Response Spectrum Analysis of Bridges Crossing Faults

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Analysis of bridges crossing faults



Investigation of the problem with consideration of

- Ground motion spatial variability near the fault
- Characteristics of near-fault ground motions
 - Pulse-type nature
 - 'Fling' step

$$E\left[\max|z(t)|\right] = \left[\sum_{k=1}^{m}\sum_{l=1}^{m}a_{k}a_{l}\rho_{u_{k}u_{l}}u_{k,\max}u_{l,\max} + 2\sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{j=1}^{N}a_{k}b_{lj}\rho_{u_{k}s_{lj}}u_{k,\max}D_{l}(\omega_{j},\zeta_{j}) + \sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{i=1}^{m}\sum_{j=1}^{N}b_{ki}b_{lj}\rho_{s_{ki}s_{lj}}D_{k}(\omega_{i},\zeta_{i})D_{l}(\omega_{j},\zeta_{j})\right]^{1/2}$$

- *k*, *l* indices for support motions (m = total number of support DOF)
- *i*, *j* indices for modes (N = total number of modes)

$$E\left[\max|z(t)|\right] = \left[\sum_{k=1}^{m} \sum_{l=1}^{m} a_{k}a_{l}\rho_{u_{k}u_{l}}u_{k,\max}u_{l,\max} + 2\sum_{k=1}^{m} \sum_{j=1}^{m} \sum_{j=1}^{N} a_{k}b_{lj}\rho_{u_{k}s_{lj}}u_{k,\max}D_{l}(\omega_{j},\zeta_{j}) + \sum_{k=1}^{m} \sum_{l=1}^{m} \sum_{j=1}^{m} \sum_{i=1}^{N} \sum_{j=1}^{N} b_{ki}b_{lj}\rho_{s_{ki}s_{lj}}D_{k}(\omega_{i},\zeta_{i})D_{l}(\omega_{j},\zeta_{j})\right]^{1/2}$$

- *k*, *l* indices for support motions (m = total number of support DOF)
- *i*, *j* indices for modes (N =total number of modes)
- a_k, b_{ki} effective response factors associated with support DOF k and mode i – functions of structural mass and stiffness properties

$$E\left[\max|z(t)|\right] = \left[\sum_{k=1}^{m}\sum_{l=1}^{m}a_{k}a_{l}\rho_{u_{k}u_{l}}u_{k,\max}u_{l,\max} + 2\sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{j=1}^{N}a_{k}b_{lj}\rho_{u_{k}s_{lj}}u_{k,\max}D_{l}(\omega_{j},\zeta_{j})\right]^{1/2}$$
$$+ \sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{i=1}^{N}\sum_{j=1}^{N}b_{ki}b_{lj}\rho_{s_{ki}s_{lj}}D_{k}(\omega_{i},\zeta_{i})D_{l}(\omega_{j},\zeta_{j})\right]^{1/2}$$

k, *l* indices for support motions (m = total number of support DOF)

i, *j* indices for modes (N = total number of modes)

 a_k, b_{ki} effective response factors associated with support DOF k and mode i - functions of structural mass and stiffness properties

 $u_{k,\max}$ maximum ground displacement at support DOF k

 $D_k(\omega_i, \zeta_i)$ ordinate of response spectrum at support DOF k for mode i

$$\mathbb{E}\left[\max|z(t)|\right] = \left[\sum_{k=1}^{m}\sum_{l=1}^{m}a_{k}a_{l}\rho_{u_{k}u_{l}}u_{k,\max}u_{l,\max} + 2\sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{j=1}^{N}a_{k}b_{lj}\rho_{u_{k}s_{lj}}u_{k,\max}D_{l}(\omega_{j},\zeta_{j}) + \sum_{k=1}^{m}\sum_{l=1}^{m}\sum_{i=1}^{m}\sum_{j=1}^{N}b_{ki}b_{lj}\rho_{s_{ki}s_{lj}}D_{k}(\omega_{i},\zeta_{i})D_{l}(\omega_{j},\zeta_{j})\right]^{1/2}$$

 $\rho_{u_k u_l}$ correlation coefficient between displacements at support DOFs k and l

- $\rho_{u_k s_{lj}}$ correlation coefficient between the displacement at support DOF *k* and the response of mode *j* to the ground motion at support DOF *l*
- $\rho_{s_{ki}s_{lj}}$ correlation coefficient between the response of mode *i* to the ground motion at support DOF *k* and the response of mode *j* to the ground motion at support DOF *l*

The three sets of correlation coefficients are functions of $u_{k,\max}$, $D_k(\omega_i, \zeta_i)$, ω_i , ζ_i and a **coherency function** defining the spatial variability of the ground motion field.

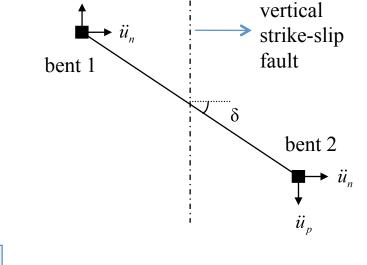
Coherency function for the case of a strike-slip fault

Transverse support motions

 $\ddot{u}_1^Y(t) = \ddot{u}_p \delta(t) \cos(\theta) \sin \theta \delta t$ $\ddot{u}_2^Y(t) = -i \delta_p(t) \cos(\theta) \sin \theta \delta i_n t$

Assuming FN and FP components are statistically independent:

$$\gamma_{12}^{Y}(\omega) = \frac{-G_{\vec{u}_{p}\vec{u}_{p}}(\omega)(\cos\delta)^{2} + G_{\vec{u}_{n}\vec{u}_{n}}(\omega)(\sin\delta)^{2}}{G_{\vec{u}_{p}\vec{u}_{p}}(\omega)(\cos\delta)^{2} + G_{\vec{u}_{n}\vec{u}_{n}}(\omega)(\sin\delta)^{2}}$$



 $G_{\ddot{u}_p\ddot{u}_p}(\omega), G_{\ddot{u}_n\ddot{u}_n}(\omega)$ obtained in terms of corresponding response spectra

Input ground motions

Concerns

- □ Modification of response spectra to account for near-fault effects
- The narrow-band and distinctly non-stationary nature of near-fault ground motions violates fundamental assumptions of the MSRS method
- Lack of strong motion records at distances less than a hundred meters from the fault
- use of simulated ground motions
- use of ground motions recorded at larger distances
- Ground motions from the NGA database do not include the 'fling step' due to baseline correction

Evaluation of static offset

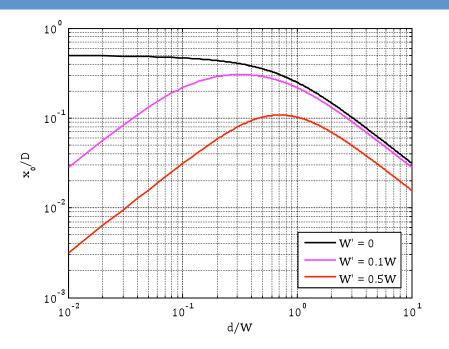
□ If the fault ruptures to the surface

$$x_o = \frac{D}{2} - \frac{D}{2} \tan^{-1}(\frac{d}{W})$$

• otherwise

$$x_o = \frac{D}{\pi} \left[\tan^{-1}(\frac{d}{W'}) - \tan^{-1}(\frac{d}{W}) \right]$$

- *W* : Fault width
- *W*: Depth to the top of fault rupture
- *d* : Distance from the fault
- *D* : Average fault slip



'Fling' step time histories

- Velocity $\dot{x}(t) = ct^{\zeta} e^{-t/a}$
- *c* : normalizing parameter so that $\int x(t)dt = x_o$
- $a = T_R / 4$, T_R : Rise time
- ζ : controls the high-frequency decay rate, $\zeta = 1 \rightarrow$ Brune source

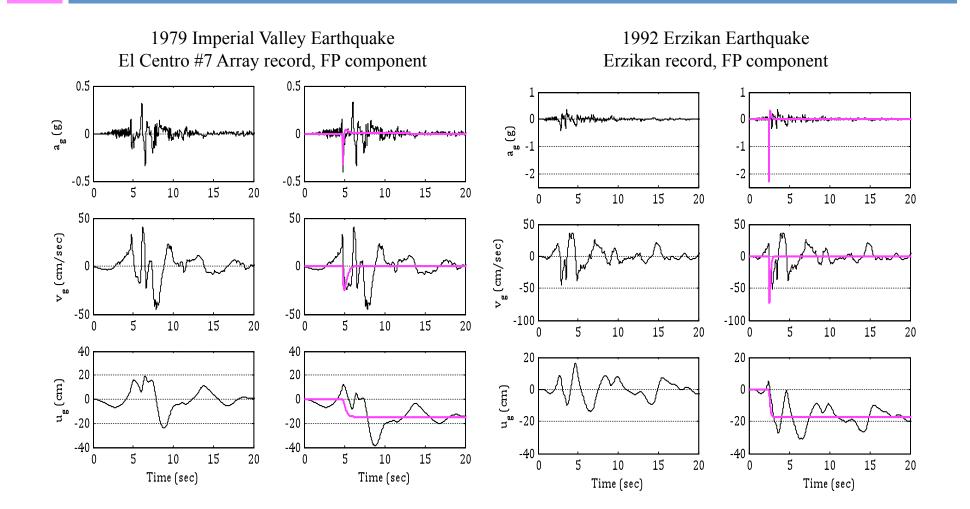
Displacement

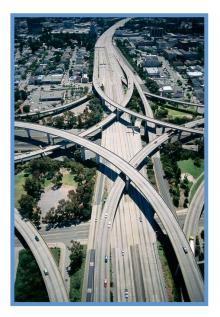
 $x(t) = -c(at + a^{2})e^{-t/a} + ca^{2}$

□ Acceleration

$$\ddot{x}(t) = c \left(1 - \frac{t}{a}\right) e^{-t/a}$$

Example applications





Thank you

Questions?

