

Mitigation of Seismic Risk pertaining to Non-Ductile Concrete Buildings using Seismic Risk Maps Matthew J. Zahr, UC Berkeley, <u>bokie89@berkeley.edu</u> Nicolas Luco, Research Structural Engineer and Hyeuk Ryu, Postdoctoral Researcher USGS Geologic Hazards Science Center, Golden, CO **COMPONENTS OF RISK**

ABSTRACT

This poster presents the creation, improvement, and application of a web-based seismic risk map tool developed at the USGS in Golden, CO (http://earthquake.usgs.gov/research/hazmaps/risk).

Reinforced concrete buildings built prior to implementation of modern seismic code standards in 1976 behave in a non-ductile manner under seismic loading, potentially leading to catastrophic failure. The high degree of seismic activity in the western United States makes retrofitting such non-ductile concrete buildings a necessity. Due to the associated cost and time, it is virtually impossible to use a brute force approach to mitigate the seismic risk created by these older concrete buildings. This has motivated the development and improvement of a web-based seismic risk map tool as a way to quantify seismic risk. This tool provides a means to quickly identify the regions of the US where non-ductile concrete buildings are at a high risk of failure. Furthermore, with an inventory of non-ductile concrete buildings for a particular area, the buildings at the highest risk in that area can be pinpointed for seismic retrofit.

MOTIVATION

In 1971, an earthquake in the San Fernando area caused an estimated \$500 million in property damage and 65 deaths, due mainly to the collapse of some older concrete buildings. Post-earthquake investigations revealed that a vast majority of the buildings that collapsed were built with too much spacing between stirrups and inadequate flexural reinforcement, which caused them to behave in a non-ductile manner and fail catastrophically. In response to the damage and casualties resulting from the 1971 San Fernando earthquake, building codes were updated to increase the ductility of concrete buildings during the cyclic loading caused by earthquakes.

Concrete buildings constructed after 1976 have a high degree of ductility, which lessens their risk of catastrophic failure. However, this problem still exists with buildings in the western United States constructed prior to the building code revisions. This introduces the necessity of seismic retrofit.

Due to the large number of these substandard buildings and the high cost of retrofit, an efficient strategy to identify high-risk buildings is crucial.

METHOD & MATERIALS

MATLAB & Google Earth **USGS Hazard Data** USGS or User-Specified Fragility Data **USGS or User-Specified** Vulnerability Data

A MATLAB code was created to generate seismic risk maps in the KML file format to be viewed using Google Earth. This code serves as a first-step toward animproved web-based seismic risk map tool. The MATLAB code will be translated into JAVA and incorporated into the existing USGS website at a later date.

BACKGROUND

HAZUS

HAZUS is a multi-hazard risk assessment software (http://www.fema.gov/plan/prevent/hazus/). HAZUS categorizes buildings in terms of construction material, height, lateral force-resisting system, level of seismic design, and occupancy type. The USGS risk maps currently consider only these building categories. The tables below and in the next column present a brief overview of some of the aspects of the HAZUS building categories (including only concrete structural types and excluding occupancy type). For more information on HAZUS, refer to the HAZUS Technical Manual.

HAZUS does not explicitly define a structural type corresponding to non-ductile concrete, but we assume that when any concrete structure is coupled with a pre-code level, it behaves in a non-ductile manner under seismic loading. Likewise, concrete structures built at a high-code level are considered ductile.

Label	Description	Height		
	Description	Name	# Stories	
C1L		Low-Rise	1 - 3	
C1M	Concrete Moment Frame	Mid-Rise	4 - 7	
C1H		High-Rise	8+	
C2L		Low-Rise	1 - 3	
C2M	Concrete Shear Walls	Mid-Rise	4 - 7	
C2H		High-Rise	8+	
C3L	Concrete Frame with	Low-Rise	1 - 3	
C3M	Unreinforced	Mid-Rise	4 - 7	
C3H	Masonry Infill Walls	High-Rise	8+	

Partial Description of HAZUS Building Categories

Seismic Level of Design	Description	Affect on HAZUS Concrete Structures	
Pre-Code	Minimal Strength Minimal Ductility	Non-Ductile	
High-Code	High Strength High Ductility	Ductile	

NEHRP Site Class Definition

Site Class	Soil Profile Name	Soil shear wave velocity, V _s 30 (m/s)
A	Hard rock	V _S 30 > 1500
В	Rock	1500 ≥ V _S 30 ≥ 760
С	Very dense soil and soft rock	760 > V _s 30 > 360
D	Stiff soil profile	360 ≥ V _s 30 > 180
E	Soft soil profile	180 ≥ V _S 30

SITE CLASS

• NEHRP defines site class in terms of shear wave velocity values to a depth of 30 meters ($V_{S}30$)

• Approximate $V_{s}30$ values can be determined from topography (Wald & Allen 2007) to estimate a site class distribution in the US (see Figure in next column)

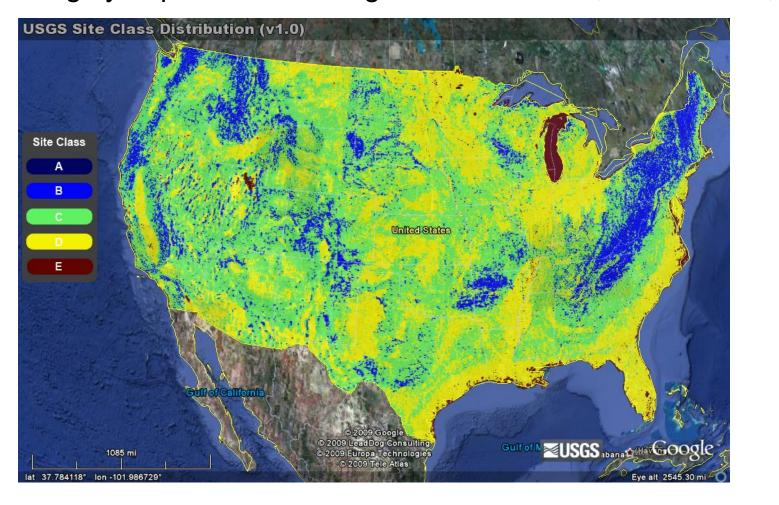
FURTHER INFORMATION

Refer to: Zahr, M. J. (2009). "Mitigation of Seismic Risk pertaining to Non-Ductile Reinforced Concrete Buildings using Seismic Risk Maps." Or Contact:

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HAZARD

•Mean annual frequency of ground motion (spectral acceleration at a particular period of oscillation) at a particular location exceeding some value. • Highly dependent on the ground conditions, or site class, at a particular site



FRAGILITY/VULNERABILITY

• HAZUS divides building damage into four states. Due to our interest in catastrophic failure of non-ductile concrete structures, the complete damage state will be emphasized. • Fragility is the probability of exceeding a particular damage state given a certain ground motion (spectral acceleration at a particular period of oscillation) for a specific building. • Vulnerability is the expected value of the loss ratio (repair cost/replacement cost) for a given spectral acceleration. • The USGS fragility/vulnerability functions used in the risk maps were derived by Karaca and Luco (2009).

	Damage State	Descrip	tion	Quantification
	Slight	Flexural or Shear in some beams/co within jo	lumns near or	~0%-5% of Replacement Cost
	Complete	Structure is coll imminent dange due to brittle fai ductile eler	r of collapse lure of non-	~100% of Replacement Cost
Fragility Fun			Vul	nerability F Concre
	te Structu	C3: Concrete Frame with Unreinforced	Cl: Co	ncrete Moment Frame
$STR = 1 \\ 0.8 \\ 0.6 \\ 0.4 \\ 0.2 \\ 0.2 \\ 0.0^2 \\ 10^{-2} \\ 10^{-1} \\ 10^0 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^2 \\ 10^2 \\ 10^2 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10^1 \\ 10^0 \\ 10$	STR 0.8 0.6 0.4 0.4 0.4 0.2 0 0 0 0 0 0 0 0 0 0 0 0 0	Mansonry Infill Walls STR 10 ⁻¹ 10 ⁰ 10 ¹ Sa(T=0.5sec)	Low see 0.5 Rise 0.5 Use 0.5 Use 0.5 Use 0.5	STR 10 ⁰ 10 ¹ 10 ¹ SA(T=0.5sec)
$\begin{array}{c} 1 \\ 0.8 \\ 0.6 \\ 0.4 \\ 0.2 \\ 0 \\ 10^{-2} \\ 10^{-1} \\ 10^{0} \\ 10^{0} \\ 10^{0} \\ 10^{1} \\ 10^{0} \\ 10^{1} \\ 10^{2} \\ 10^{2} \\ 10^{2} \\ 10^{1} \\ 10^{2} $	1 0.8 ₩ 0.6 0.4 0.2 0 ¹ 10 ⁰ 10 ¹ 10 ³ Sa(T=1.0sec)	10 ⁻¹ 10 ⁰ 10 ¹ Sa(T=1.0sec)	Mid Rise 9 0.5 0.5 0.5 0 0 10 ⁻¹	10 ⁰ 10 ¹ 10 ¹
	1 0.8 0.6 0.4 0.4 0.2 0 0 10 ¹ 10 ⁰ 10 ¹ 10 ³ Sa(T=2.0sec)	Pre Code — — Low Code — — Low Code — — High Code 10 ⁻¹ 10 ⁰ 10 ¹ Sa(T=2.0sec)	High ss 0.5 Rise 0.5 U 10-1	10 ⁰ 10 ¹ 10 ⁻¹ SA(T=2sec)
Con	iplete Damage	R	ISK	
ROM FRAGI			D D	

• Ha cation of Total Probability Theo ain damage state • The Poisson Process can then be used to extend the time interval, but due to the assumptions inherent in the Poisson Process, this is an approximation

Risk Summation using	$\lambda[DS_i]$
Fragility Functions	L []

Poisson Process to Extend Time Interval

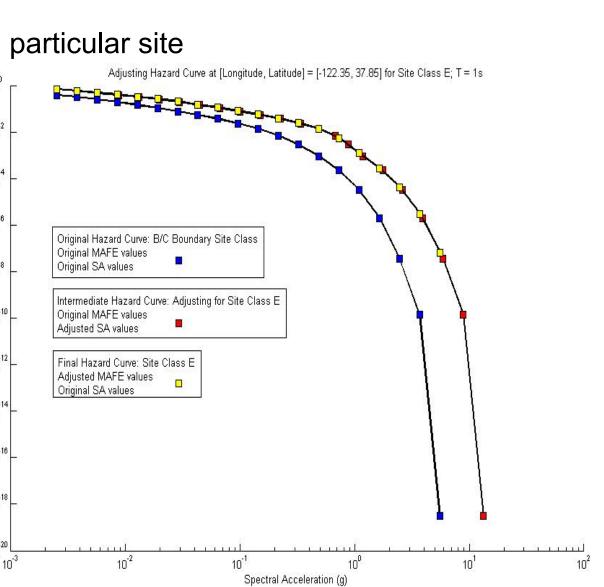
 $P(DS \ge ds \text{ in t years}) = 1 - exp(-\lambda t)$ λ = mean annual frequency of exceedance (MAFE)

FROM VULNERABILITY AND HAZARD

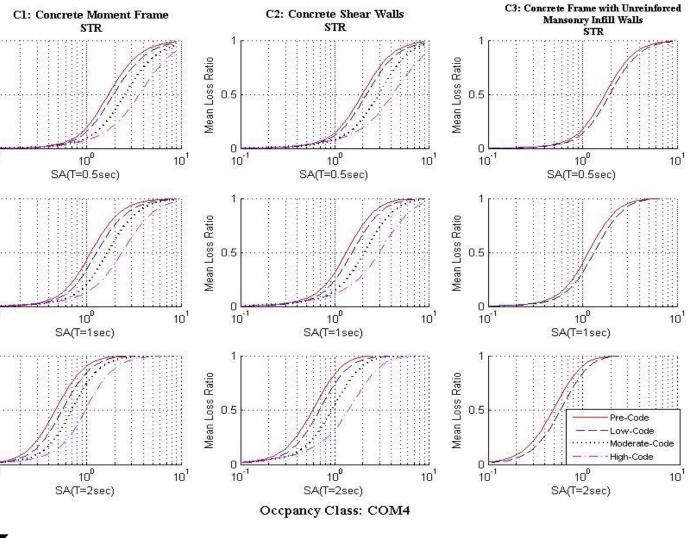
• Hazard and vulnerability information can also be combined using the risk summation, but this will define risk in terms of an expected annual loss ratio • The expected annual loss ratio can be multiplied by the value of a building to quantify risk in terms of expected annual monetary loss, but this approach is not presented in this poster

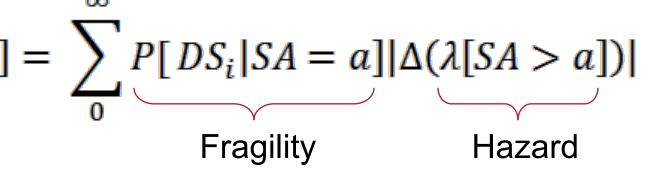
E[LR] =

Risk Summation using Vulnerability Functions



unctions for HAZUS te Structures





$$\sum_{0} E[LR|SA = a]|\Delta(\lambda[SA > a])|$$

Vulnerability Hazard

ORIGINAL RISK MAP TOOL

•Users specify structural type and height, construction material, planning horizon, code level, and damage state

• A general risk map is made with the assumption that this set of parameters exists at every point on the grid of the continental US

• Hazard curves for reference Site B/C only

UPDATED RISK MAP TOOL

 Designed to alleviate restrictions of the original options

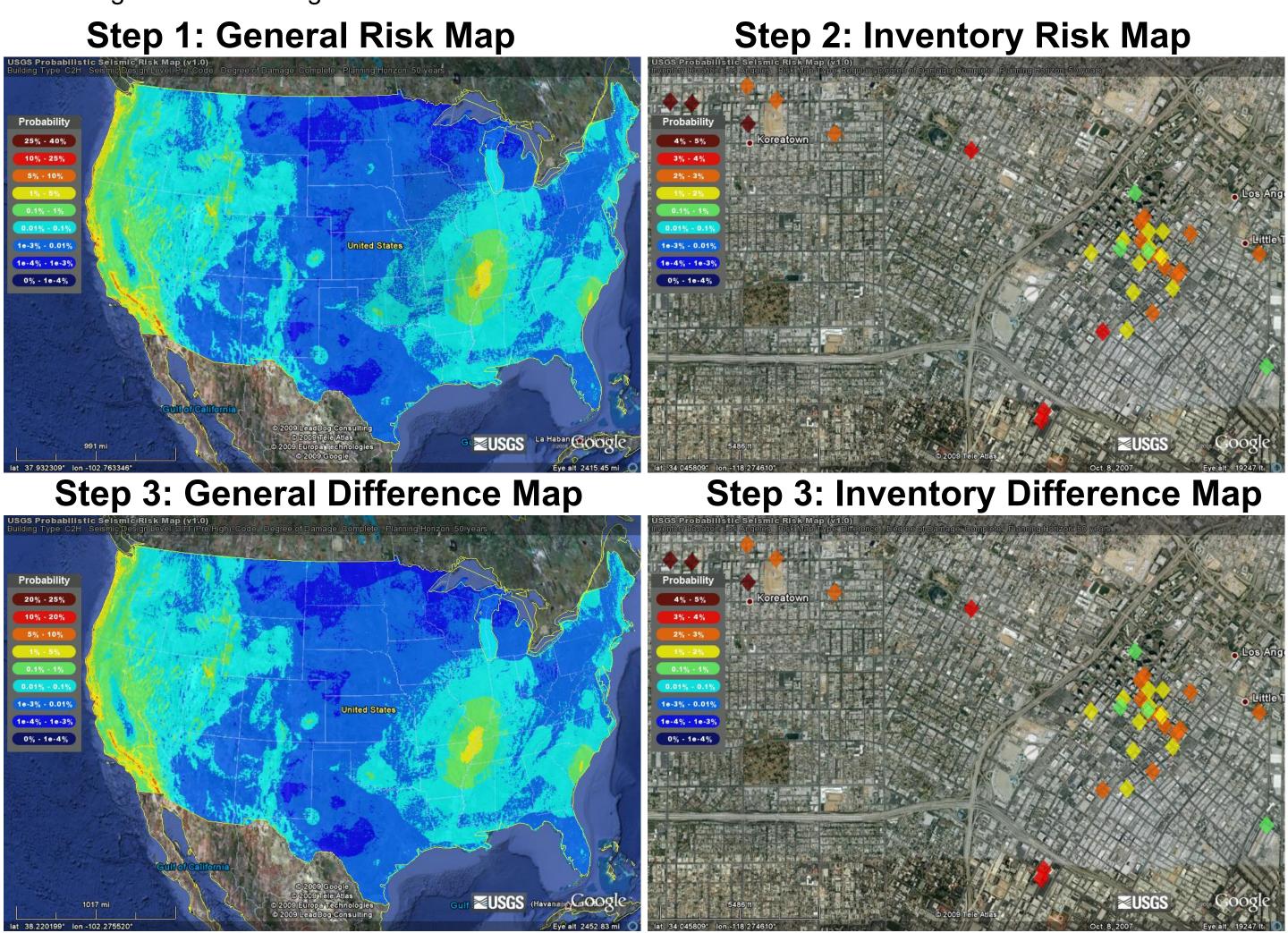
• Allows for, but doesn't require, user-specified: Inventory and/or Fragility/Vulnerability Data • New Features:

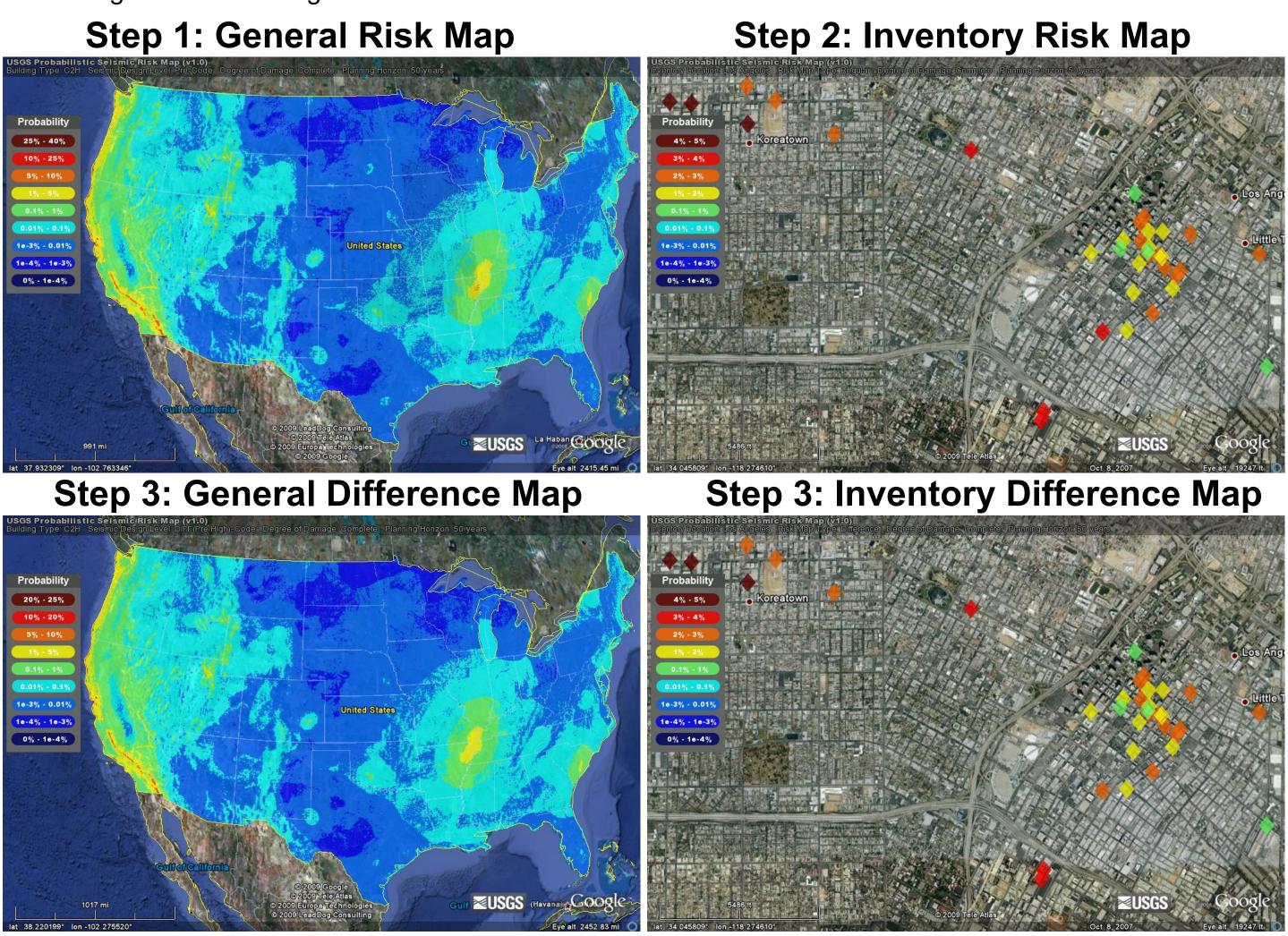
- Difference Maps
- Loss Ratio Maps
- Inventory-Specific Maps

RETROFIT INVENSTIGATION METHODOLOGY

The updated USGS risk map tool described above can be used to carry out the following systematic approach to retrofitting non-ductile concrete buildings: 1) Locate the areas in the continental US that pose the greatest seismic risk using the General Risk Map

- option of the updated tool
- pinpoint the highest risk buildings in the region
- regions and buildings

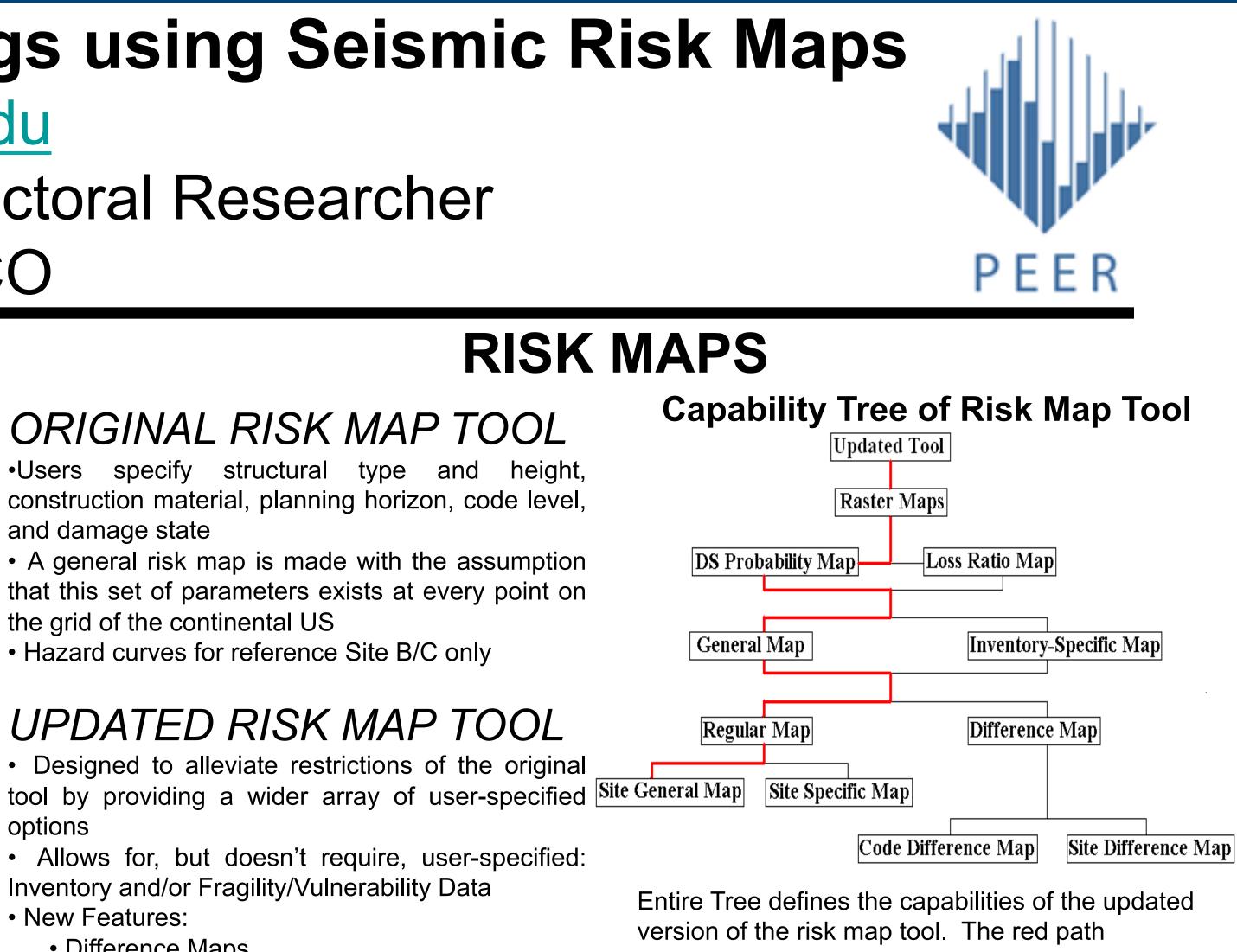




The inventory used in the maps on the right is a sample of a comprehensive non-ductile concrete building inventory in the Los Angeles area being developed by Anagnos et. al.

I would like to thank Nicolas Luco and Hyeuk Ryu for their guidance as I updated the USGS risk map tool and created its associated documentation. I would also like to thank Professor Anagnos et. al. for providing the sample non-ductile concrete inventory for the LA area. Finally, I would like to thank the PEER Center and Nicolas Luco for the tremendous opportunity I was given as a 2009 PEER intern.

Faison, H. (2008). "Magnitude 5.4 Earthquake in Los Angeles, July 29, 2008." PEER News, <http://peer.berkeley.edu/news/2008/la_eq_july_2008.html> (August 05, 2009). Federal Emergency Management Agency (2004), "NEHRP Recommended Provisions for Seismic Regulations for New Buildings and Other Structures, Part 1: Provisions," FEMA 450-1/2003 Edition, Washington, DC. FEMA. (2006). "HAZUS-MH MR-3 Technical Manual." Federal Emergency Management Agency, Washington, D.C. Karaca, E & Luco, N (2009), "Development of Seismic Hazard Compatible Building Fragility Functions: Application to HAZUS Building Types," Under revision for publication in Earthquake Spectra. Ryu, H., Luco, N., Baker, J. W., and Karaca, E. (2008). "Converting HAZUS Capacity Curves to Seismic Hazard-Compatible Building Fragility Functions: Effect of Hysteretic Models." Proceedings of The 14th World Conference on Earthquake Engineering, Bejing, China. "SCEDC | San Fernando Earthquake (1971)." Southern California Earthquake Data Center Home, (August 05, 2009).



outlines the capabilities of the original risk map tool.

2) Input inventories (e.g. of non-ductile concrete buildings) for these areas into the updated tool to 3) Using the difference map option of the updated tool, quantify the benefits of the retrofit for these

ACKNOWLEDGMENTS

LITERATURE CITED