#### **Structural Simulation Models**

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#### PEER OpenSees Framework

- Software framework for integrating
  - Material and component models. Emphasize degradation and failure behaviors
  - Solution strategies: Static and dynamic for degrading and collapsing systems
  - Performance evaluation based on simulated behavior
- Utilize new computing resources
  - Engineering desktop workstations (SMP, distributed)
  - High-performance computing
  - Computational grids
- Provide network communication mechanisms with scientific visualization methods and databases





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**Structural Beam-Column Models** 





#### **Axial Force-Flexure-Shear Behavior**







#### Other issues

- Parameter uncertainty Sensitivity (DerKiureghian, Conte)
- Shear Wall Models
- Solution Strategies
- Pull-out, bond deterioration



#### Advances in Frame Element Formulations

- Force based formulation for 1rst and 2nd order theory (exact internal force distribution)
- Large displacements with corotational formulation
- Mixed force-displacement formulation for frame elements with complex interactions (composite action, pile-soil interaction)
- Robust algorithms of state determination



#### Push-Over of Two Story Frame (Distributions)



#### **Tapered Beam - Curvature Distribution**

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E

E

R



### **Tapered Beam - Bending Moment Distribution**

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#### Advantages of Force Formulation

- equilibrium is satisfied exactly along the element in every iteration; end compatibility is satisfied on convergence
- distributed loads can be readily accommodated
- a single element suffices for the entire member; no mesh refinement is necessary; localization problems are minimized
- formulation is very robust in the presence of strength softening





3 2 1 0 -1 Tip Displacement y (cm) -2 -3 --3 -2 -1 0 1 2 Tip Displacement z (cm)

#### and Variable Axial Load

#### **Biaxial Bending**







#### Low-Moehle Specimen 5: Response in y





#### Low-Moehle Specimen 5: Response in z



### Low-Moehle Specimen 5: Reinforcing Steel Strain History



#### Correlation Studies for ISPRA columns (Bousias et al.)

#### 12 Column Specimens with identical geometry and reinforcing

- S0
  - Uniaxial displacement cycles in x
  - constant axial compression ~ 16% of axial capacity (?)
- S1
  - Alternating uniaxial displacement cycles in x and y
  - constant axial compression ~ 10% of axial capacity

S5, S7

- Different biaxial displacement histories in x and y;
- constant axial compression ~ 12% of axial capacity
- S9
  - Biaxial displacement history in x and y;
  - two levels of axial compression ~ 3%->15% of axial capacity
- S4
  - Displacement in x, Force in y
  - constant axial compression







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## **ISPRA Specimen S1 - Lateral Displacement History**



#### ISPRA Specimen S1 - Flexural Response in x



#### ISPRA Specimen S1 - Flexural Response in y





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### **ISPRA Specimen S1 - Axial Displacement History**





#### ISPRA Specimen S5 - Lateral Displacement History



#### ISPRA Specimen S5 - Flexural Response in x





#### ISPRA Specimen S5 - Flexural Response in y





#### **ISPRA Specimen S5 - Axial Displacement History**

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#### **ISPRA Specimen S9 - Lateral Displacement History**





#### ISPRA Specimen S9 - Flexural Response in x





#### ISPRA Specimen S9 - Flexural Response in y





-6

-8

0

5

10

15

20

25

Load Step

30

35

40

45

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### **ISPRA Specimen S9 - Axial Displacement History**

#### Second order analysis - Large displacements



The co-rotational formulation separates rigid-body modes fill local deformations, using a sill coordinate system that continuously translates and rowith the element as the deformation proceeds.





Lee's Frame







### Parking Garage, 1994 Northridge Earthquake



## Shaking Table Specimen of Shahrooz-Moehle (1987)







#### Shaking Table Specimen El Centro 7.7

6th Floor Displacement Time History to EC7.7L



#### Shaking Table Specimen El Centro 49.3

6th Floor Displacement Time History to EC49.3L



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