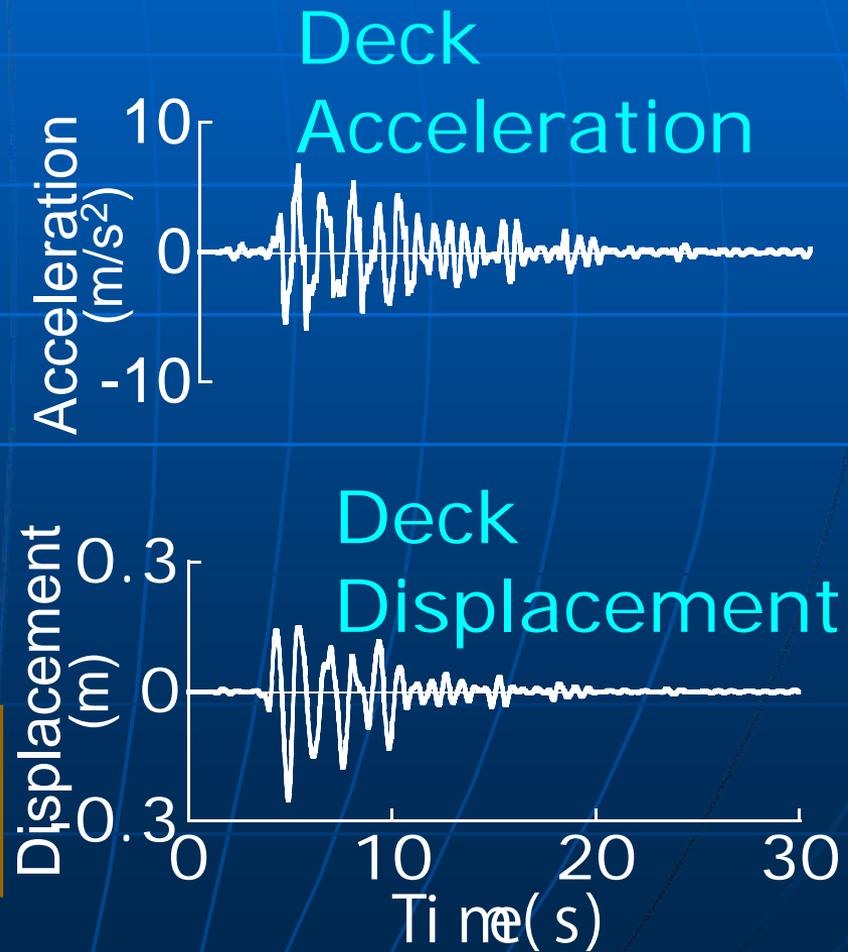
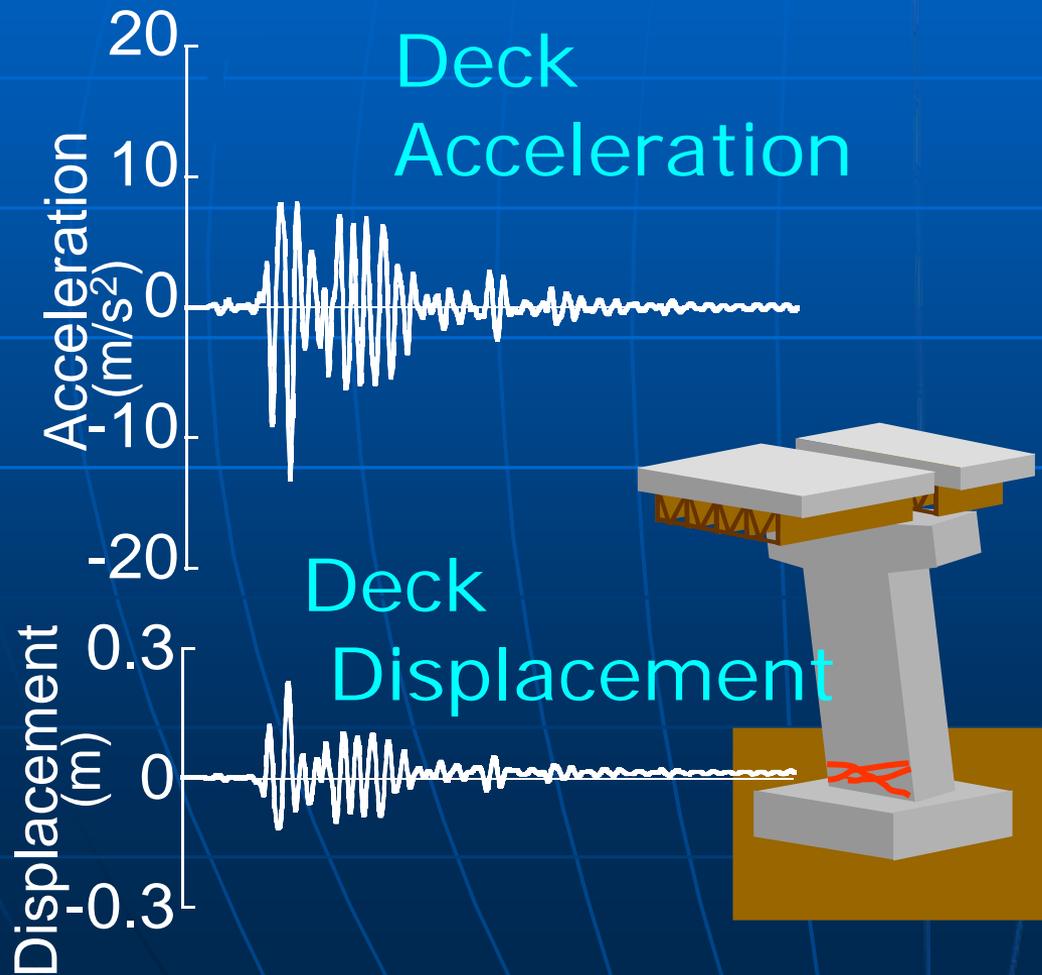
Designed based on the static analysis assuming 0.2g response acceleration



Deck Response under Longitudinal and Vertical Excitation

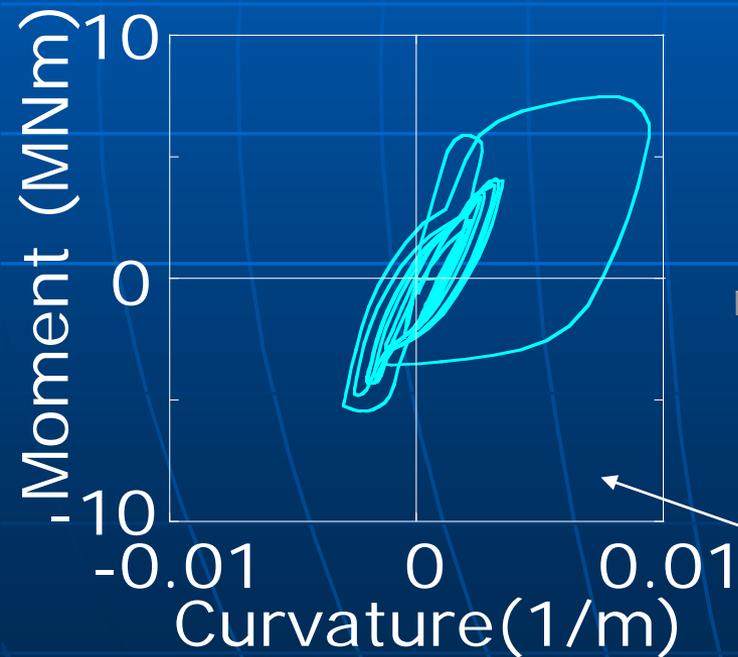
Conventional Analysis

Rocking Isolation

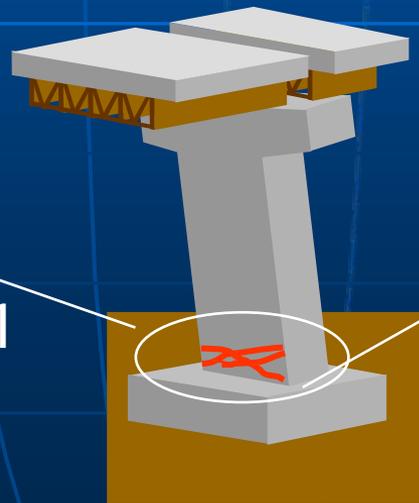
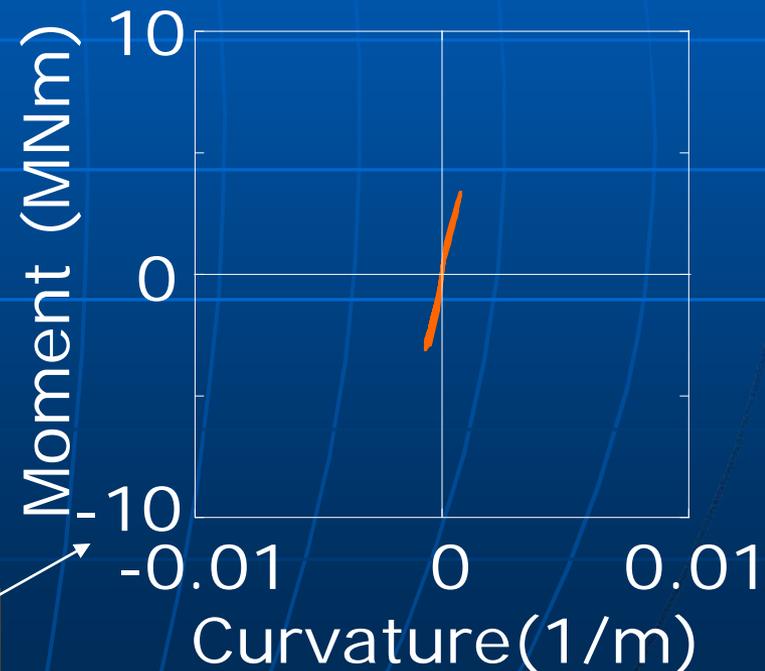


Column Curvature at the Plastic Hinge under Longitudinal and Vertical Excitation

Conventional Analysis



Rocking Isolation



How Large Uplift occur at the Footing?

Conventional Analysis

Rocking Isolation

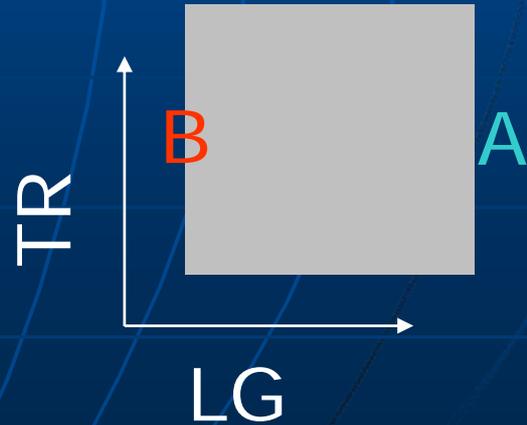
Uplift



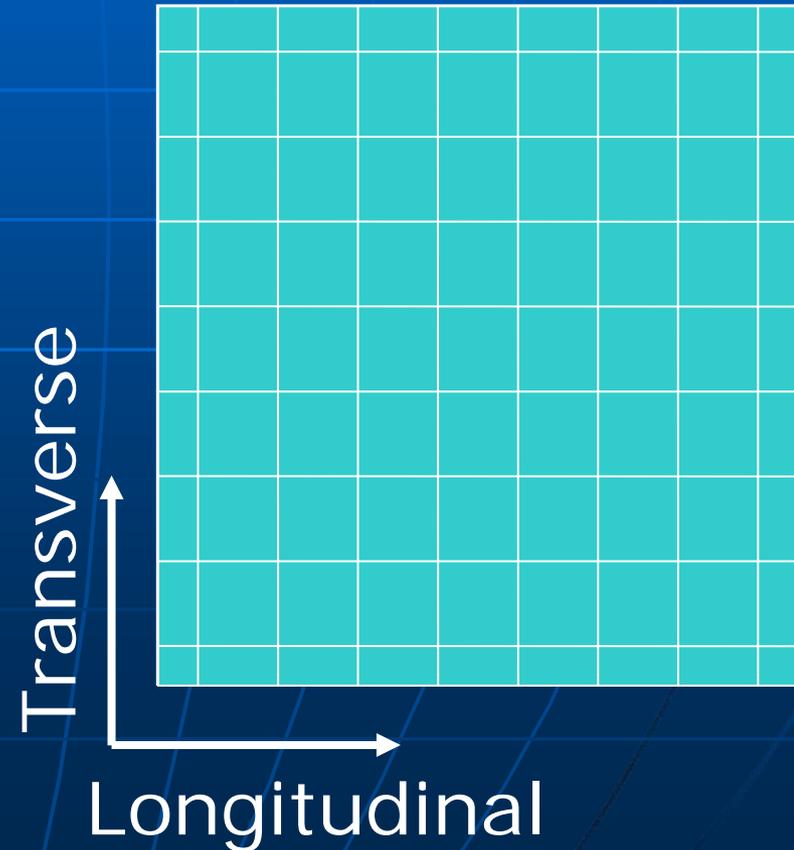
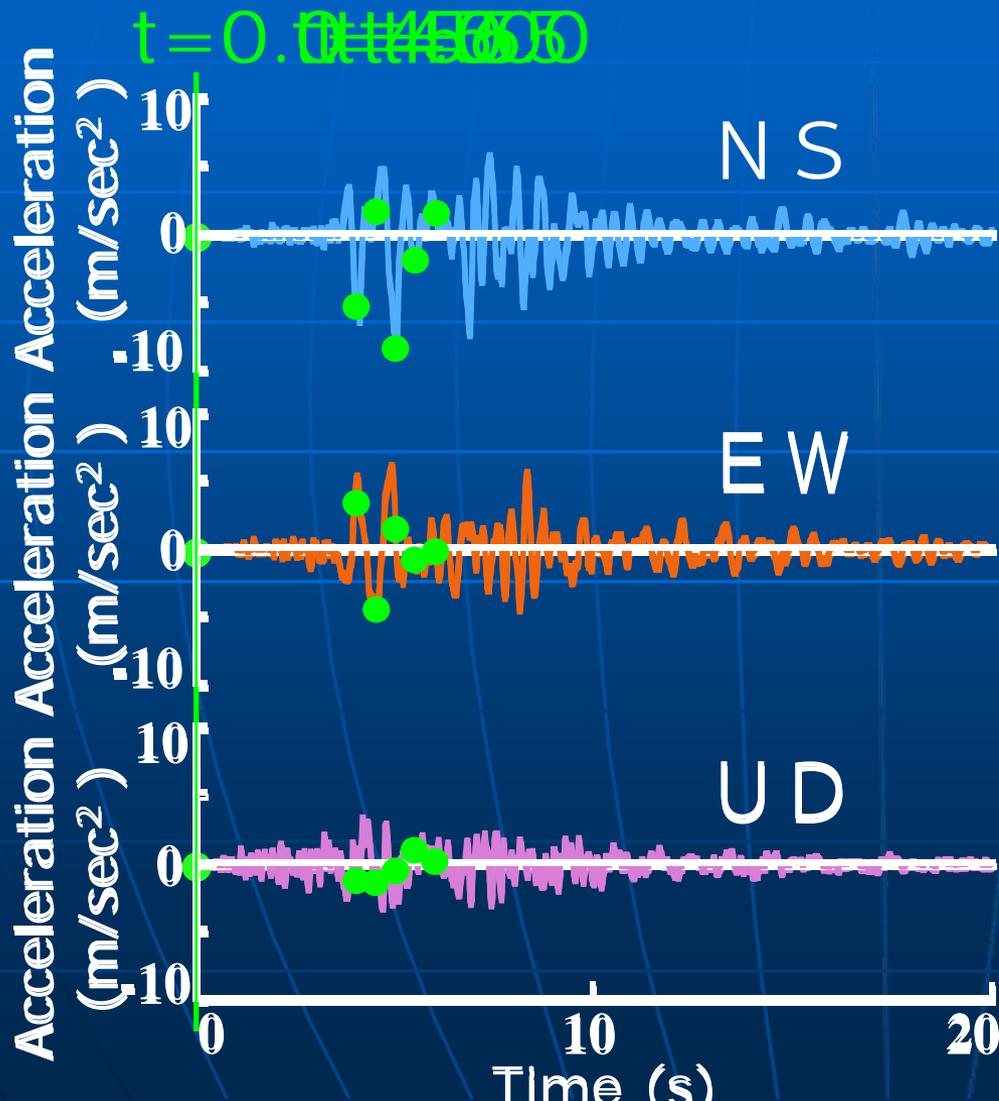
Reaction of Underlying Ground



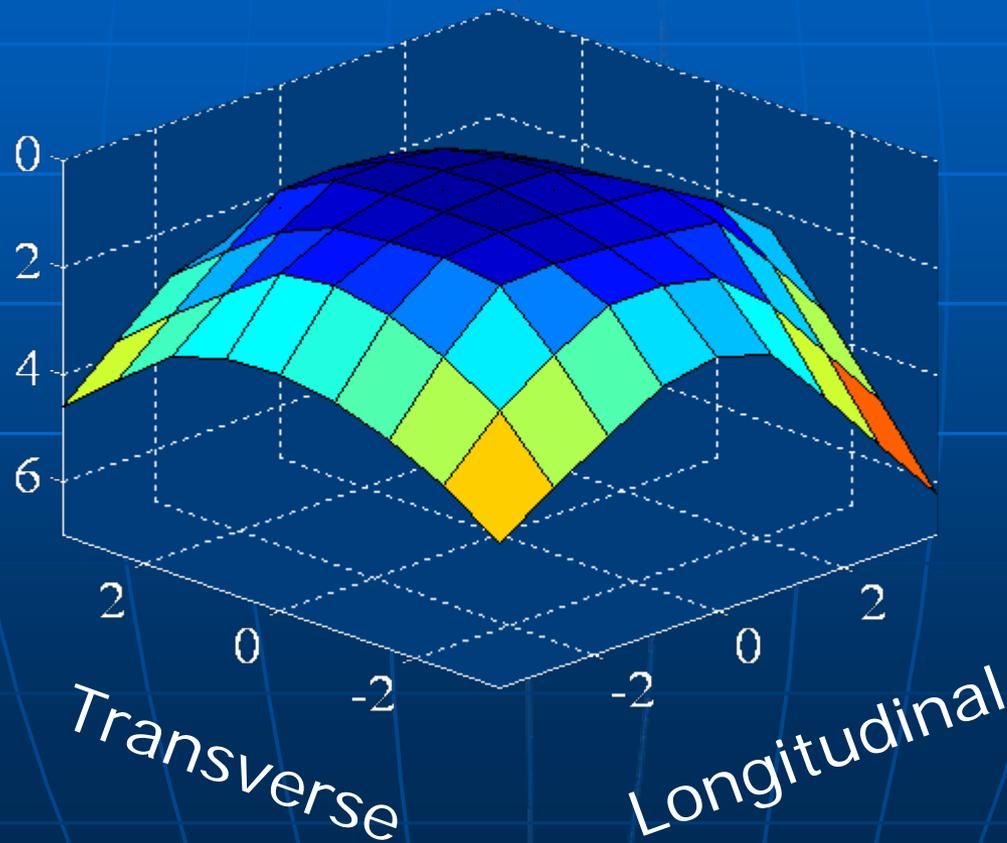
Footing



Uplift of the Footing from the Underlying Ground



Reaction Force of the Underlying Ground at Corners increases under Bilateral Excitation



Seismic Response under 3 Directional Excitation

Bi-Lateral

Deck

Acceleration



Tri-Directional

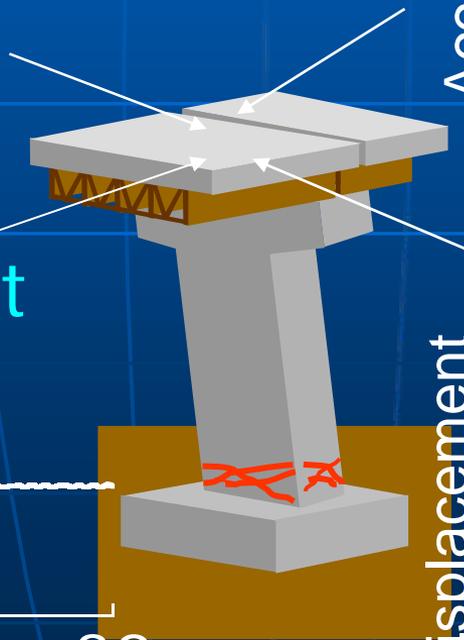
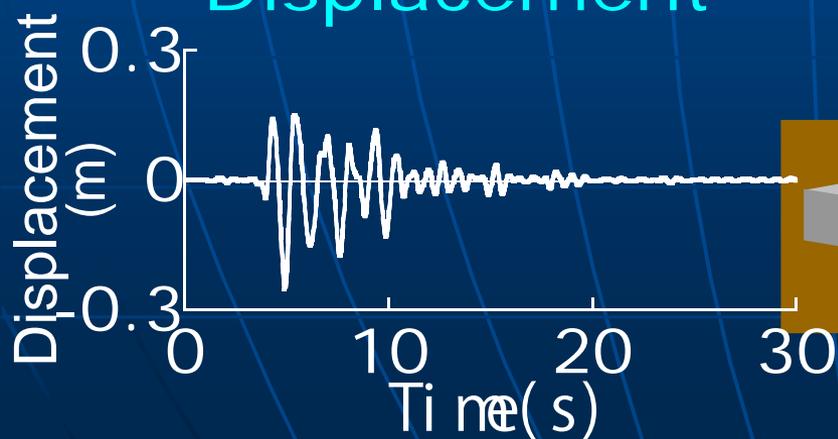
Deck

Acceleration



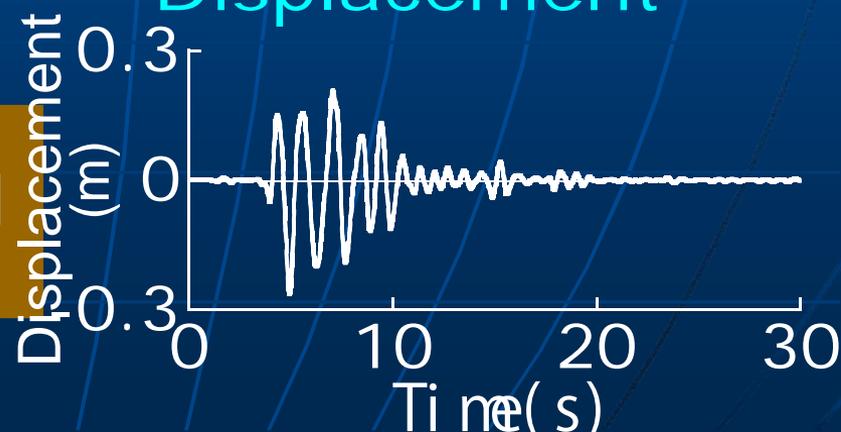
Deck

Displacement



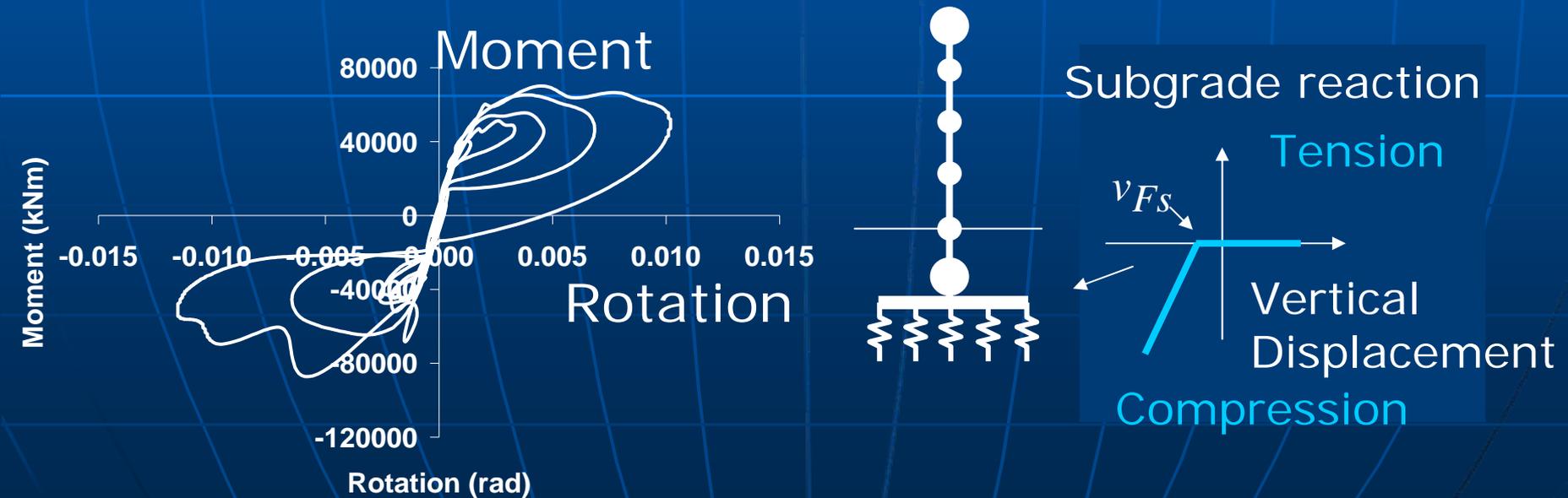
Deck

Displacement



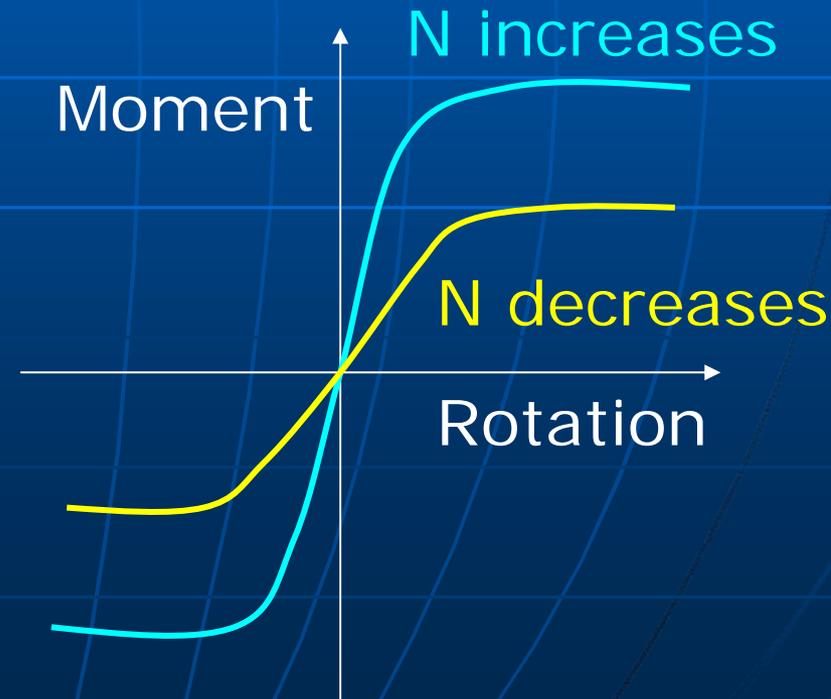
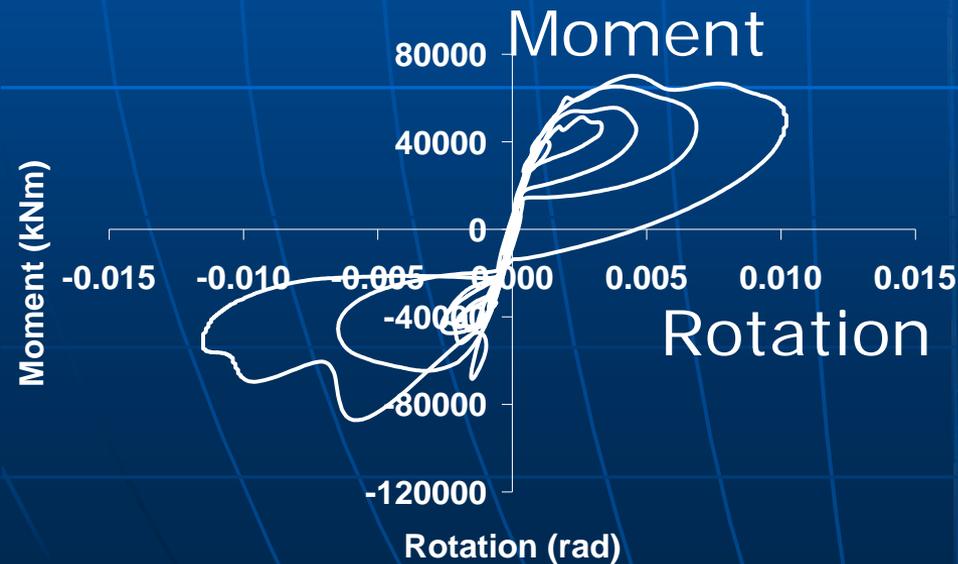
Why do we have an isolation effect by the foundation rocking no matter how we assume the elastic soil spring?

- There is no energy dissipation in the soil spring if we assume the elastic behavior
- Amplitude dependent period shift



Why do we have “hysteretic-like” moment vs. curvature relation no matter how we assume the elastic soil spring?

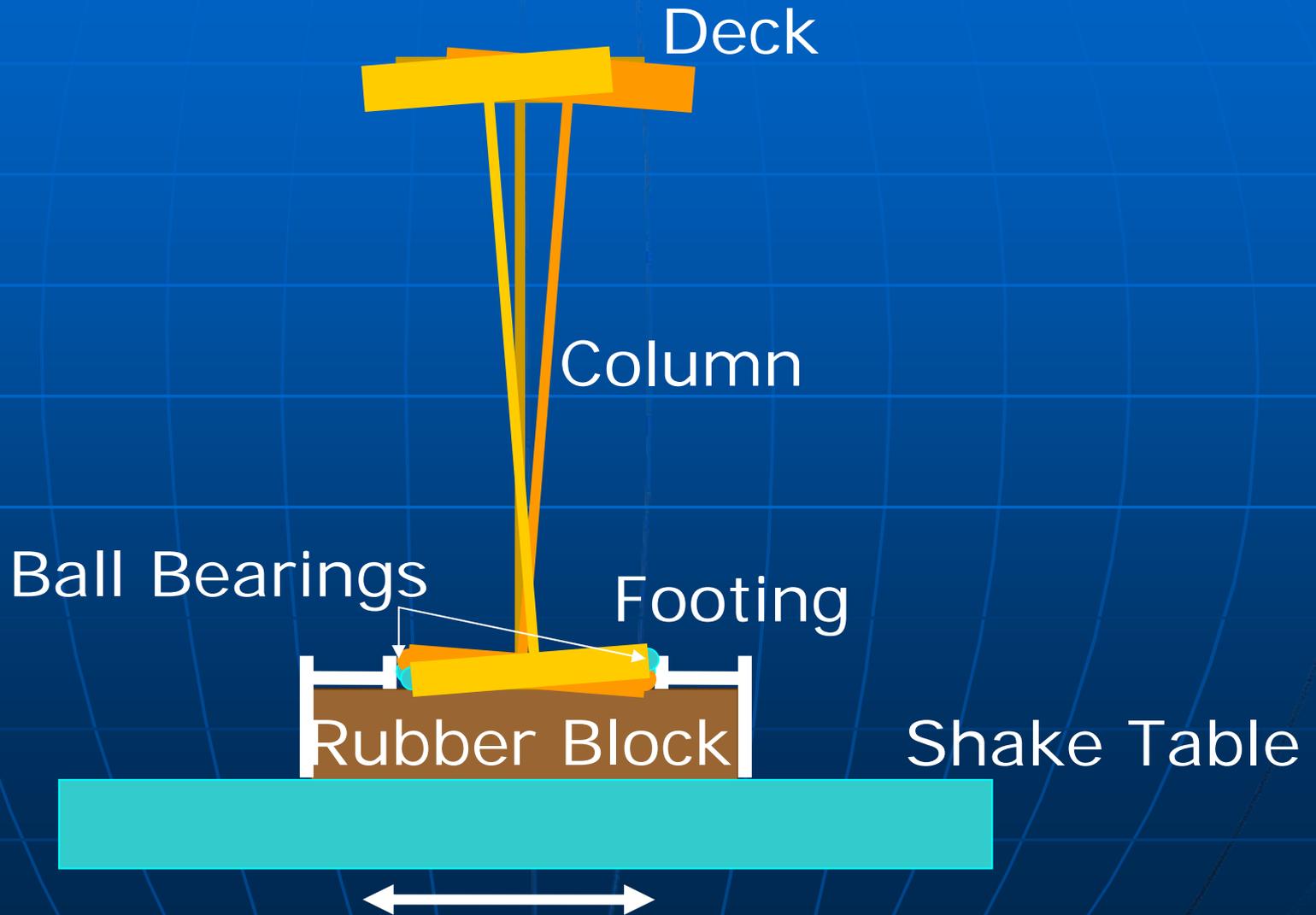
- Moment vs. rotation relation depends on the vertical force, and this results in “hysteretic-like” relation under variation of the vertical force.



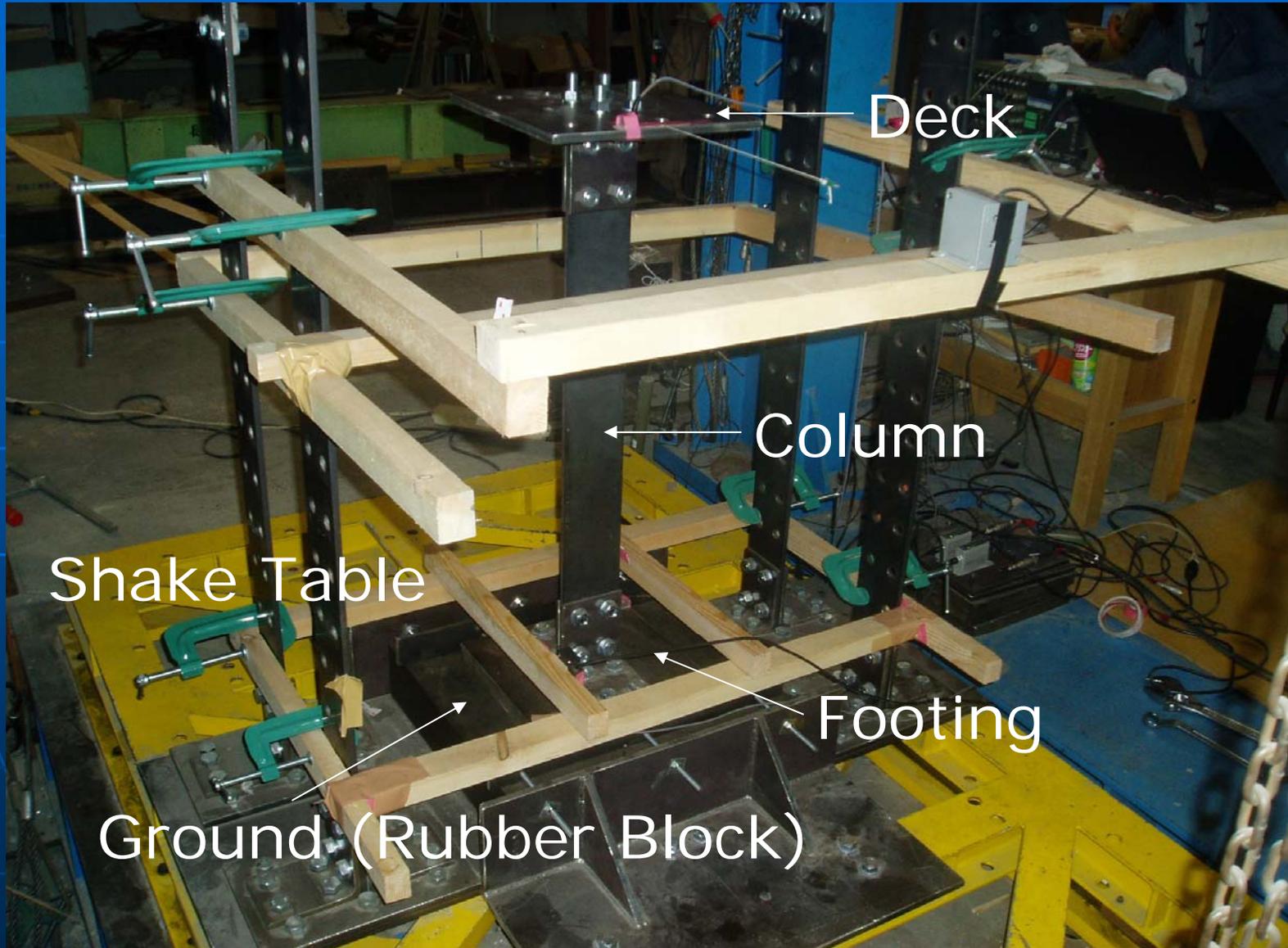
Factors which contribute to the energy dissipation of a foundation during rocking response

- Nonlinear soil behavior around a foundation (nonlinearity of soils, yield of bearing capacity, sliding & slip, etc)
- Energy dissipation due to pounding of the footing to the underlying ground
- Radiational damping
- ...

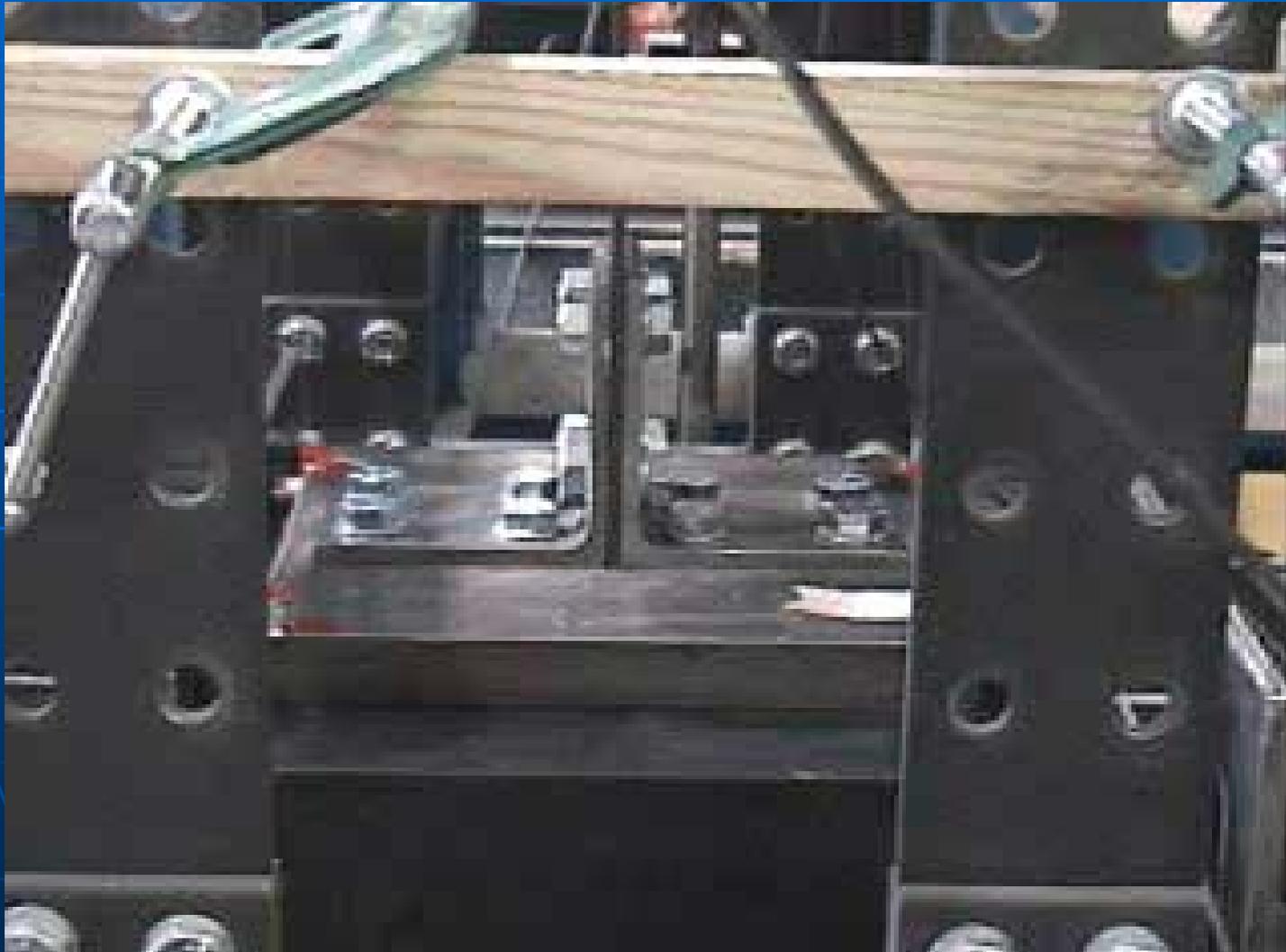
Verification of Seismic Rocking Isolation by a Shake Table Experiment



Experimental Model

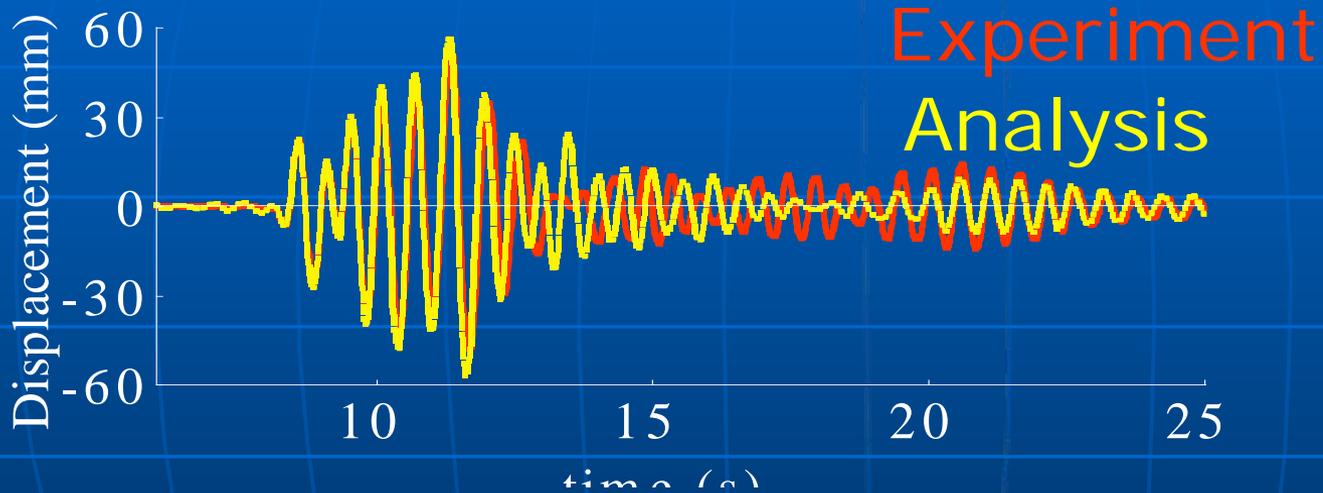


Excitation of Model Foundation under Niigata-Chuetsu Ground Motion

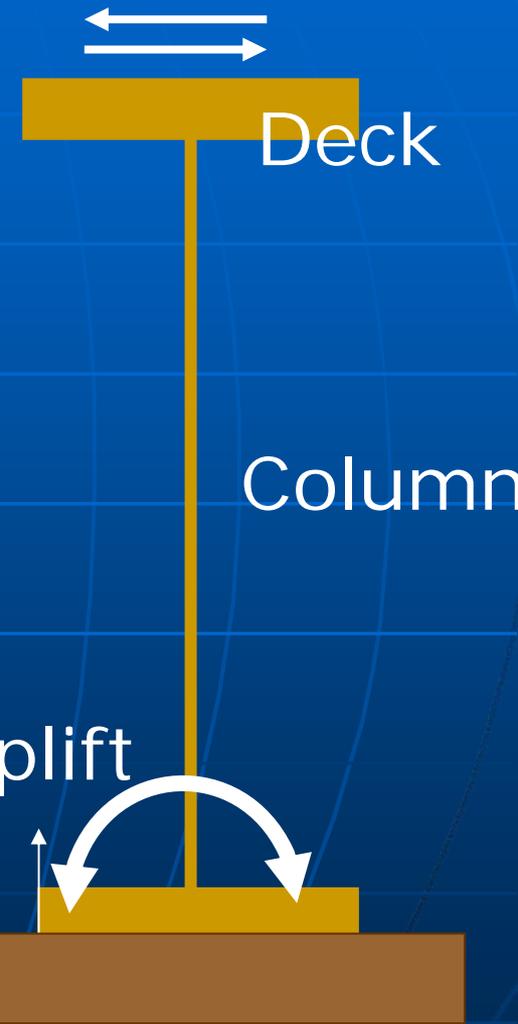
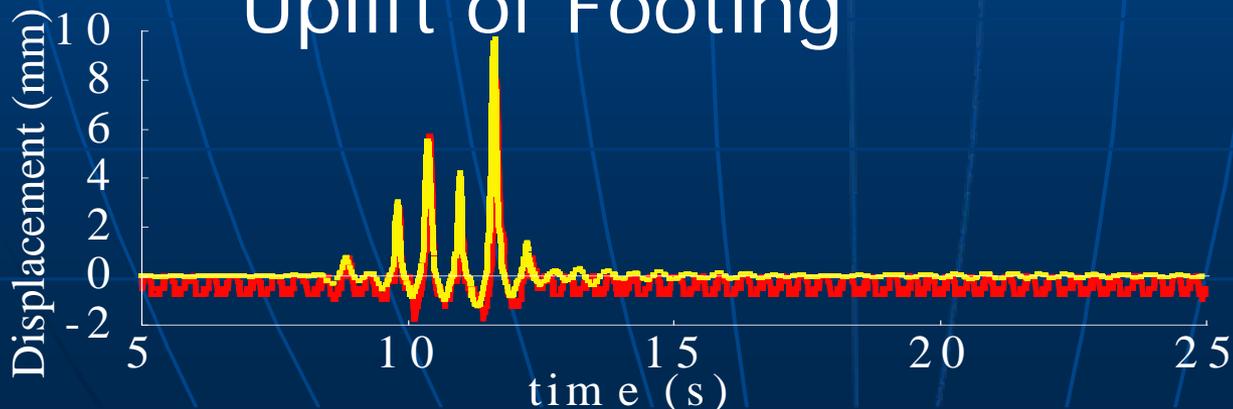


Correlation of the Experimental Response by Analysis

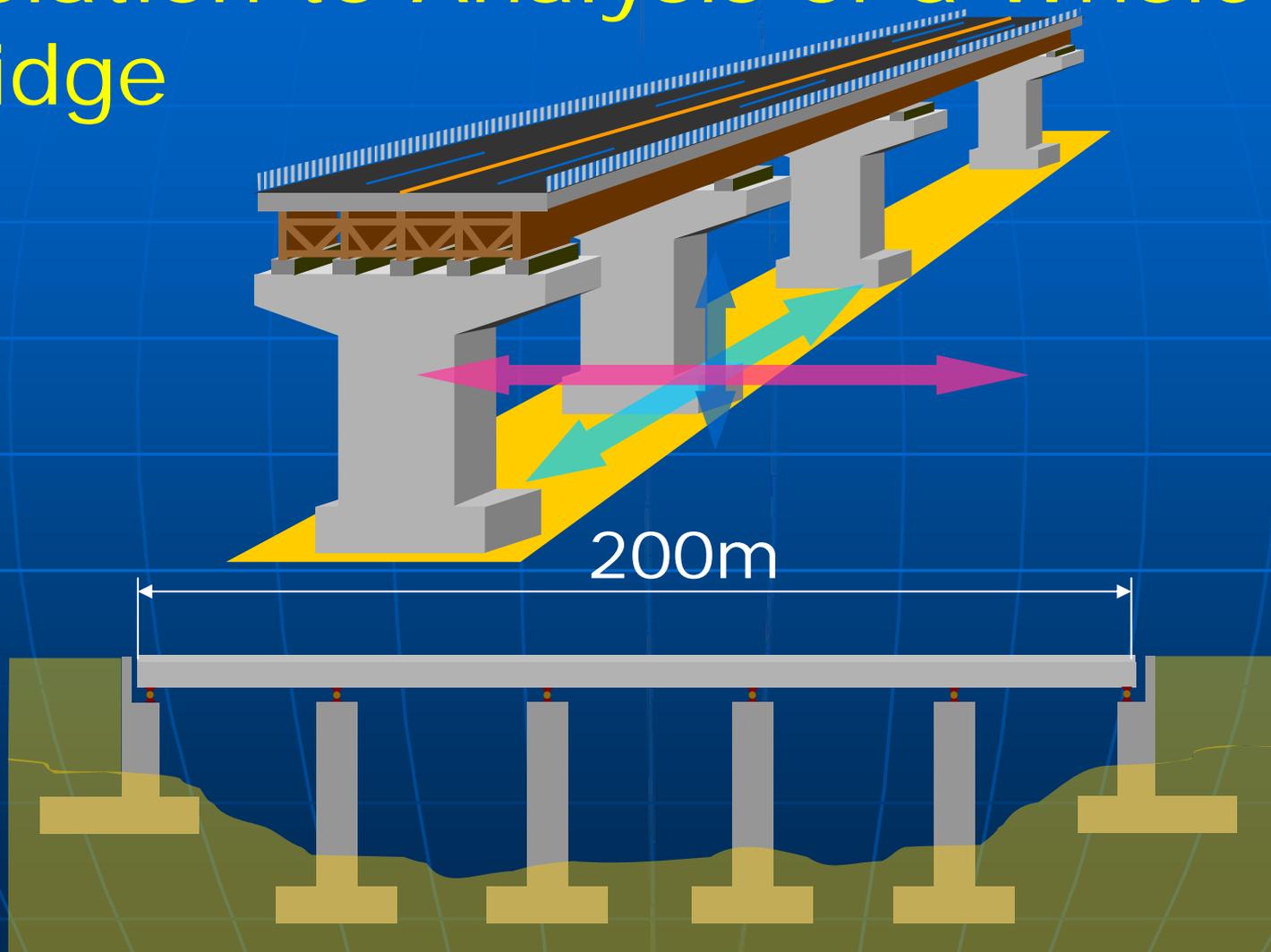
Deck Displacement



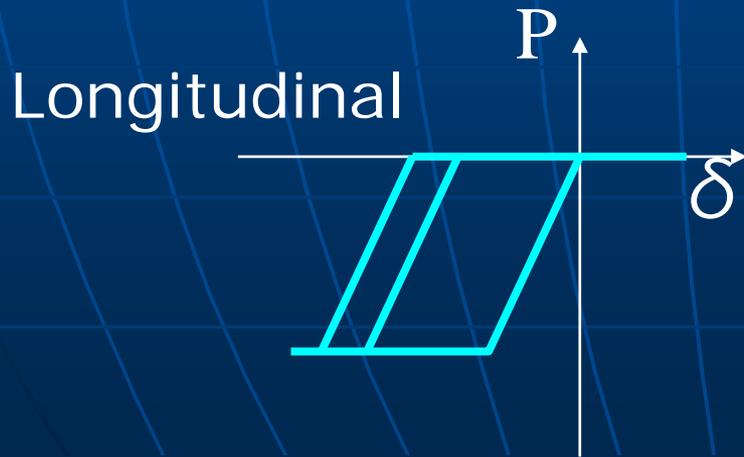
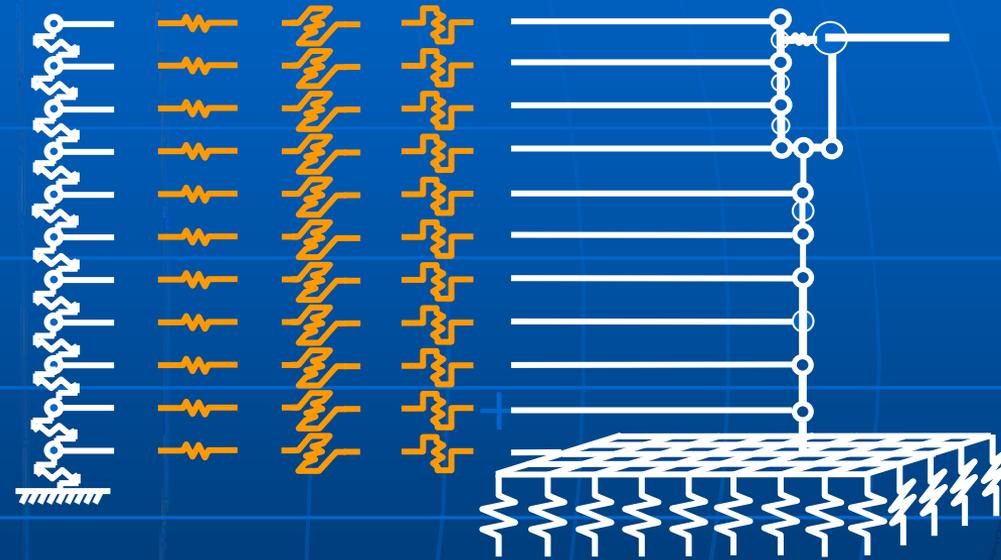
Uplift of Footing



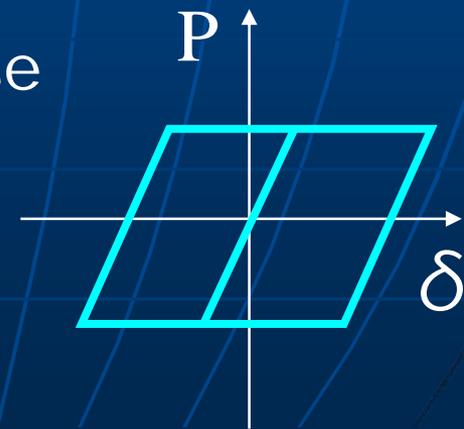
Implementation of Rocking Isolation to Analysis of a Whole Bridge



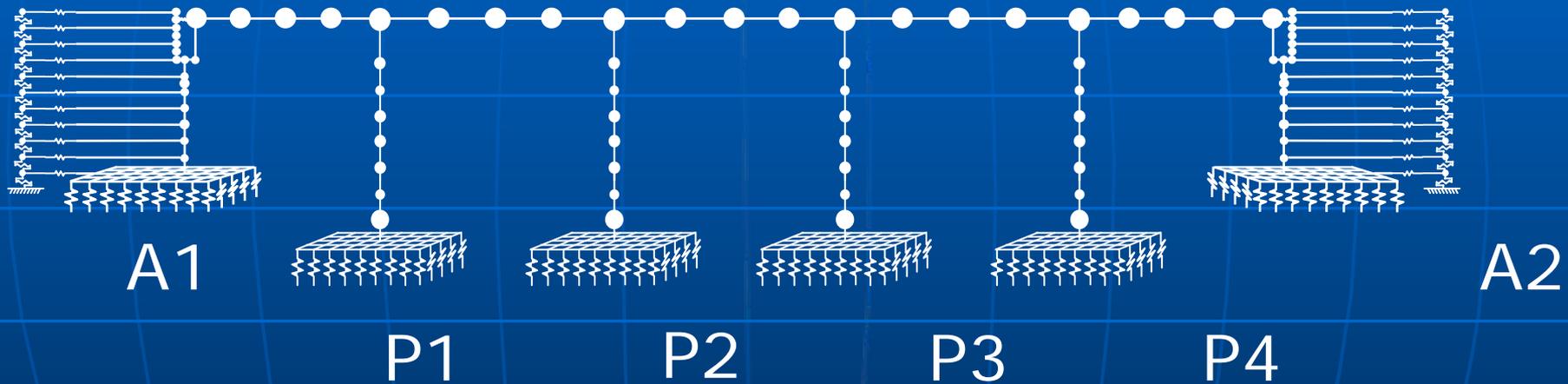
Idealization of Interaction between Abutment and Backsoils



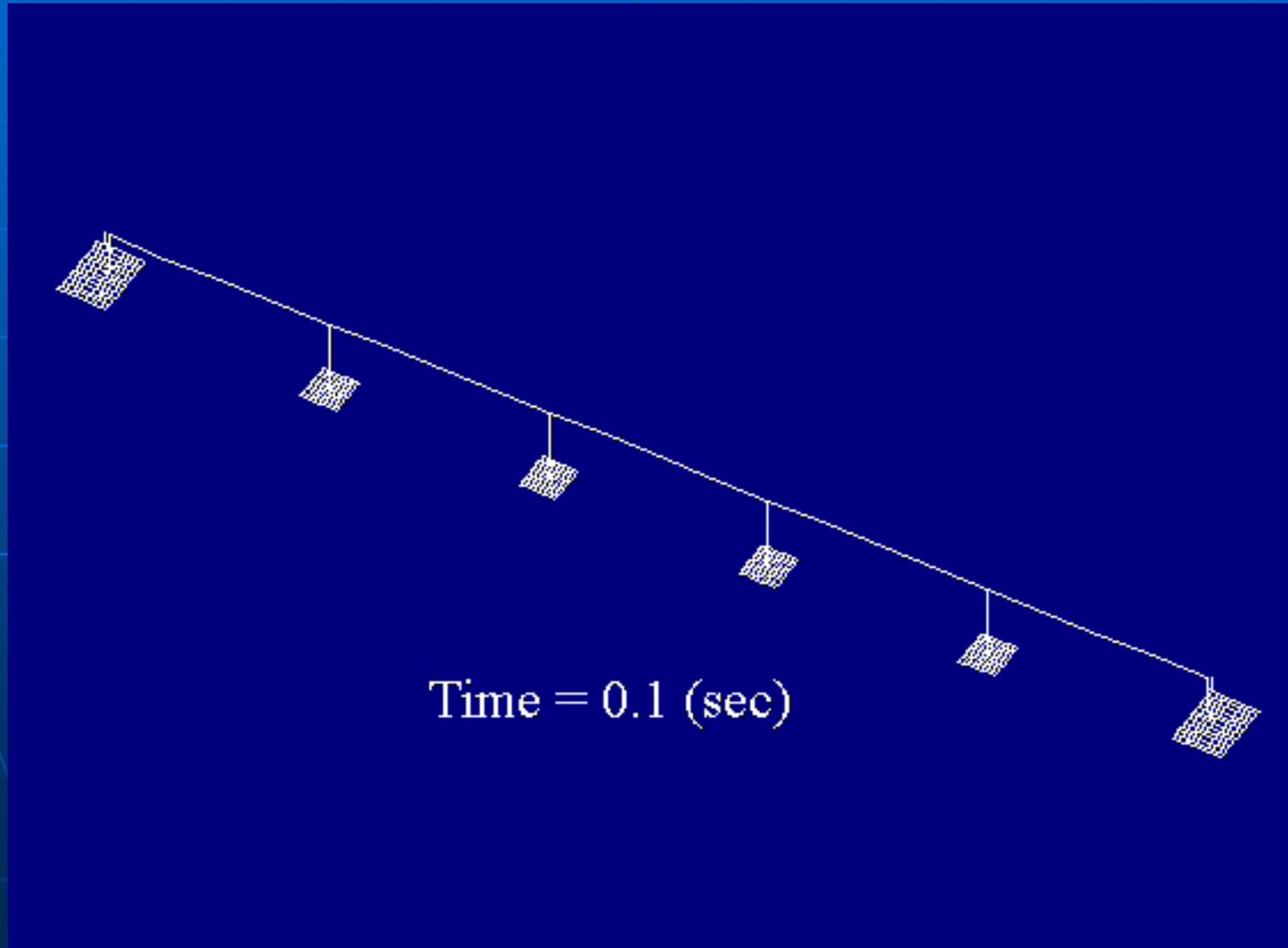
Transverse and Vertical



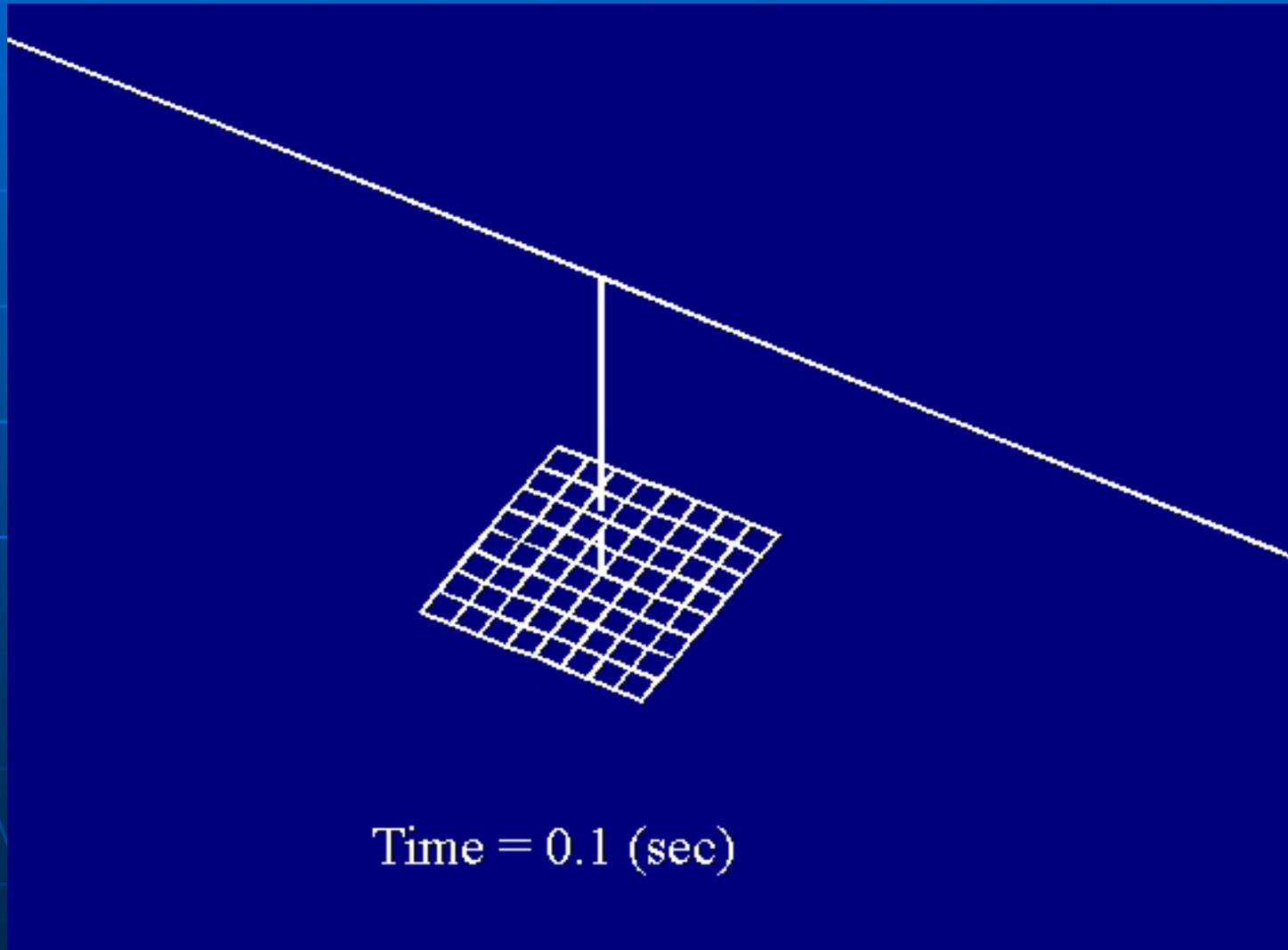
Idealization of Target Bridge



Response of Target Bridge under JMA Kobe Ground Motion

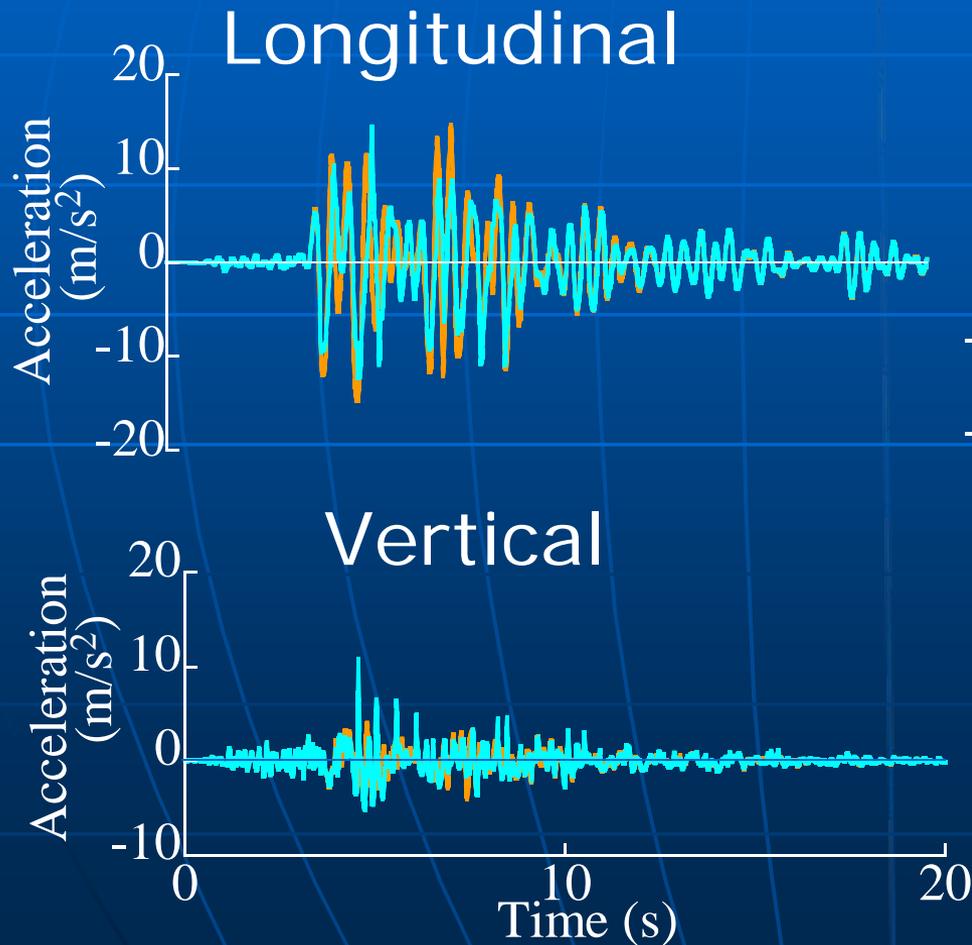


Response of P2 under JMA Kobe Ground Motion

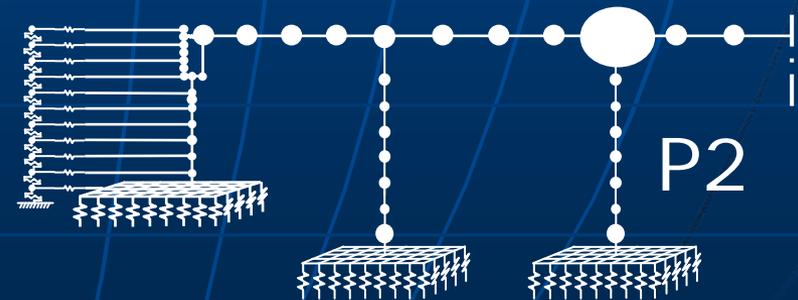
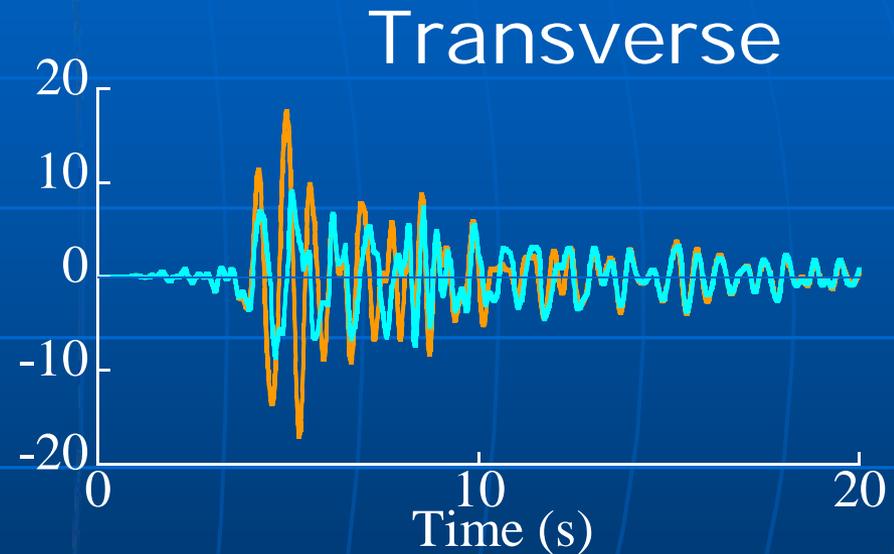


Deck Acceleration at P2

Conventional

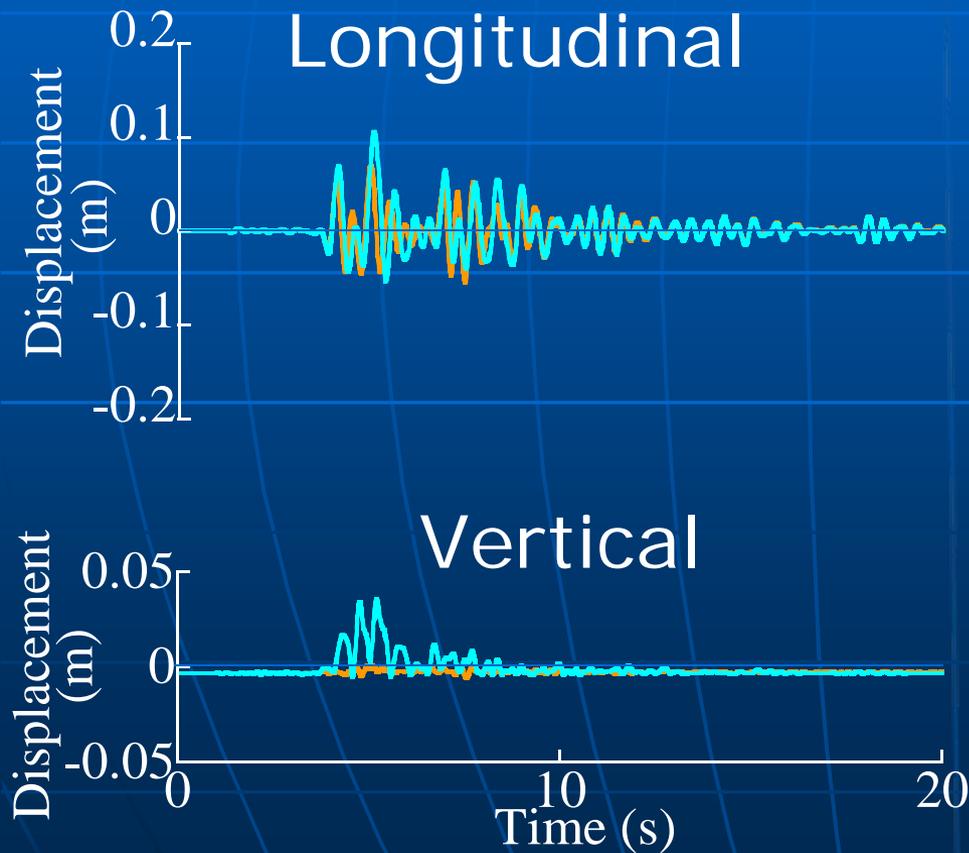


Rocking Isolation

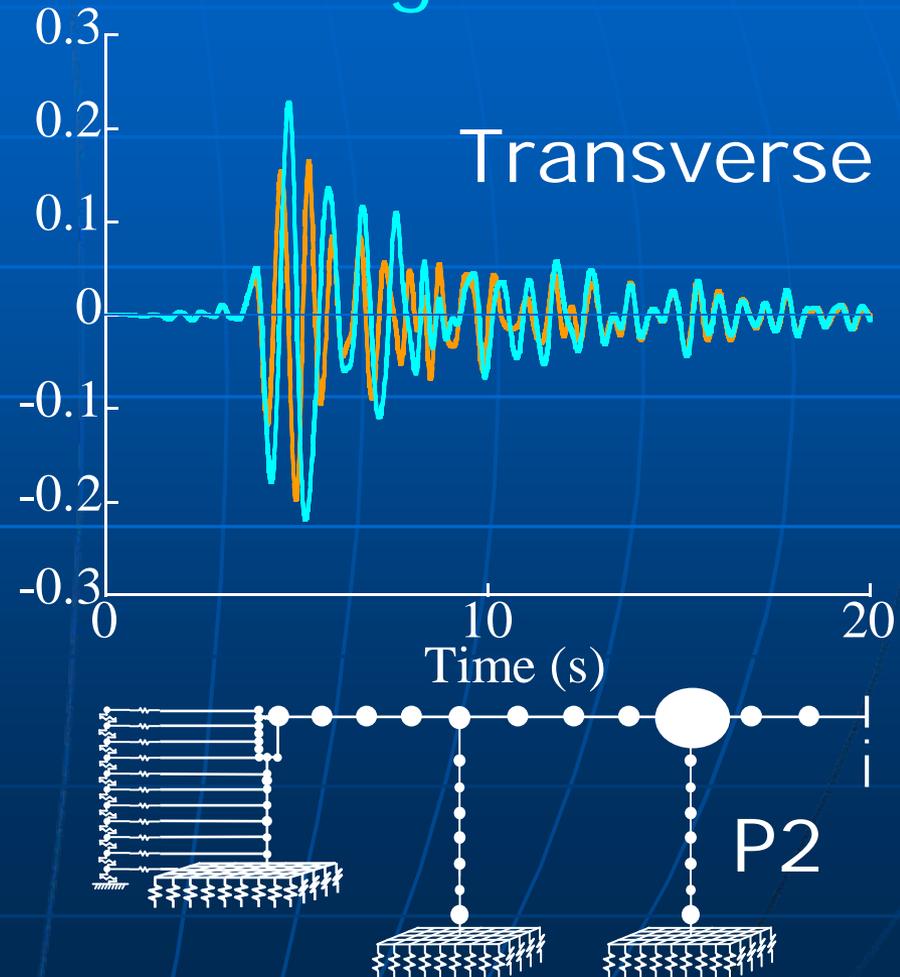


Deck Displacement at P2

Conventional

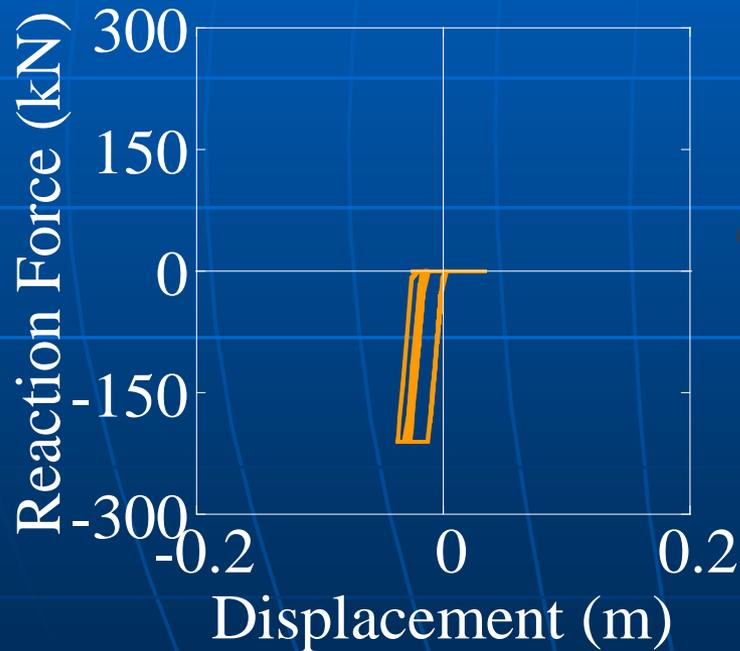


Rocking Isolation

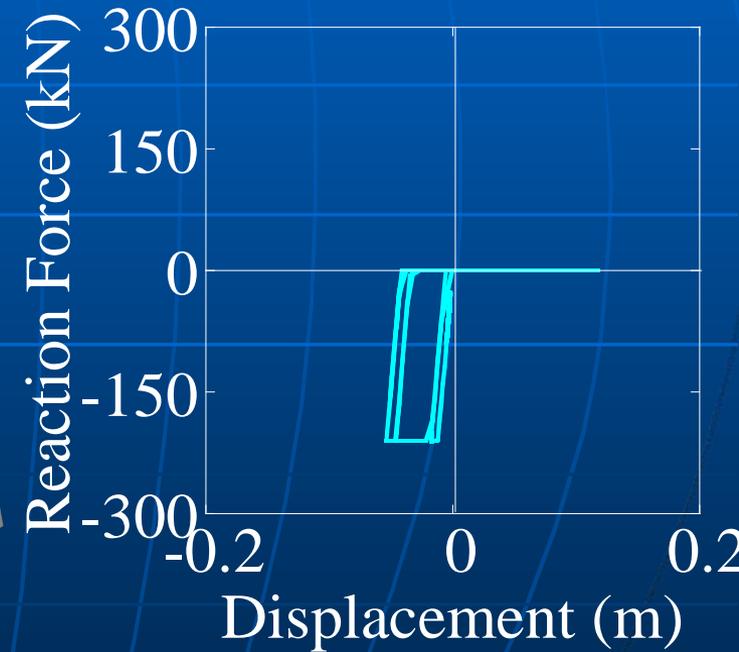


Abutment and Backsoils Interaction in the Longitudinal Direction

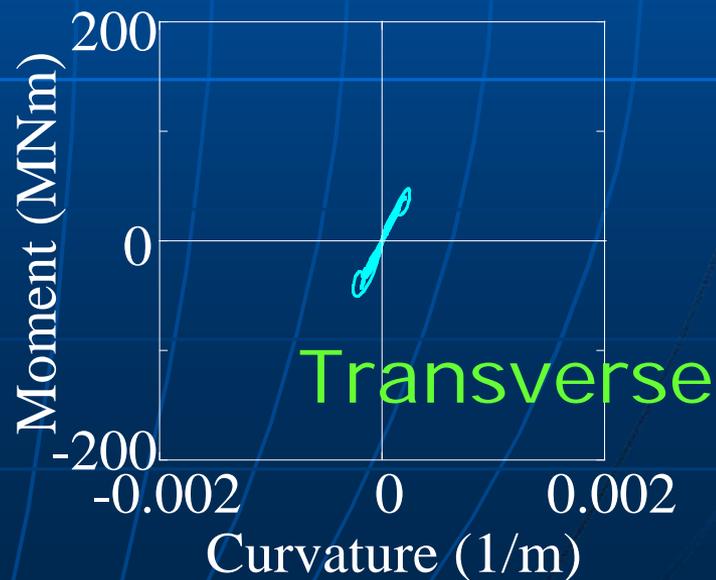
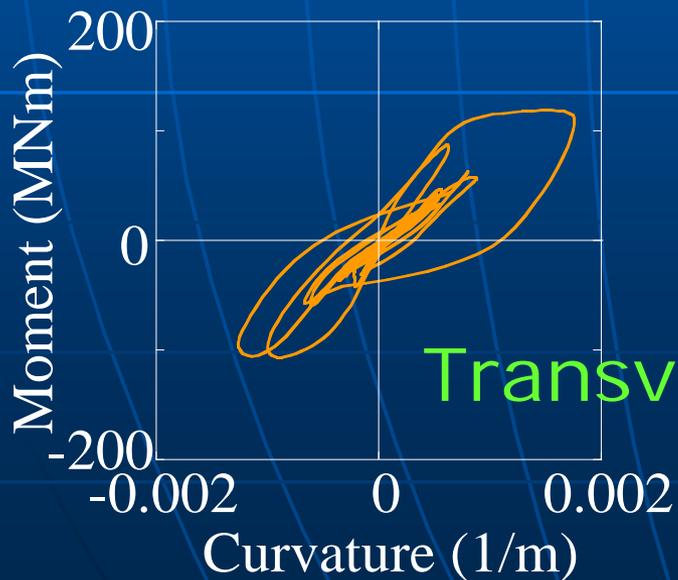
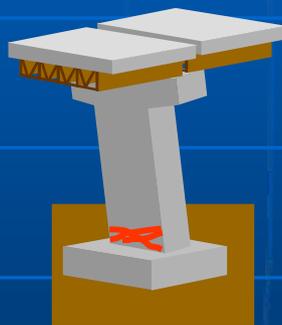
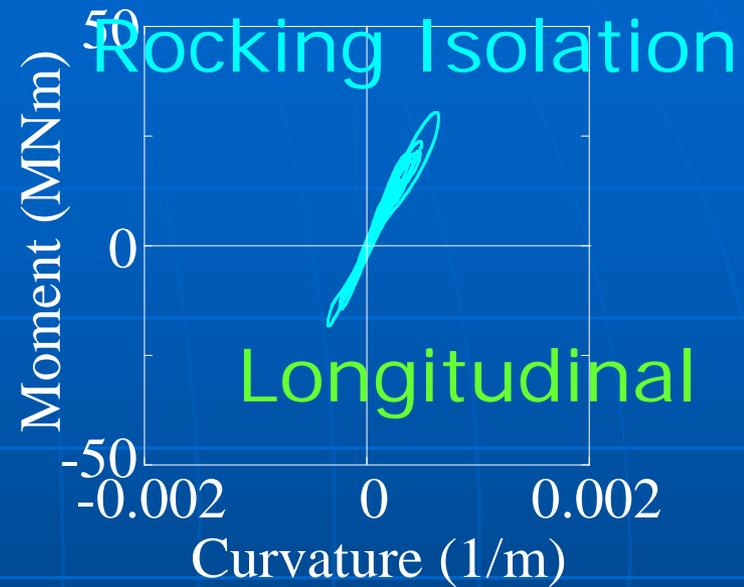
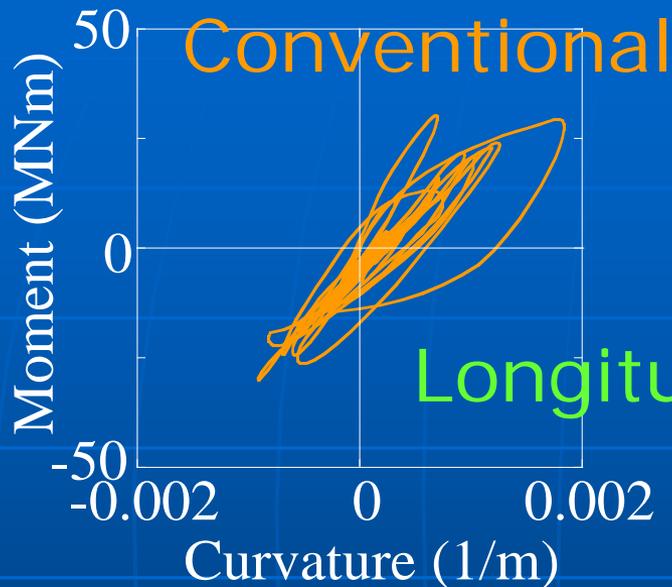
Conventional



Rocking Isolation

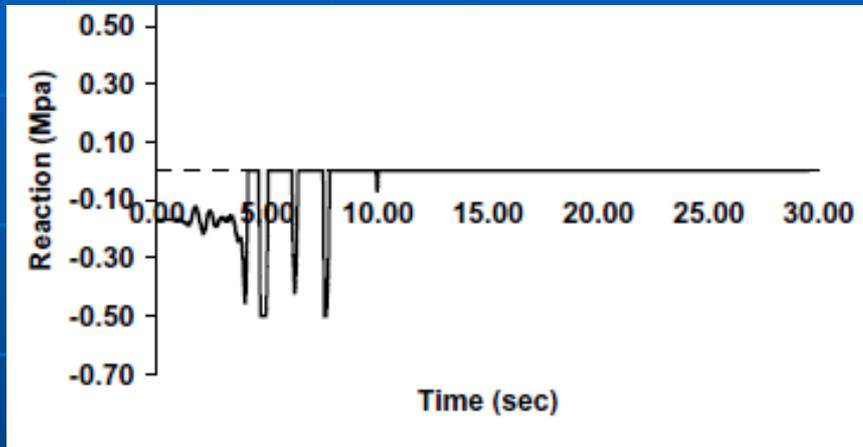


Curvature at the Plastic Hinge of P2

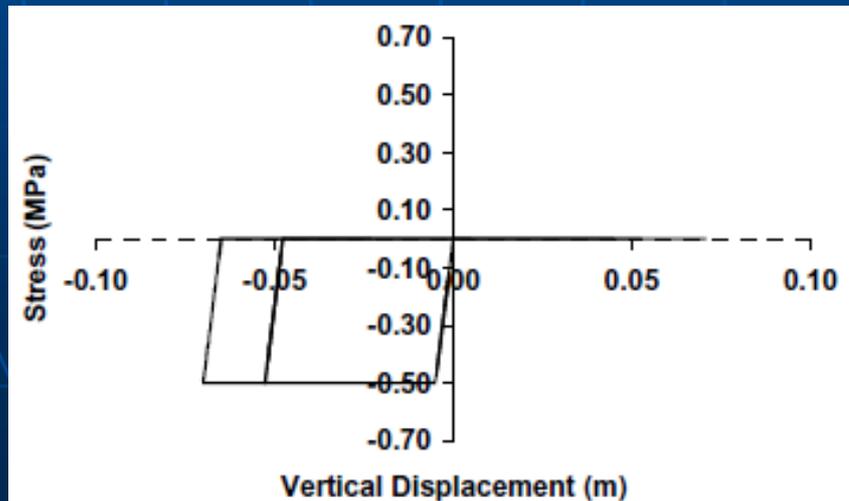
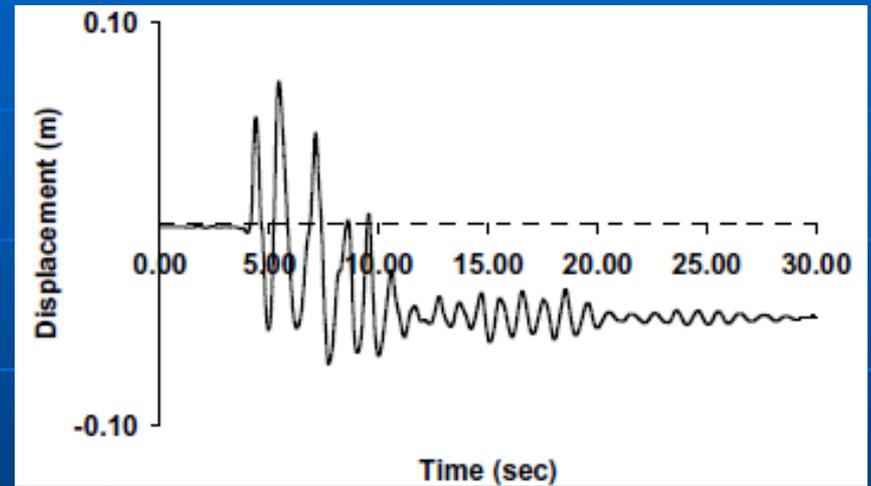


Effect of Yield of the Underlying Ground (0.5MPa)

Vertical stress



Vertical displacement



Stress vs. vertical displacement

Conclusions

- When separation of a footing from the underlying ground due to rocking response is included in analysis, the plastic deformation of a column significantly decreases as a result of softening of the moment vs. rotation hysteresis of the footing.
- If the underlying ground yields, it enhances the effect of rocking isolation, however it increases the deck response displacement and it can result in residual drift.
- Bridge response acceleration decreases under the seismic rocking isolation, however bridge response displacement increases.

Related publications

- Kawashima, K., Unjoh, S. and Shimizu, H.: Analysis of rocking vibration of rigid foundations, Proc. 8th US-Japan Bridge Workshopn, Panel on Wind & Seismic Effects, UJNR, Chicago, USA, pp. 3-17, 1992
- Kawashima, K., Unjoh, S., Shimizu, H. and Mukai, H.: Analytical method of seismic rocking response of a rigid foundation considering the response of superstructure, Journal of Civil Engineering, Vol. 36, No. 2, pp. 42-36, 1994
- Kawashima, K. and Hosoiri, K.: Rocking response of bridge columns on direct foundation, Proc. Fib-Symposium, Concrete structures in seismic region, Paper No. 118 (CD-ROM), Athens, 2003
- Mergos, P. E., and Kawashima, K.: Rocking isolation of a typical bridge pier on spread foundation, Journal of Earthquake Engineering, Vol. 9, Special Issue 2, pp. 395-414, 2005
- Sakellaraki, D. and Kawashima, K.: Effectiveness of seismic rocking isolation of bridges based on shake table test, First European Conference on Earthquake engineering and Seismology, Paper No. 364, pp. 1-10, Geneva, Switzerland, 2006

Thank you for your kind attention.