Investigation of Soils Placed Behind Caltrans' Abutments

Caltrans-PEER Seismic Seminar June 8, 2009

Po Lam Earth Mechanics, Inc.

Topics:

- How many bridges were investigated?
- How were the bridges selected?
- What type of soil is placed behind the back wall of Caltrans' bridges?
- What were the significant parameters in the study?
- What were the results of the study?
- What is the confidence level in generalizing your results?
- How did the study influence Caltrans' practice?
- What is the expected performance of abutment backfill?

How Many Bridges were Investigated:

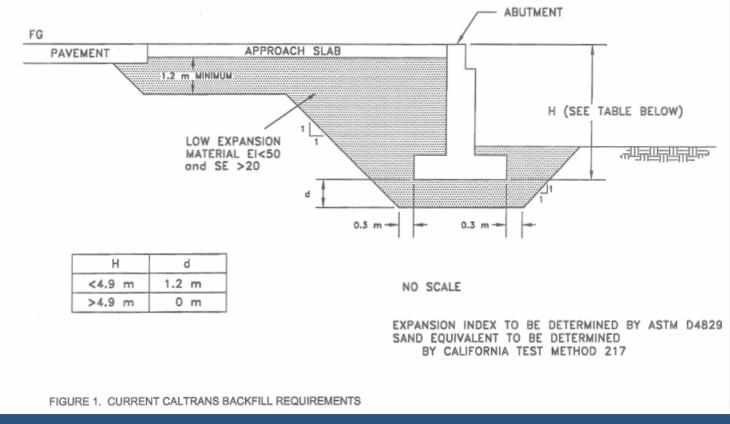
Project from 2002-2006, through an UCSD Contract.

- Two Phase Approach:
- Phase I- Screening for Candidate Sites for Borings.
- Phase II- Conduct Site Investigation.
- During Phase I:

Reviewed 90 grew to 115 Bridge Plans and Boring Logs Statewide for Screening for about 20 sites for more In-depth Investigation, including Site Reconnaissance. Reconnaissance Sites grew to 35 sites.

- During Phase II Data led to Scope of Site Investigation for 10 Sites.
- Actual Phase II Scope: Boring at 11 Sites including 26 Soil Borings, 22 CPTs and 14 PMT Plus Bulk Sample Testing at 4 Other Sites

Caltrans Abutment Backfill Requirements



- Prior to 1995-96, there is no backfill requirements regarding soil type.
- Caltrans Standard Specifications, 1999a, Sections 19.306 and 19-3.065 contain gradation requirements and a compaction requirement of 95% relative compaction.

Database Compiled in Phase I

Т	able	1. Bridg	e Dat	abas	е						Date:	1/19/200)5 3:58:15 F	MK/EMI
ID#	Bridge No.	Bridge Name	County	Route	Date Built	Have Plans?	Visited	Candidate for Drilling?	Comments	# of Lanes	Rdwy Width	Latitude	Longitude	Location Description
86	57-0791 S	"E* Street On-Ramp	San Diego	1-5	1992	Y	N	N?	drill thru cone. appr. Slabs	2	11, 13.7	32,6396	-117.0998	Chula Vista, I-5/route 54 intersection
17	23-0213	Allison Drive OC	Solano	I-80	1997	Y	N			4	28	38.3615	-121.9693	Vacaville, just south of Nut Tree Factory
87	57-0935	Ammo Rd OC	San Diego	1-15	1982	Y	N	N		2	8.5	32.8751	-117.1070	Miramar Marine Corps Airstation-Difficult to pinpoint
29	39-0154	Arburua Rd OC	Merced	1-5	1971	Y	N			2	24.1	36.9643	-120.8824	~7 mi SW of Los Banos
72	54-1107	Archibald Ave Offramp SEP	San Bernardino	I-10	1996	Y	Υ	N		2	11.2	34.0676	-117.5933	North of Ontario Airport
51	53-1204	Atlantic Ave OC	Los Angeles	I-405	1963	Y	Y	Y*		4	28	33.8153	-118.1851	Long Beach
54	53-1986	Balboa Boulevard OC	Los Angeles	I-5	1971	Y	N	N	no struct. Plans, T/C diff. @ north abut	2	12.2	34.3209	-118.4964	LA, near I-5/I-210
33	44-0124	Bardin Rd OC	Monterey	US-101	1955	Y	N			2	8.5	36,6610	-121.6230	Near Salinas Municipal Airport-CANT FIND SPECIFIC ON MAP
21	28-0083 S	Barrett Ave UC	Contra Costa	I-80	1997	Y	Y	Y	busy, may require TC	1	7.3	37.9355	-122.3268	Richmond over Barrett Ave
53	53-1605	Barrington Ave	Los Angeles	I-10	1963	Y	Y	N		10	42.7	34.0296	-118.4441	LA, just west of I-10, I-405 intersection
23	34-0047 S	Bay Shore Ave UC	San Francisco	US-101	1950	Y	Y	N	No LOTB avail, busy loc, TC req'd, permit	2	7.9	37.71592	-122.3985	San Francisco, US-101 south of 280
114	51-0234	Beckstead OC	Santa Barbara			N	Y	Y	Collected bulk sample			34.4738	-120.2090	Santa Barbara
43	52-0247	Borchard Rd OC	Ventura	US-101	1962, 1992	Y	N	N	T/C not feas.	2	11.3	34.1844	-118,9256	7.1 mi west of LA County line in Ventura County
34	44-0131 L	Boronda Rd OC	Monterey	US-101	1965	Y	N	N	has MSE walls	2	12.3	36.7220	-121.6599	Salinas
65	54-0415	Bridge Across Colorado River at	San Bernardino	I-40	1966	Y	N	Y?	req. T/C	4	18.3	34.71696	-114.48657	Colorado River Crossing on I-40 at CA/AZ borde
50	53-1203	California Ave OC	Los Angeles	1-405	1964	Y	Υ	Y		2	17.1	33.8143	-118.1806	Signal Hill
92	57-1083 R/L	Camino Ruiz UC	San Diego	SR-56	2002	Y	Y	Y				32.9580	-117.1553	San Diego, on SR-56 between I-5 and I-15
100	57-1077 L	Carmel Valley Rd UC	San Diego	SR-56	2004	N	Y	Y	Drilled 2/7/04. Collected bulk sample			32.9487	-117.2031	San Diego, on new SR-56 between I-5 and I-15
89	57-0991 S	Carmel Valley UC	San Diego	I-5	1996	Υ	Y	Y		2	11.9	32.9326	-117.2410	Del Mar, north of 1-5/I-805 merge
48	53-1200	Cherry Ave OC	Los Angeles	I-405	1964	Y	Y	Y*		4	25.6	33.8135	-118.1676	Long Beach
108	53-1353	Coldwater Canyon	Los Angeles	SR-101	1971	Y	Y	Y	Drilled 4/19/04			34.1569	-118,4137	101 over Coldwater Canyon
56	53-2264	Colorado Boulevard SEP	Los Angeles	SR-134	1975	Υ	N	N		5	26.5	34.1457	-118.1549	Pasadena

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Selected Drilling Sites for Phase II



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Site Explored Bridges in Phase II

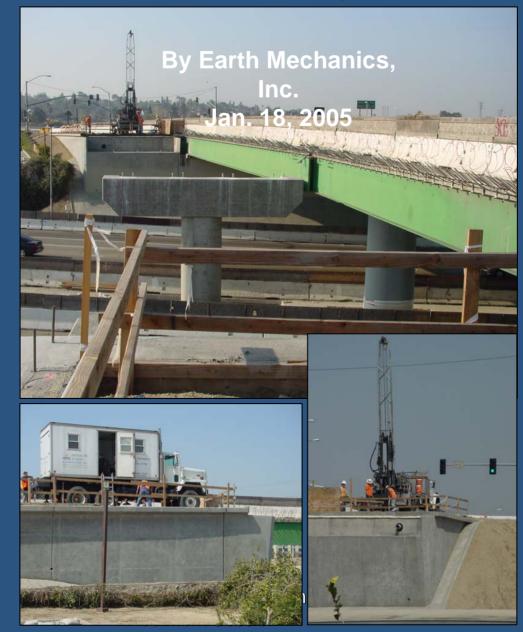
									(17) III	_		10.0	
				Abut		rings/S			CPT			PMT	
ID#	Bridge No.	Bridge Name, County	Drilled	Wall Height	No.	Depth (ft)	Setback (ft)	No.	Oepth (ft)	Setback (ft)	No.	Depth (ft)	Setback (ft)
27	37-0609	Rte 87/880 SEP	Drilled 1/30/04	13.6	B-1	31.5	42.5	CPT-1	30.0	44.5			
		Santa Clara		12.2	B-2	31.5	45.0	CPT-2	30.0	47.0			
101	57-1078 S	Gonzalez Creek Off-Ramp San Diego	Drilled 2/7/04	8.0	04-1	25.5	2.0	04-1C	17.4	7.0			
100	57-1077 L	Carmel Valley Rd UC San Diego	Drilled 2/7/04. Collected bulk sample	8.0	04-2	22.8	8.0	04-2C	25.0	8.0			
106	55-0257	Harbor Blvd UC (Widen)	Drilled 2/28/04	27.3	04-3	21.5	13.0	04-3C	15.0	13.0			
		Orange		10.0	04-4	21.5	2.0	04-4C	20.0	3.7			
				27.3				04-5C	20.0	5.0			
107	55-0414	Tustin Ave OC (Widen)	Drilled 3/6/04	20.3	04-6	21.5	5.5	04-6C	20.0	16.0			
		Orange		25.4	04-7	21.5	6.5	04-7C	20.0	16.0			
				25.4	04-8	11.5	18.0						
108	53-1353	Coldwater Canyon	Drilled 4/19/04		04-9	16.5	5.0	04-9C	14.3	5.0	04-9P	7.5	5.0
		Los Angeles									04-9P	12.5	5.0
					04-10	16.5	5.0	04-10C	14.9	5.0	04-10P	10.0	5.0
109	33-0250	Patterson Slough	Drilled 6/17/04	5.5	04-11A	21.5	3.0	04-11C	20.0	3.0	04-11P	5.0	3.0
		Fremont		5.5	04-12A	21.5	3.0	04-11C	20.0	3.0	04-12P	5.0	3.0
10	35-0083	Route 101/84 SEP	Drilled 6/13/04		04-14A	21.5	6.0	04-14C	28.0		04-14P	7.5	6.0
		San Mateo			04-13A	21.5	9.0	04-13C	20.0		04-13P	7.5	9.0

				Abut	Bo	rings/S	PT		CPT			PMT	
ID#	Bridge No.	Bridge Name, County	Drilled	Wall Height	No.	Depth (ft)	Setback (ft)	I No.	Depth (ft)	Setback (ft)	No.	Depth (ft)	Setback (ft)
112	04-0236	Painter St	Drilled 7/1/04	12.3	04-15A	15.5	5.0	04-15C	14.0	4.0	04-15P	8.0	5.0
		Humboldt		11.5	04-16A	14.0	4.0	04-15D	9.5	5.0	04-16P	12.5	2.0
				12.3				04-16C	12.5	5.0			
111	58-0215	Meloland Rd OC	Drilled 8/20/04	13.0	04-17A	21.5	3.0	04-17C	30.0	3.0	04-17P	11.0	3.0
		Imperial		13.0	04-18A	21.5	2.5	04-18C	4.0	3.0	04-18P	7.5	2.5
											04-18P	11.5	2.5
114	51-0234	Beckstead OC	Collected bulk		04-19	3.0							
		Santa Barbara	sample										
84	56-0318	Spruce Street OC (Replace)	Collected bulk		04-20	3.0							
		Riverside	sample										
81	56-0390	Jackson St OC (Replace)	Bulk sample		04-21	3.0						_	
		Riverside	collected										
113	53-0687	Santa Clara River (Replace)	Collected bulk		04-22	3.0							
		Los Angeles	sample. Drilled 6/5-6/04		04-23	25.0	3.0	04-23C	25.0	5.0	04-23P	5.5	4.0
					04-24	25.0	5.0	04-24C	25.0	7.0	04-24P	7.5	4.0
116	42-0398	Maple Ave UC	Collected bulk		04-25	10.0							
		Fresno	sample										
Total	Count of Brid	Iges Selected: 15	Tot	al Counts:	26 B	orehol	es	22	CPT's	;	14	PMT	s
		w existig grade. Setback is distance in fi	t behind back face	of abutment	wall								

				Abut	Bo	rings/S	PT		CPT			PMT	
ID#	Bridge No.	Bridge Name, County	Drilled	Wall Height	No.	Depth (ft)	Setback (ft)	No.	Depth (ft)	Setback (ft)	No.	Depth (ft)	Setback (ft)
112	04-0236	Painter St	Drilled 7/1/04	12.3	04-15A	15.5	5.0	04-150	: 14.0	4.0	04-15P	8.0	5.0
		Humboldt		11.5	04-16A	14.0	4.0	04-15D	9.5	5.0	04-16P	12.5	2.0
				12.3				04-160	12.5	5.0			
111	58-0215	Meloland Rd OC	Drilled 8/20/04	13.0	04-17A	21.5	3.0	04-170	30.0	3.0	04-17P	11.0	3.0
		Imperial		13.0	04-18A	21.5	2.5	04-180	4.0	3.0	04-18P	7.5	2.5
											04-18P	11.5	2.5

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Final Documentation Report



Research Documentation to Caltrans/UCSD 2005

Field Boring Program Documentation

Soil Borings with Sampling

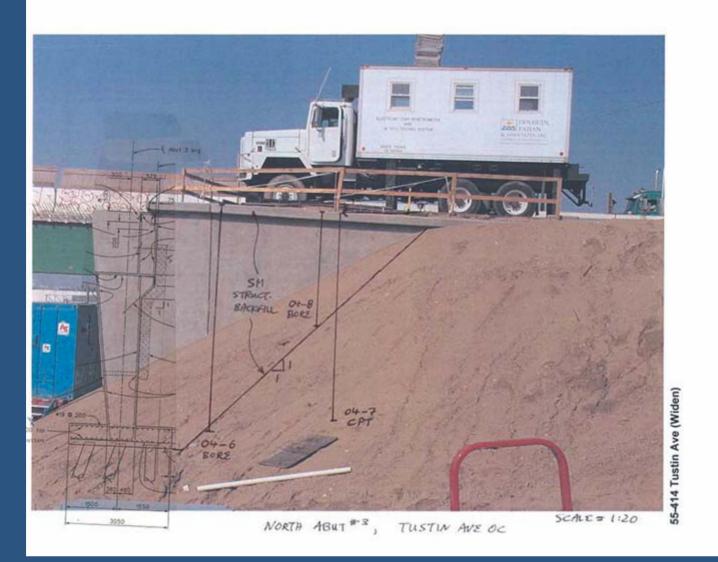
7-1-04, Painter St, West Abutment PC Exploration Drilling 04-15A

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Caltrans Bridge 58-0215, Meloland

Verification of Compliance for New Constructions



Backfill Testing behind Bridge Abutment

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Seismic Cone Penetrometer Tests

6-30-04, Painter St Bridge, West Abutment. First CPT attempt, 04-15, refusal at 9.5' depth.

Cone Penetrometer Testing with Seismic Wave Velocity Measurement

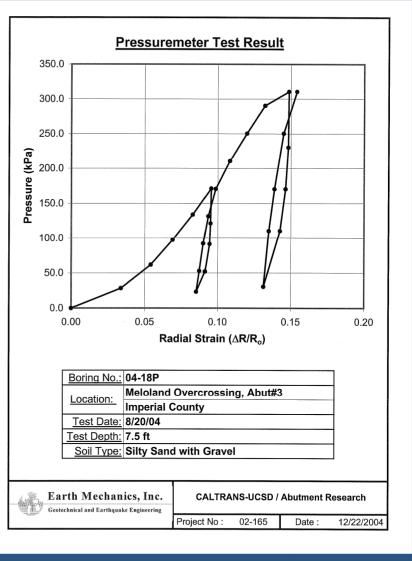




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Pressuremeter Tests





In-hole Pressuremeter Testing

Soil Boring Logs

-

Bridge	County	Boring	Sample	Depth	Soil Type	SPT Blow- count	Moist. Content	Dry Density	Void Ratio	Poro- sity	Gra	ain Size	e Distri		Att	terberg	Limits	Sand Equi- valent	Expan- sion Index	Com-p	paction	n Unco	onsolidated Undra	ained Tri	iaxial		Direct	Shear		РМТ
Ъ	õ	No.	No.			N _{SPT}	w	ρ _d	е	n		Sand		sand size		_	plast.	SE	EI	ρ _{d,max}		ρ _d	σ	¢	с	¢ _{peak}	Cpeak	φ _{res}	Cres	Depth
				(ft)	(USCS)	(bpf)	(%)	(pcf)	(-)	(%)	(%)	(%)	(%)	(-)	(%)	(%) (%) (-)	(-)	(-)	(pcf)	(%	(pcf)	(ksf)	(deg)	(ksf)	(deg)	(ksf)	(deg)	(ksf)	(ft)
			B-0		ML/CL																									
			D-1		ML/CL	18											_					120.7	1.08/2.15/4.32	6.9	0.92					<u> </u>
		04-11A	S-2		Sa. CL	12	15.5				0	30	70	fine	28	16 12	2 med.									_				i
_			D-3	12.50		23	15 7				_	15	0.5	fine	07	17 0		-				121.8	1.73/3.46/6.91	15.2	4.34	-				i
Patterson Slough	_		S-4		Sa. CL	32	15.7	107.7	0.50	20.00	0	15	85	fine	3/	17 20) med.									-				i
S	Alameda		D-5 B-0	20.00 0-5	Sa. CL	19	18.7	107.7	0.56	36.08	0	35	65	fine			l-m									-				<u> </u>
5	Ĕ		Б-0 D-1		SM w/ Gr	26	5.1	125.0	0.34	25.27	20	50	25	ooorco			none									-	-			<u> </u>
ŝ	Ala		D-1 D-2		Sa. CL w/ Gr	30	10.7	125.9 120.8	0.34	28.30	10	30	60	coarse coarse			low									+				5.0
ا ﷺ			S-3		Sa. CL W GI Sa. CL	15	15.4	120.0	0.39	20.30	0	25	75	fine			med.									+				0.0
۵Ľ		04-12A	D-4		Sa. CL Sa. CL	32	19.2	115.8	0.45	31.27	0	30	70	fine			I-m	1						1		-				11.0
			S-5		SM w/ SP	32	18.3	110.0	0.40	51.27	0	25	75	fine	38	26 12	_					-				+				11.0
			D-6		Sa. CL	45	15.7	107.6	0.57	36.13	0	30	70	fine		20 12	med.													
			S-7		Sa. CL	19	21.0		0.01		0	30	70	fine			med.													
			B-0	0-	Sa. CL w/ Gr		11.8				-			coarse			med.	9												
			D-1	2.50	SP	19	21.0	95.7	0.76	43.20	0	95	5	medium			none													
			S-2	5.00	SM w/ Gr						10	70	20	coarse			none													
			D-3	7.50	SP	10	5.8	89.8	0.88	46.70	0	95	5	medium			none													7.5
			S-4	10.00	SP	2	7.3				0	95		medium			none													
•		04-13A	D-5	12.50	SP-SM	10	7.2	95.2	0.77	43.49	0	90	10	medium			none													í
SEP	0		D-6	15.00	SP-SM	11	7.9				0	90	10	medium			none													1
84	Mateo		S-7	18.50			8.1																							
ģ	Ŵ		D-8		Sa. CL w/ Gr	27	7.6	125.9	0.34	25.27	10	25	65	coarse			med.													
e l	San		S-9		SC w/ Gr	37	11.0							coarse			l-m													<u> </u>
Route 10/84	s		D-10		CL-CH	39	28.2	94.6	0.78	43.85				fine			m-h													<u> </u>
°≃ ∣			D-1		SC w/ Gr	45	6.7	124.1	0.36	26.34	20	55	25	coarse			low													<u> </u>
			D-2		SC-SM	22	10.0	114.4	0.47	32.10	10	60	30	coarse			low													7.5
		04-14A	D-3		SC w/ Gr		7.3				5	60	35	coarse	29	15 14	_					125.1	1.37/2.73/5.47	22.8	1.28					<u> </u>
			S-4		Sa. CL	26	8.6				0	35	65	coarse			I-m									_				<u> </u>
			D-5		Sa. CL/SC w/ Gr	36	10.0	115.0	0.47	31.74	20	40	40	coarse			low	-								-				i
\rightarrow			S-6		SM w/ Gr	24	16.4				0	65	35	fine			low	40								-				
			B-0 S-1		GW-GM	50	2.2				59	35	6	coarse	\vdash		none									+				
			S-1 S-2B	2.50 5.00	SP w/ Gr SP w/ Gr	59 47	3.1 3.9				25 25	70 70	5 5	coarse coarse			none none					129.5	0.87/1.01/1.3	43.3	0.13	+	-			
			3-26 В-3А		SP W/ CL	47	4.5				20	70	5				-	-				129.5	0.0771.0171.3	43.5	0.13	+				
		04-15A	Comp.	2.5-7	SP W/CL SP		4.0							coarse			none					129.5	1.00	NA	NA	-				
		04-13A	D-3B		Se. CL	32	19.9				3	35	62	coarse	34	19 1	5 low						1.0/2.01/4.02			+				8.0
st	Ŧ		S-4		CL w/ Sa.	16	16.4				5	55	02	medium	54	13 1.	med.					113.7	1.0/2.01/4.02	11.5	5.50	-				0.0
5	Humboldt		D-5		CL w/ Sa. CL w/ Sa.	18	21.0	116.2	0.45	31.03				coarse			med.					116.2	1.07	0.0	7.75	+				
Painter	đ		S-6	14.00		20	21.0	110.2	0.40	01.00				medium			med.	1				110.2	1.07	0.0	1.15	+				i
Pai	로		B-0	0.00	SP		2.2				35	60	5	coarse			none	1								+				i
-	_		S-1	25	SP	63	4.2				25	70	5	coarse			none	-								+				í
			D-2		SP	90	4.1	124.5	0.35	26.10	20	75	5	coarse			none	-								+				i
		04-16A	S-3A	7.50	CL		19.6				0	20	80	fine			med.	1								1				í
			S-3B		SP	11	5.8				10	85	5	coarse			none	1								1				í
			D-4		CL w/ ML	19	28.9	90.2	0.87	46.46	0	25	75	fine	42	27 1		1				115.2	1.3/2.59/5.46	7.1	3.30					í
			S-5	12.50		19	21.2				0	25	75	fine			med.					1				-				12.5

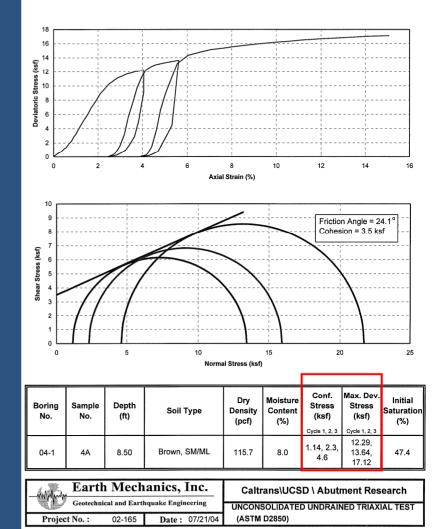
Laboratory Testing of Samples

Laboratory Triaxial Tests

			F	ASTM D2850	1	
Project Name:	UCSD \ A	butment R	esearch\Car	mel Valley Rd	UC\San Diego Co.	Project No: 02-165
Boring No.:		4-1		Tested by:	J.R	Date: 07/21/04
Depth (ft):	8	3.5	_ c	hecked by:		Date:
Sample No.	4	1A	Sa	mple Type:	Remolded	-
Soil Description:	Brown, S	M/ML	Com	ments:		-
	1	2	3			
Diameter (in.):	2.429	2.432	2.43	Average:	2.430	
Height (in.):	4.930	4.875	5.000	Average:	4.935	
						-
Moisture Content				SKETCH / F	PHOTO AFTER TE	ST:
Wt. Wet Sample + (173.95		02.165.0	Contraction of the second s
Wt. Dry Sample + C	Container (g):	161.7			
Container (g) :	No.		8.48			·
Moisture Content (%	6):		8.0	2). (*)	and the n	
				into a		114 22 (182) = [214622 (182) = [214622 (182) = [
Density and Satura				- 2		
Wt. Wet Sample + 0	Container (g):				
Container (g) :						
Wet Density (pcf) :			125.0			1 B
Dry Density (pcf) :			115.7	-		
Void Ratio : % Saturation :			0.456	-		
Gs=2.70 (assumed)			47.4			
GS-2.70 (assumed)						
				2	0	
				S20 Broatest		
Shear				At Failure	1 	
	Cycle	1, 3	2, 3	Atranule	Cycle	1, 2, 3
Confining Stress (ks			2.3, 4.6	Deviator Stres		1, 2, 3
Deformation Rate (%		·	4		cipal Stress (ksf) :	1.14, 2.3, 4.6
					oipai oileaa (nal) .	1.14, 2.3, 4.0

UNCONSOLIDATED UNDRAINED TRIAXIAL TEST

Earth Mechanics, Inc. Geotechnical and Earthquake Engineering



Determination of Cohesive Strengths - Silty Sand

Axial Strain (%) :

Eff. Major Principal Stress (ksf)

13.43, 15.93, 21.72

4.1. 5.6. 15.1

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June 8, 2009

Failure Criterion:

the maximum deviator stress

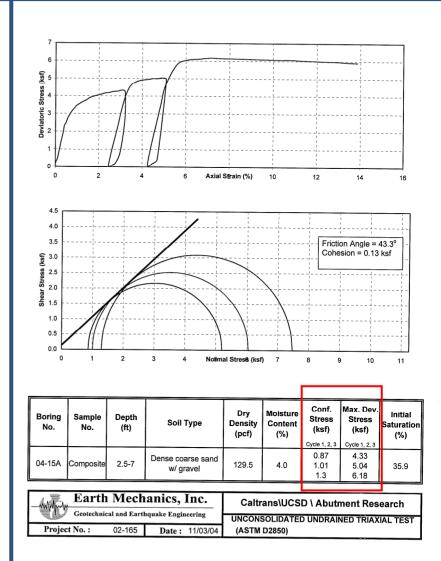
Determination of Cohesive Strengths – Sand with Gravel



Project Name:	UCSD \ At	outment Res	search\Pair	nter Street, I	Humboldt Co	Proje	ct No:	02-165
Boring No.:	04-15A 2.5-7 B-0, S-1, S-2B, S-3A Dense coarse sand w/ gra			Tested by:	J.R.		Date:	11/03/04
Depth (ft):	2.	5-7	c c	hecked by:		1.1	Date:	11/03/04
Sample No.	B-0, S-1, S	S-2B, S-3A	Sai	nple Type:	composite	- 31	-	
Soil Description:	Image: No. 04-15A Tested by: 2.5-7 Checked by: No. B-0, S-1, S-2B, S-3A Sample Type: cription: Dense coarse sand w/ gravel Com er (in.): 2.862 2.862 2.862 (in.): 2.862 2.862 2.862 5.750 5.750 5.750 Average: re Content Calculation SKETCH / PH SKETCH / PH t Sample + Container (g) : sample + Container (g) : v and Saturation 4.0 t Sample + Container (g) : istic (pcf) : 134.7 nsity (pcf) : 129.5	Comments:						
	1	2	Checked S-3A Sample Ty and w/ gravel 2 3 162 2.862 Avera 50 5.750 Avera SKE					
Diameter (in.):	2.862	2.862	Tested by:	2.862				
Height (in.):	5.750	5.750	5.750	Average:	5.750	· .		
								
Moisture Content	Calculation			SKETCH	I / PHOTO AFTER	TEST:		
Wt. Wet Sample +	Container (g):						
Wt. Dry Sample + 0	Container (g)	:				- 15A R	1	
Container (g) :	No.			1	1 102-			
Moisture Content (%):		4.0]				
]		and the state of the		
Density and Satur	ation						1 TR	
Wt. Wet Sample + 0	Container (g):					n a	
Container (g) :					line and	HIE DE		
Wet Density (pcf) :			134.7	1	Contraction of the	alen arrige in		
Dry Density (pcf) :			129.5	1		time and the		
Void Ratio :			0.301	1			1	
% Saturation :			35.9]			-	
Gs=2.70 (assumed)) 	11703	37,2

Shear		At Failure	
Cycle	1, 2, 3	Cycle	1, 2, 3
Confining Stress (ksf) :	0.87, 1.01, 1.3	Deviator Stress (ksf) :	4.33, 5.04, 6.18
Deformation Rate (% / min) :	4	Eff. Minor Principal Stress (ksf) :	0.87, 1.01, 1.3
Failure Criterion:		Eff. Major Principal Stress (ksf) :	5.2, 6.04, 7.47
the maximum deviator stress		Axial Strain (%) :	3.1, 4.9, 7.1

Earth Mechanics, Inc. Geotechnical and Earthquake Engineering



Determination of Cohesive Strengths – Sand with Gravel

Table 4 SOIL TYPE CATEGORIES CT-UCSD/ABUTMENT RESEARCH PROGRAM

		Soil Type Categories	Numb	of	Depth (ft)	Sand grain size	Plasticity	SPT Blow- count N _{SPT}	Content	Field Dry Density	<u> </u>	Size Disi Sand	ribution Fines	Plas- ticity Pl	Sand Equi- valent SE	Expan- sion Index El	Compa		so	engths o bil (95% f	f compacted R.C.)	Shear wave velocity	Secant Modulus initial Es	Secant Modulus reload E _R
			bridge	tested				(bpf)	(%)	Pd (pcf)	Gravel (%)	(%)	(%)	(%)	(-)	(-)	Yd,max (pcf)	W _{opt} (%)	ρ _d (pcf)	φ (deg)	c (ksf)	v _S (fps)	(MPa)	(MPa)
	ı	DENSE TO VERY DENSE SANDS WITH GRAVEL	2	2	2.5-7.5	coarse	none-low	52-59	3-6	120	8-25	70-79	5-13	<5 (0)	37-43 (40)	0-7	NM	ΝМ	125-130 (125)	35-43 (38)	0.13-0.3 (0)			
ľ	11	MEDIUM DENSE SILTY SANDS, SOME WITH GRAVEL	9	10	0-9.5 (0-5)	fine-coarse	none-low	9-15	5-14	105-120	1-28	60-87	12-35	5-10 (8)	21-52 (34)	0-2	109-127	8-12	116-127 (120)	24-35 (33)	0.25-1.5 (0.5)	700-1,000 (800)	2-4	10-15
	Ш	MEDIUM DENSE CLAYEY SANDS, SOME WITH GRAVEL	4	4	5-10	medium coarse	low	20-44	7-15	113-114	0-8	50-60	35-50	6-14 (10)	11	0	NM	NM	116-125 (120)	22.5-24 (23)	1.3-6 (2)			
	IV	STIFF-HARD CLAYS WITH FINE TO COARSE- GRAINED SANDS, SOME WITH SILTS	5	10	5-12.5	fine-coarse	medium- high	18-35	14-29	90-116	0-3	25-35	62-75	12-41 (22)	0-3 (0)	20-77	114-122	17-22	106-122 (110)	0-17 (6)	1-8 (3.5)	550-800	1.5-3	6-10

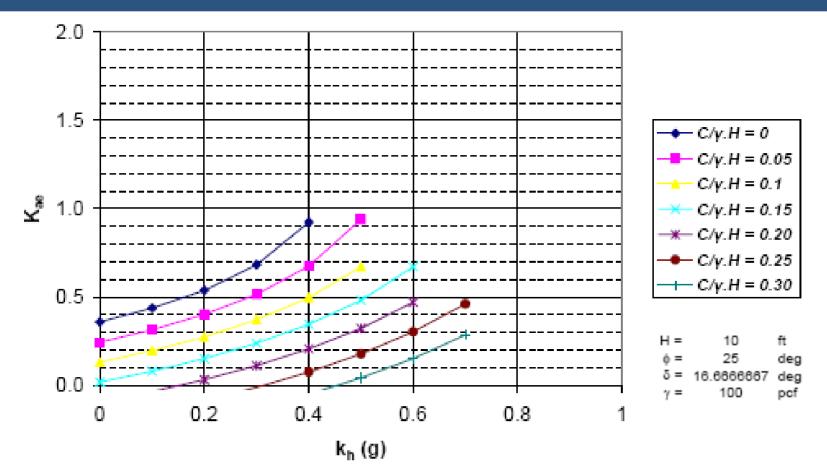
rypical parameters are in ()

NM=Not measured

Interpreted Backfill Properties

Most Significant Aspects of Backfill Property for Design

 Cohesive Strength Parameter for Design, Especially for Sandy and Gravely Soils for the Seismic Load Case

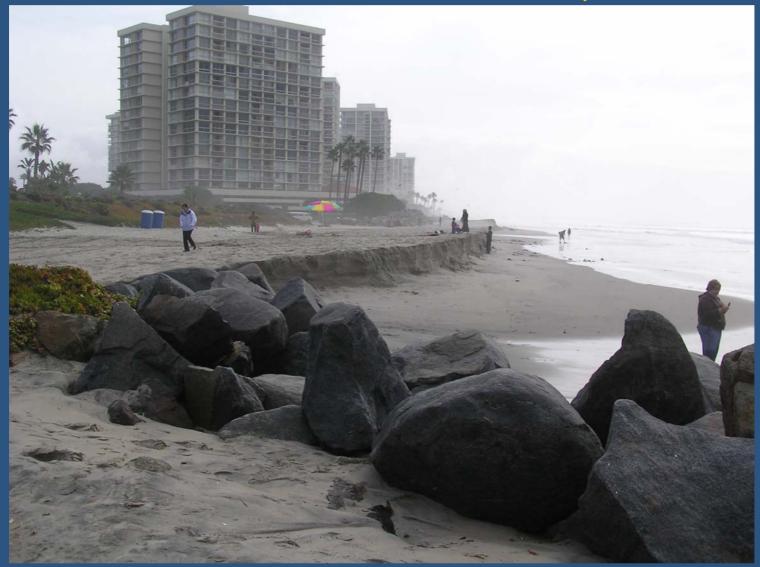


Influence of c on Earth Pressure Coefficient for Cohesionless Soils Caltrans - PEER Seismic Seminar

June 8, 2009

- Abutment Session

How Reasonable is a zero c Assumption?



June 8, 2009

THANK YOU!

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