Performance-based Evaluation of the Seismic Response of Bridges with Foundations Designed to Uplift

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# **3** Questions

- Can foundation rocking be considered as an alternative seismic design method of bridges resulting in reduced:
   i) post-earthquake damage, ii) required repairs, and iii) loss of function ?
- **2.** What are the ground motion characteristics that can lead to overturn of a pier supported on a rocking foundation?
- **3.** Probabilistic performance-based earthquake evaluation ?

# "Fixed" Base Design



Susceptible to significant postearthquake damage and permanent lateral deformations that:

• Impair traffic flow

• Necessitate costly and time consuming repairs

# **Design Using Rocking Shallow Foundations**

*"Fixed" base pier* 

Pier on **rocking** shallow foundation



# **Design Using Rocking Pile Caps**

"Fixed" base pier

Pier on rocking pile-cap



# **Design Using Rocking Pile-Caps**

Pile-cap simply supported on piles

Pile-cap with sockets



# **Rocking Foundations - Nonlinear Behavior**



Rotation, Θ

## **Nonlinear Behavior Characteristics**

Force, F





**Fixed**-base or

shallow foundation with extensive soil inelasticity Shallow foundation with limited soil inelasticity

Rocking pile-cap or shallow foundation on elastic soil

#### **SDOF Nonlinear Displacement Response**

#### Mean results of 40 near-fault ground motions



# Numerical Case Study of a Bridge

An archetype bridge is considered and is designed with:
i) fixed base piers
ii) with piers supported on recking foundations

*ii)* with piers supported on rocking foundations

Analysis using 40 near-fault ground motions





#### **Computed Response of a Bridge System**

Archetype bridge considered – Tall Overpass



 $\bullet$ 



- 5 Spans
- Single column bents
- Cast in place box girder

- Column axial load ratio  $N / f_c A_g = 0.1$
- Longitudinal steel ratio  $\rho_l = 2\%$

# **Designs Using Rocking Foundations**

 $\begin{array}{c} 39 \text{ ft} \\ 6 \text{ ft} \\ 50 \text{ ft} \\ \end{array} \xrightarrow{} D = 6 \text{ft} \\ \overrightarrow{D} = 24 \text{ ft} (4D) \end{array}$ 

Shallow foundation



**Rocking Pile-Cap** 

Soil ultimate stress  $\sigma_u = 0.08$  ksi

 $B = 18 \, ft \, (3D)$ 

$$FS_v = A\sigma_u / N = 5.4$$

# Modeling of Bridge

#### **OPENSEES 3-dimensional model**





# Monotonic Behavior – Individual Pier



## Ground Motions Considered – Response Spectra , 2% Damping



# **Computed Response of Bridge**

 $\Delta$ : total drift

 $\Delta_{f}$ : drift due to pier bending

*z:* soil settlement at foundation edge







# **Computed Bridge Response**



# Ground motion characteristics that may lead to overturn ?

Ground motions with strong pulses (especially low frequency) that result in significant nonlinear displacement demand



Rocking response of rigid block on rigid base to pulse-type excitation Zhang and Makris (2001)



#### Near Fault Ground Motions and their representation using Trigonometric Pulses



#### Conditions that may lead to overturn



Minimum a, at different T, that results in overturn ?

#### Conditions that may lead to overturn



#### Conditions that may lead to overturn



#### **Probabilistic Performance Based Earthquake Evaluation (PBEE)**

The PEER methodology and the framework of Mackie et al. (2008) was used for the PBEE comparison of the fixed base and the rocking designs.

- Ground Motion Intensity Measures [Sa (T<sub>1</sub>)]
- Engineering Demand Parameters (e.g. Pier Drift )
- Damage in Bridge Components
- Repair Cost of Bridge System

#### PBEE Evaluation – Damage Models (Mackie et al. 2008)



# **PBEE Evaluation Foundation Damage Model**



Normalized Edge Settlement z / z<sub>yield</sub>



# **PBEE – Disaggregation of Cost**

#### Fixed Base Bridge



# **PBEE – Disaggregation of Cost**

#### Bridge with Shallow Foundations B=4D



