Design practice for pile foundations in laterally spreading ground

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 - Being developed for ODOT and WashDOT



Research developments

Last 20 years have seen considerable progress in understanding, analyzing, and designing for lateral spreading effects.





Simulation

Hierarchy of 2-D and 3-D modeling capabilities developed; proprietary, commercial, and open source.





Validation

Dynamic centrifuge, shake table, and field tests addressed knowledge gaps for deep foundations.









Development of design guidance

Parametric studies to evaluate ESA methods against case histories, physical modeling data, and nonlinear dynamic analyses.



DDC01 in Large Kobe Motion



Exchanges

♦ ASCE Geotechnical Special Publication No. 145, 2006.





Recommended Design Practice



PACIFIC EARTHQUAKE ENGINEERING RESEARCH CENTER

Recommended Design Practice for Pile Foundations in Laterally Spreading Ground

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Design practice for lateral spreading

- Steps for design or performance evaluation of a bridge include:
 - Design/evaluate for inertia loading that would occur in the absence of liquefaction.
 - Evaluate the potential for liquefaction and associated ground displacements.
 - Design/evaluate for the lateral spreading and inertia demands that would occur if liquefaction is triggered.



Local and global analyses

Stidge pinning effects on ground displacements can be modeled using different approaches for different local or global analysis methods.





Estimating lateral spreading displacements

- Three key evaluation steps
 - Site characterization
 - Liquefaction triggering
 - Ground deformations
- Site characterization and evaluation of liquefaction susceptibility
 - Use appropriate mix of SPT and CPT
 - Detailed cross-section showing in situ data
 - Key factor: spatial extent and continuity



Liquefaction Triggering

Potential for liquefaction triggering in susceptible soils

- Comparison of CSR (Seed & Idriss 1971) and CRR
- For CRR, use Youd et al (2001) for now
 - Until consensus is reached in profession
 - Cetin et al ('04), Idriss & Boulanger ('06), Moss et al ('06)
- Fines consideration
 - Boulanger and Idriss (2006)
 - Silts and clays with PI≥7, treat as cohesive soil
 - For PI<7, evaluate using SPT/CPT procedures</p>
 - Or do lab testing
 - Bray and Sancio (2006)
 - For 7≤PI≤20, do lab testing



Consequences of Liquefaction

Residual strength

 Instability of a slope or embankment due to loss of shear strength in liquefied zones.

Strains

- Lateral spreading of level or sloping ground
- Displacements
 - Settlement due to one-dimensional reconsolidation of liquefiable soil



Residual strength estimation

Estimation of residual shear strength of a liquefied soil for slope/embankment instability investigation



(Idriss and Boulanger 2007)



Liquefaction-induced ground displacements

- Free-field lateral spreading displacement can be estimated using different approaches
 - Empirical relationships
 - Specific to the cases that relationships were developed
 - Youd et al. 2002, Bardet et al. 2002, Rauch & Martin 2000
 - Integration of shear strain profiles with depth
 - Estimated in conjunction with SPT- and CPT- based liquefaction analysis (Zhang et al. 2004)
 - Newmark sliding block analyses
 - Can depend heavily on the residual strength
 - Nonlinear dynamic analyses



ESA with imposed soil displacements

- Soil displacement
 - Magnitude
 - Shape
- Soil springs
 - Nonliquefied layers
 - Liquefied layers
- Inertia loads
- Kinematic loading
 - Loads from a nonliquefied crust
- Inertial and lateral spreading loading combinations



 W_{ss}



p-y behavior in liquefied sand

ESA requires crude approximation of complex behavior



m_p
0.0 to 0.1
0.05 to 0.2
0.1 to 0.3
0.2 to 0.5



Reduction of ultimate lateral loads in the overlying or underlying nonliquefied layers





- Pile group interaction effects
 - Apply group p-multipliers underlying nonliquefied layer
 - No p-multiplier for liquefied soil
 - No group effects for nonliquefied crusts





- Loads from nonliquefied crusts
 - Passive earth pressure
 - Practical problems, case B will result smaller foundation loads.
 - Controlling mechanisms depends on size and number of piles, thickness ^{crust} of crust





Ref. Design examples by Tom Shantz



Displacement demands

Combined lateral spreading & inertia loading:





Displacement demands Combined lateral spreading & inertia loading: Longitudinal Δ_{CG} ← Δ_{CG} Δ_I $\Delta_{ heta_{cap}}\Delta_{I}$ Δ_{cap} Δ_{cap} Original column Ð C.G. C.L. Ð Original column C.L. Δ_{cap} Δ_{cap} . Ө_{сар}



Displacement demands

Lateral spreading with deck restrained





Design of piles in Approach Embankments

- Pile pinning analyses
 - Slope stability analyses of the embankment for a range of restraining forces
 - Consider range of failure surfaces



Design of piles in Approach Embankments

Compatibility of embankment and pile displacement





Global bridge response

- Provide more realistic evaluation of the distribution of force and displacement demands than from local analyses of individual bent or frames
- Ordinary bridge without liquefaction effect
 - Linear elastic- ESA
 - Linear elastic dynamic analysis for complicated case
- Global analyses for the effect of liquefaction are warranted when the subsurface conditions and expected liquefaction-induced ground displacement vary substantially along the bridge alignment







2010 Chile Earthquake

Darfield Earthquake

E 172°

30

2010-11 New Zealand Earthquakes



Thank You...

