

TBI



Tall Buildings Initiative Guidelines for Performance- Based Seismic Design of Tall Buildings

Version 1.0
November 2010

Developed by
Pacific Earthquake Engineering Research Center
Report No. 2010/05

Sponsored by
Charles Pankow Foundation
California Seismic Safety Commission
California Emergency Management Agency
Los Angeles Department of Building and Safety



(ATC 58)



Next-Generation Performance-Based Seismic Design Guidelines

Program Plan for New and Existing Buildings

FEMA-445 / August 2006



FEMA



TBI Partners

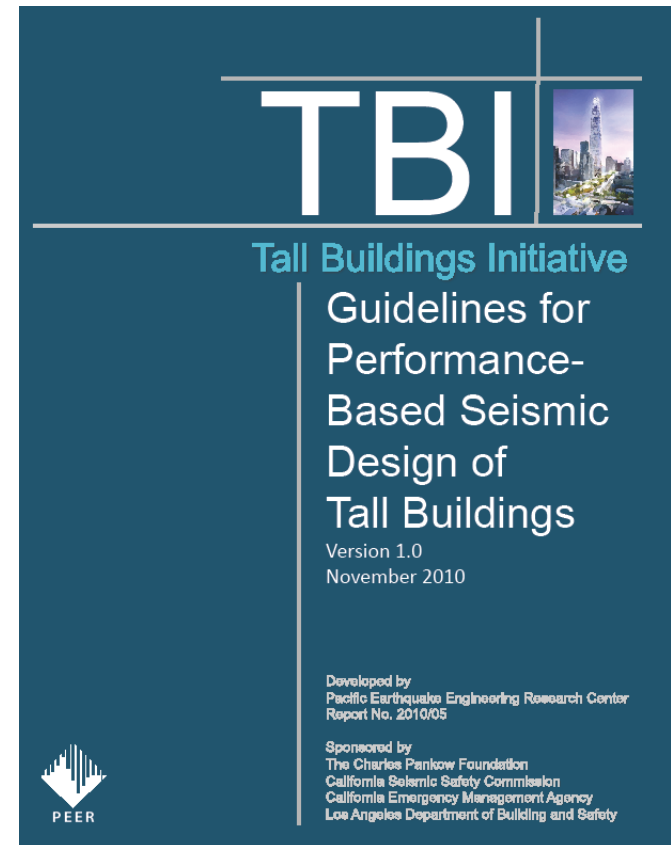
- Applied Technology Council
- California Geological Survey
- California Emergency Management Agency
- California Seismic Safety Commission
- FEMA
- Los Angeles Dept. of Buildings & Safety
- Los Angeles Tall Buildings Council
- National Science Foundation
- Pankow Foundation
- PEER
- San Francisco Building Department
- SCEC
- SEAOC
- USGS

TBI thanks

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- TBI Project Advisory Committee (TPAC)
 - N. Abrahamson, Y. Bozorgnia, R. Hamburger, H. Krawinkler, J. Moehle, and F. Naeim
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- TBI Guidelines Development Team
 - R. Hamburger, J. Moehle, Y. Bozorgnia, C.B. Crouse, R. Klemencic, H. Krawinkler, J. Malley, F. Naeim, J. Stewart
- Case Studies
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TBI Guidelines Chapters

1. Introduction
2. Performance objectives
3. Design process
4. Design criteria documentation
5. Seismic input
6. Preliminary design
7. Service level evaluation
8. MCE level evaluation
9. Presentation of results
10. Project review



1. Introduction



2. Seismic performance objectives

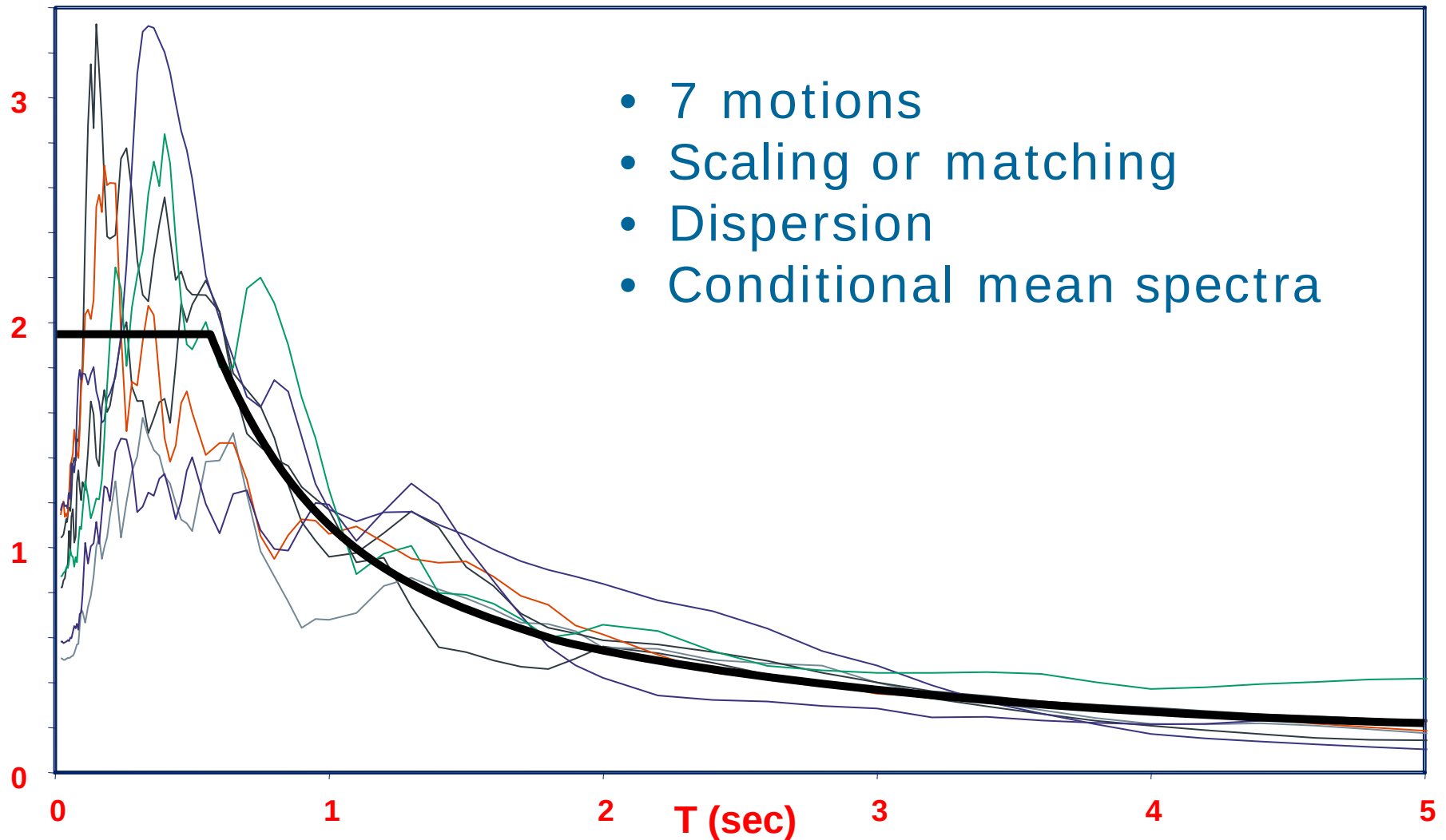
after SEAOC Vision 2000

		Earthquake Performance Level			
		Fully Operational	Operational	Life Safe	Near Collapse
Earthquake Design Level	Frequent (43 years)	Basic Objective	Unacceptable	Unacceptable	Unacceptable
	Occasional (72 years)	Essential/Hazardous Objective	Basic Objective	Unacceptable	Unacceptable
	Rare (475 years)	Safety Critical Objective	Essential/Hazardous Objective	Basic Objective	Unacceptable
	Very Rare (975 years)	Not Feasible	Safety Critical Objective	Essential/Hazardous Objective	Basic Objective

Serviceability
check

Stability
check

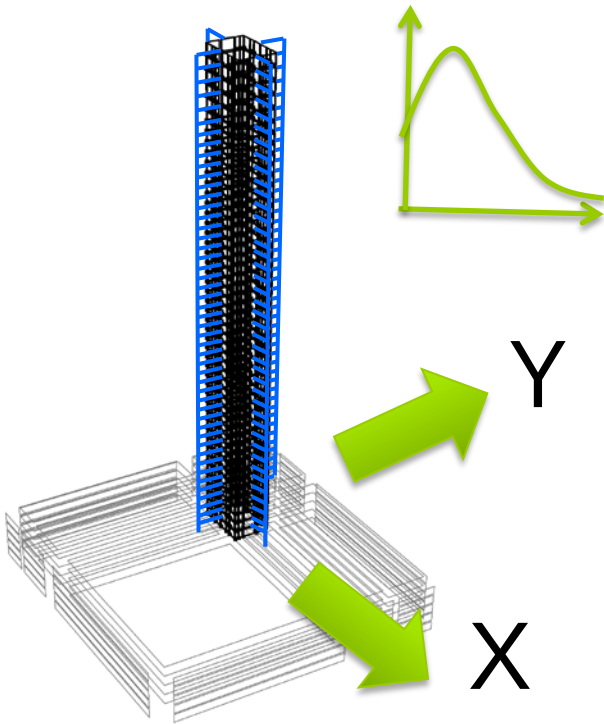
5. Ground motion selection and scaling



Service Level Analysis

- Performance Goal: Minor structural damage
- Model
 - All components that provide strength and stiffness
- Analysis
 - Linear response spectrum analysis
 - 43-yr return period, 2.5% damping
- Acceptance criterion
 - Effectively linear response

Load Combinations & Acceptance



$$Q = D + L_{\text{exp}} + 1.0E_X + 0.3E_Y$$

$$Q = D + L_{\text{exp}} + 0.3E_X + 1.0E_Y$$

$$L_{\text{exp}} = 0.25L$$

$$Q < 1.5\phi C_n$$

$$\text{Story Drift} < 0.005h$$

- C_n = nominal capacity (per code)
- ϕ = resistance factor per ACI or AISC

Maximum Considered Level Analysis

- Performance goals:
 - Minor implicit risk of collapse
 - Modest residual drift
 - Limited potential for failure of cladding
- Model
 - All components that provide strength and stiffness
- Analysis
 - Nonlinear dynamic analysis
 - MCE level
- Acceptance criteria
 - Force and deformation demands within limits
 - Transient and residual drifts within limits

Transient and residual drift

- Transient story drift
 - Mean of 7 runs < 0.03
 - Maximum of any run < 0.045
- Residual story drift
 - Mean of 7 runs < 0.01
 - Maximum of any run < 0.015

Non-ductile actions

$$F_u \leq \phi F_{n,e}$$

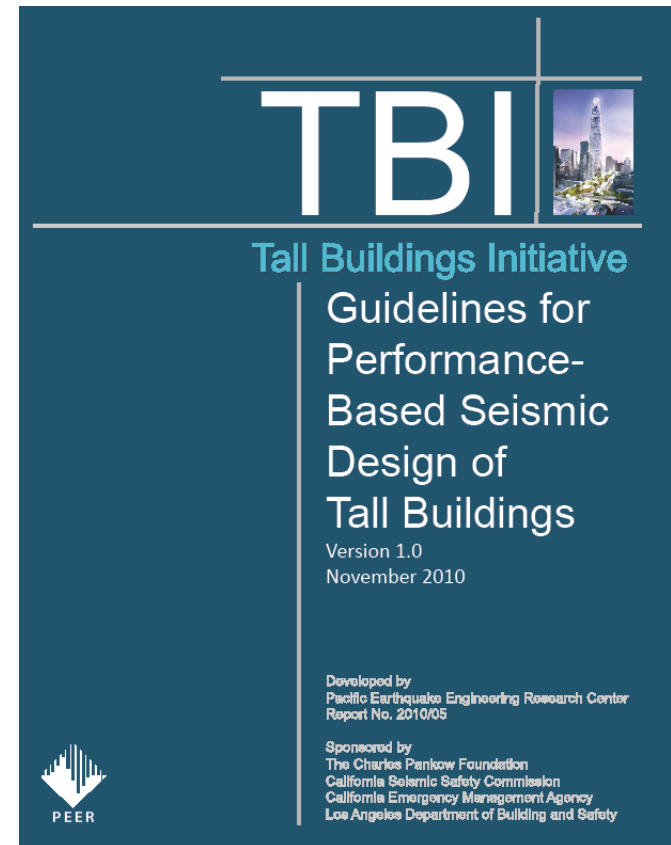
$$F_u = 1.5\tilde{F}$$

$$F_u = \tilde{F} + 1.3\sigma \geq 1.2\tilde{F}$$

- $\phi = 1$ for inconsequential failures
- $\phi =$ per code otherwise

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TBI Case Studies



Sierra Madre
(San Fernando)

San Andreas

Sierra Madre
(Cucamonga)

Verdugo

Raymond



Santa
Monica

Hollywood

Elsinore
(Whittier)

Elsinore
(Chino)

Newport-Inglewood-Rose Canyon

Data SIO, NOAA, U.S. Navy, NGA, GEBCO
Image County of San Bernardino
Image U.S. Geological Survey

Google

2009

- 1.5Km, Puente Hills
- 7.3Km, Hollywood
- 8.8Km, Raymond
- 11.5Km, Santa Monica
- 24.5Km, Elsinore
- 40.0Km, Sierra Madre
- 56Km, San Andreas

Imagery Dates: Mar 16, 2006 - Nov 16, 2009

34°02'17.04" N 118°10'05.62" W elev 288 ft

Eye alt 67.25 mi

Building Design and Modeling

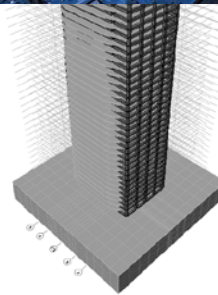
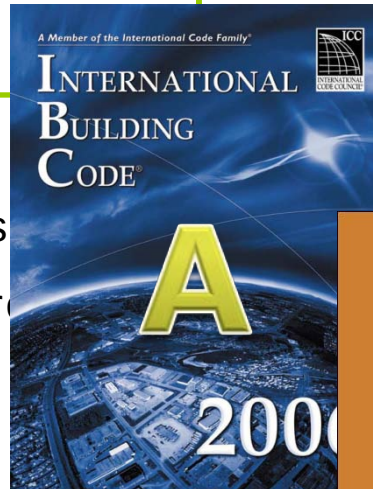
Three Building Systems

42-story reinforced
concrete core wall

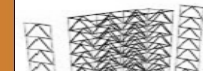
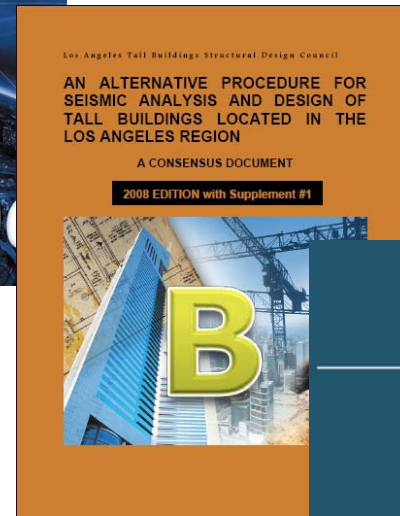


1

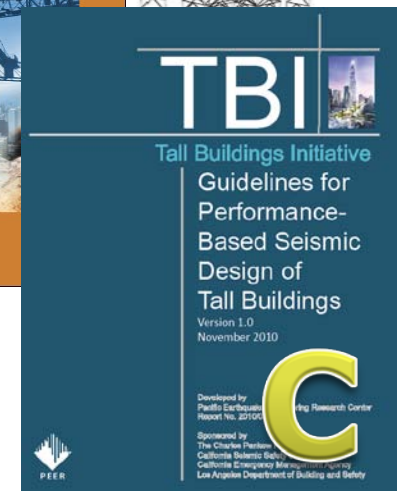
42-s
concr



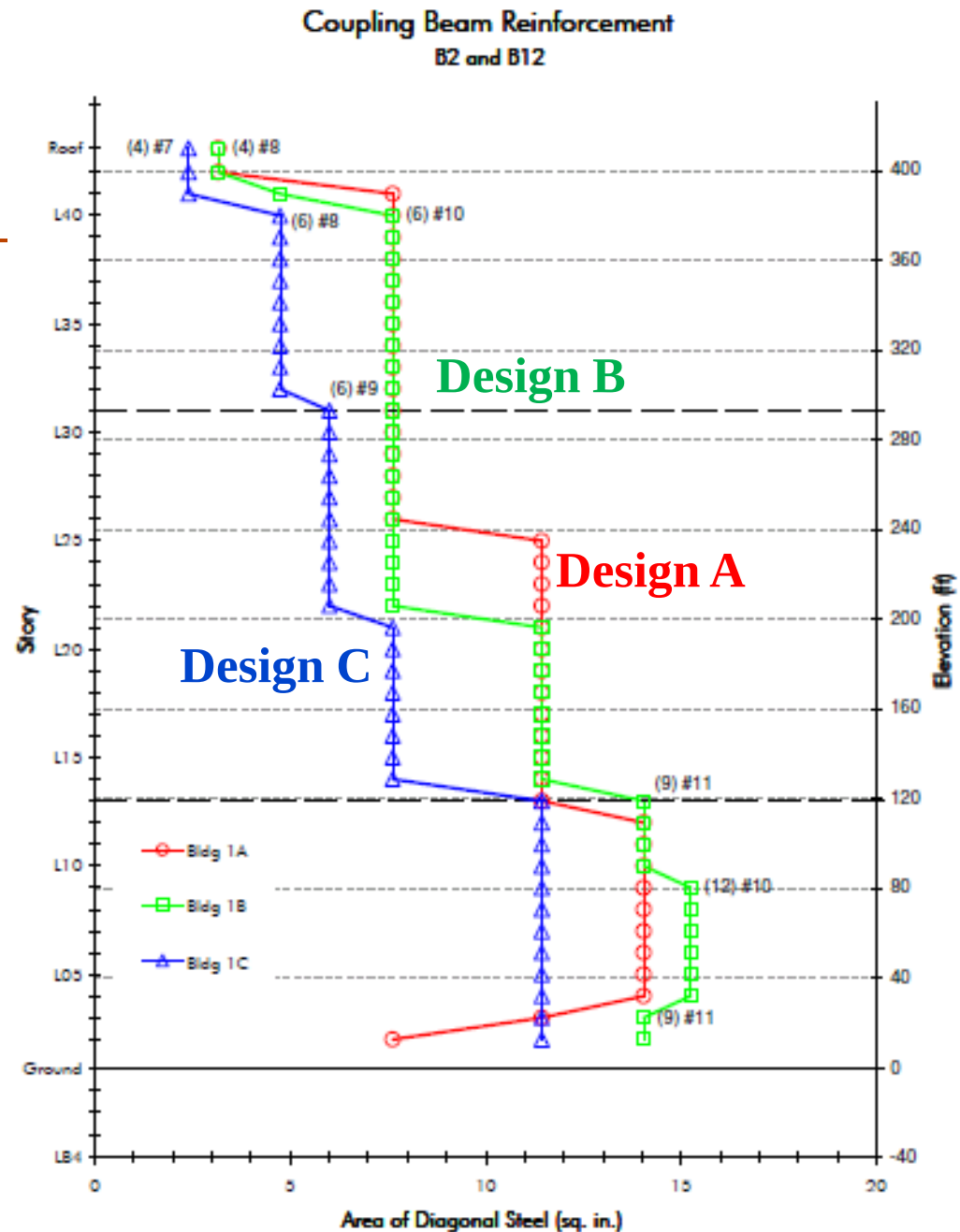
2



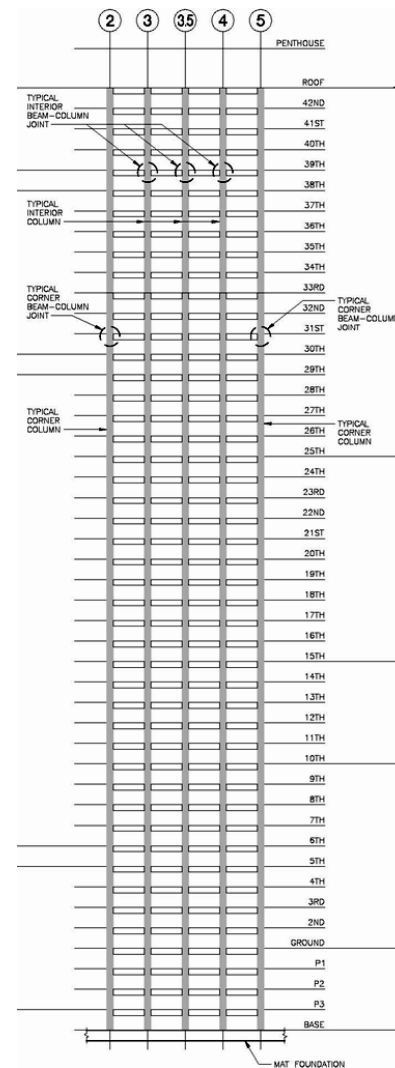
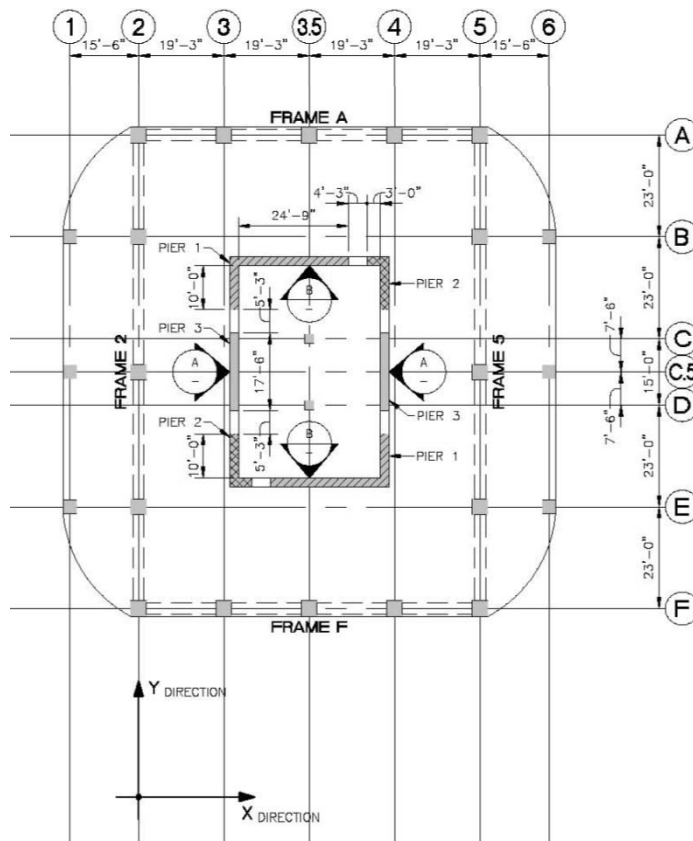
steel buckling-
resistant braced frame



Building 1 – Core only

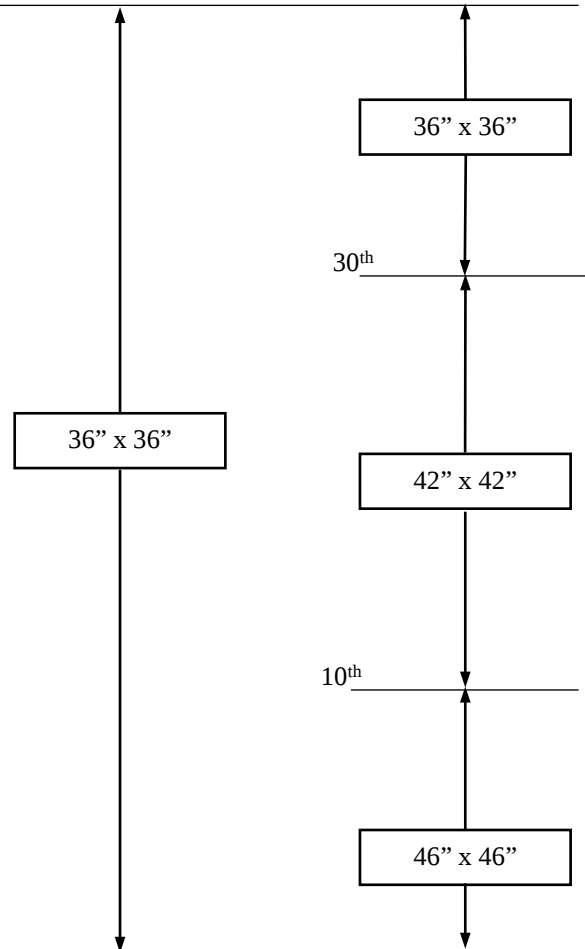


Building 2 – Dual system

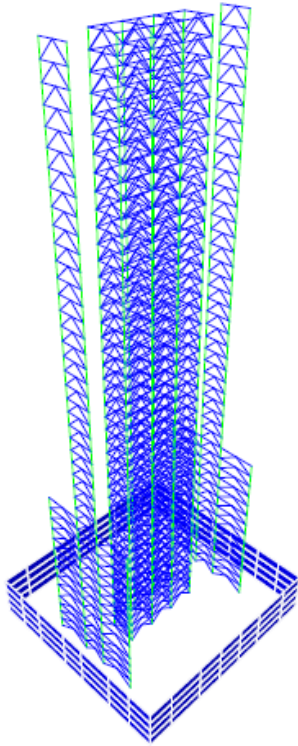


Design A

Design B/C

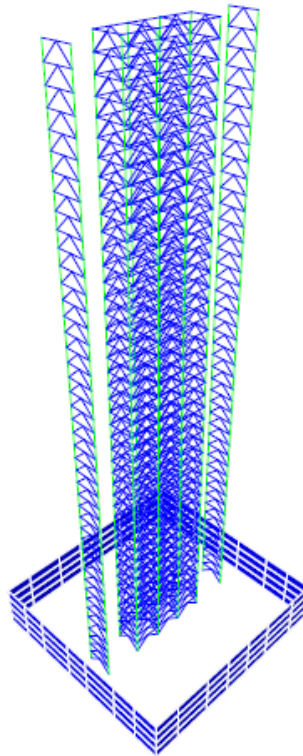


Building 3 - Buckling Restrained Braced Frame



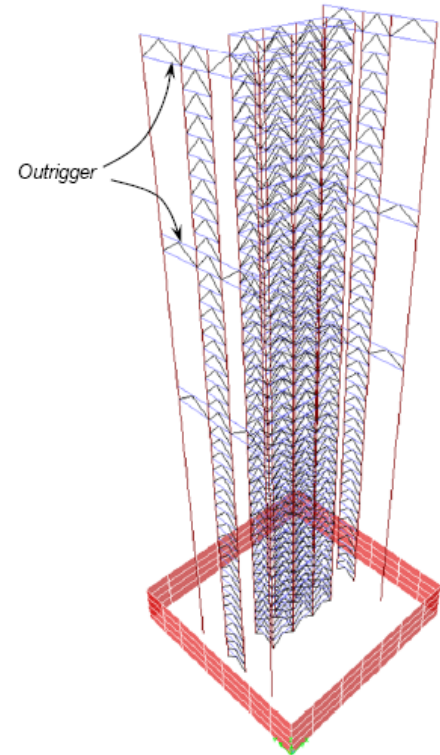
Bldg. 3A

$T_{1NS} = 5.3\text{sec}$



Bldg. 3B

$T_{1NS} = 6.5 \text{ sec}$

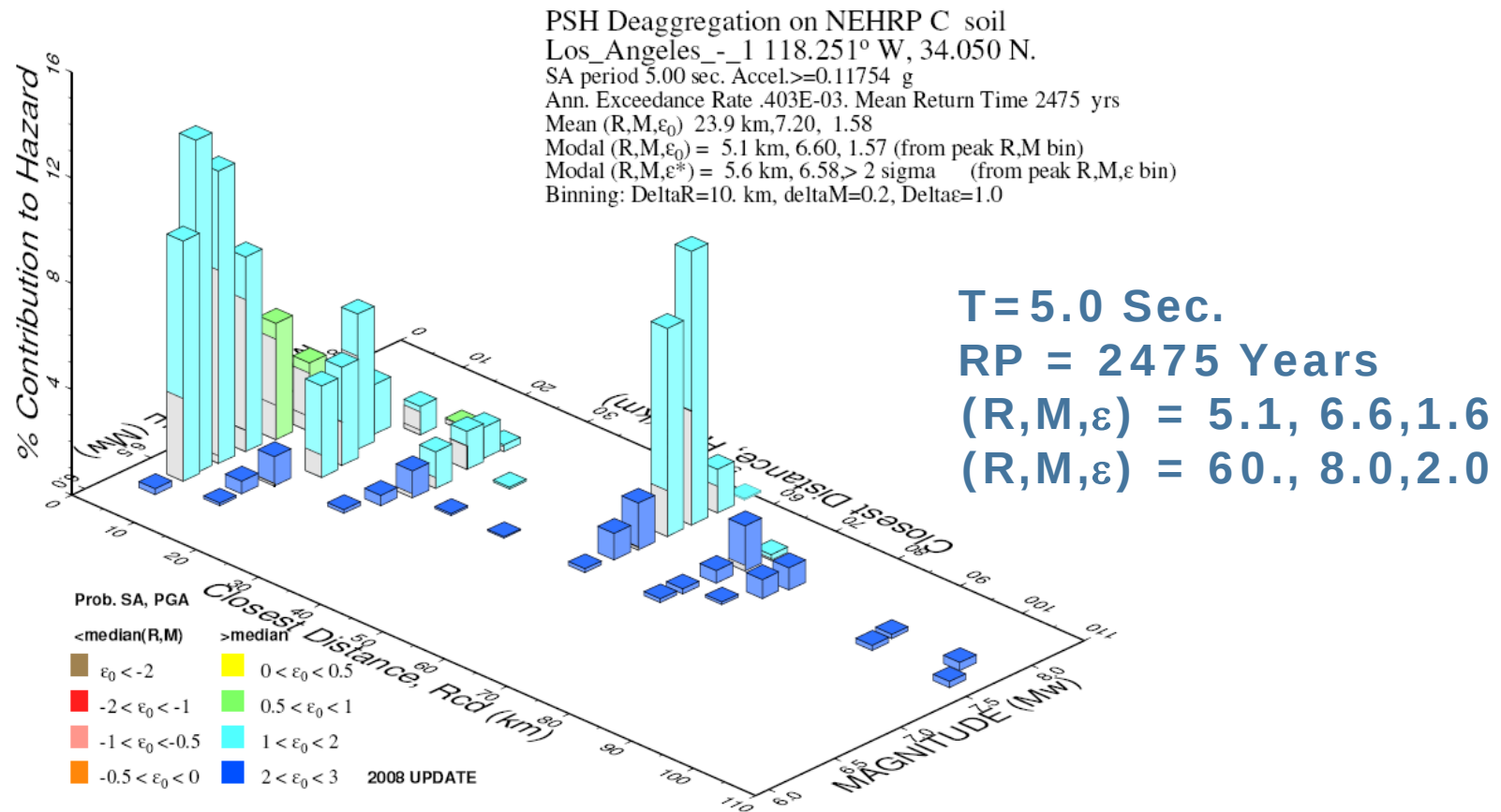


Bldg. 3C

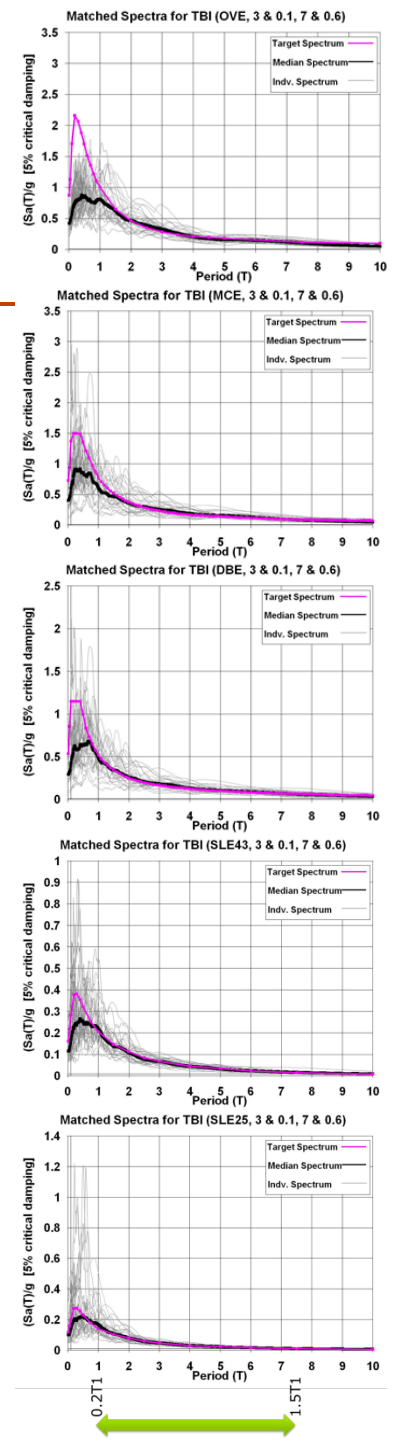
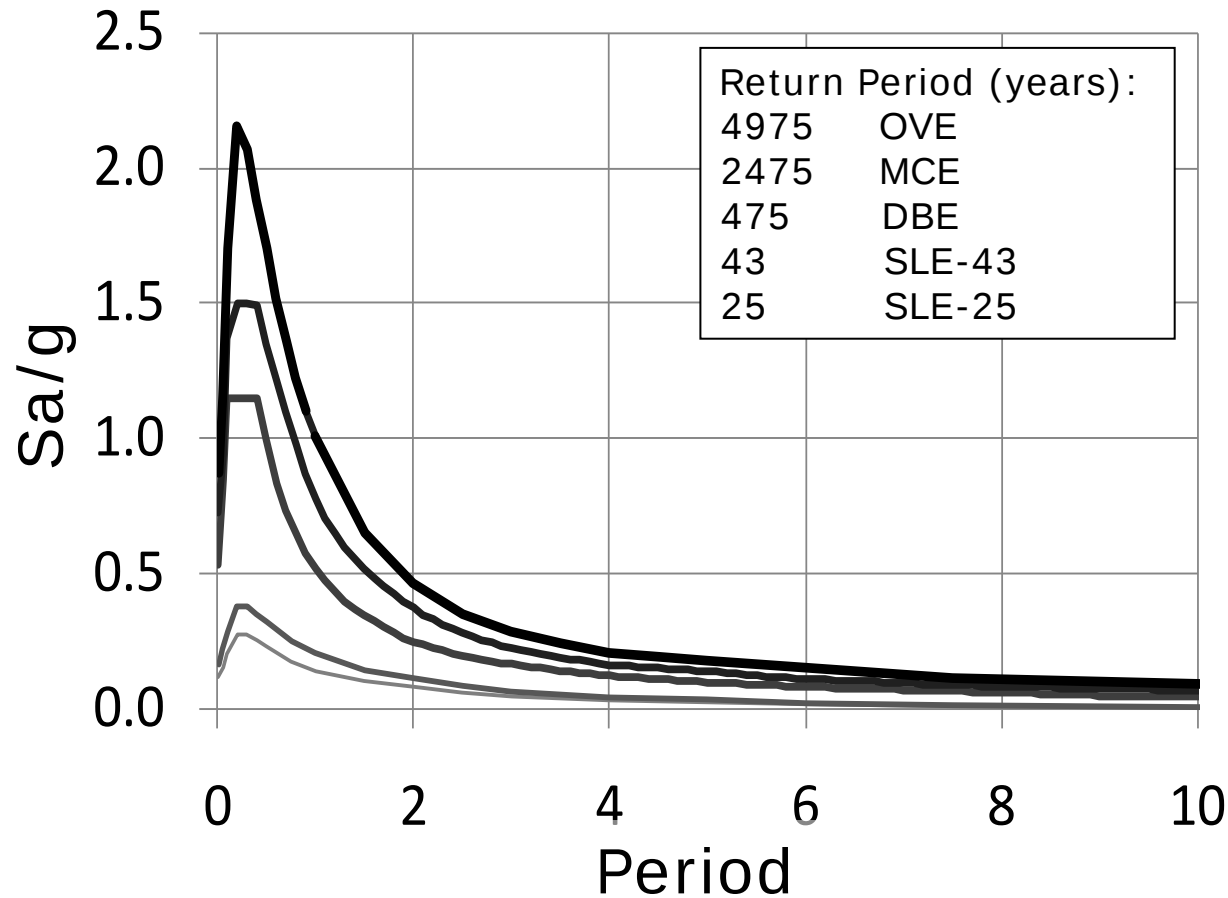
$T_{1NS} = 5.7 \text{ sec}$



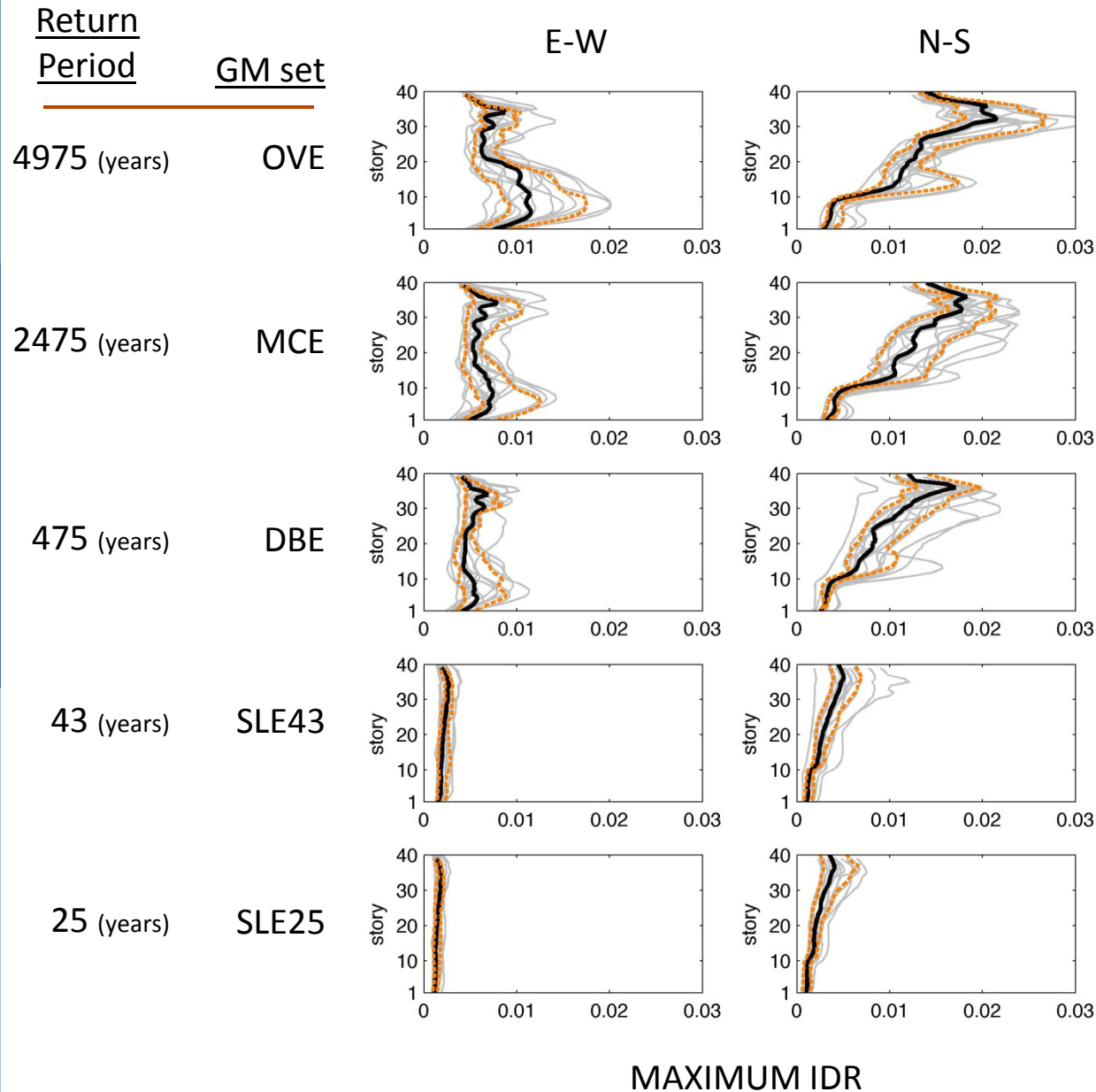
Disaggregation of Seismic Hazard



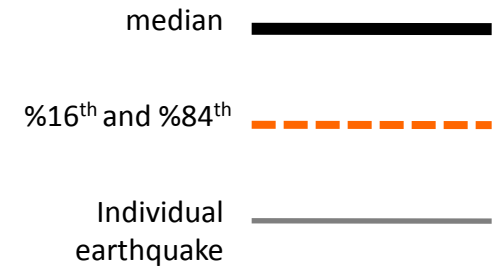
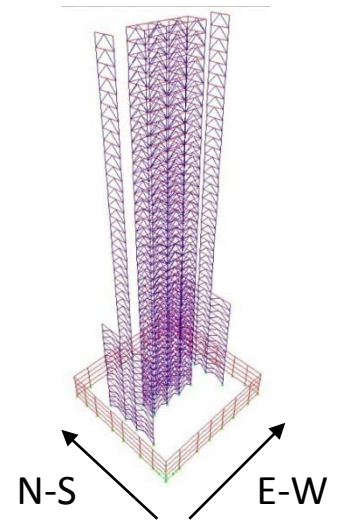
Response Spectra



Example results



Building 3A



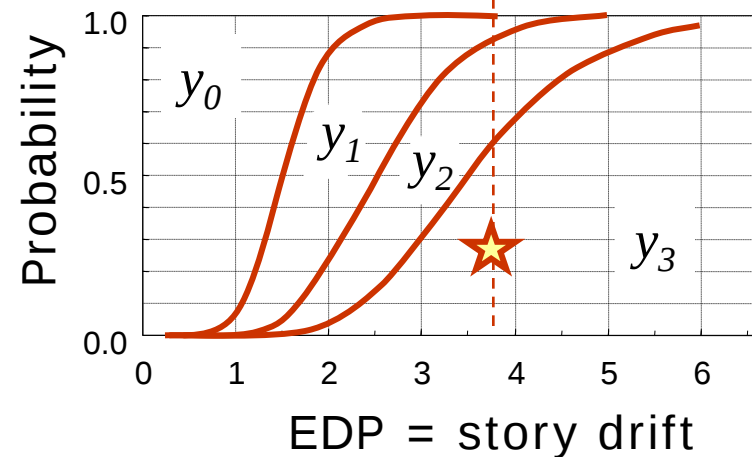
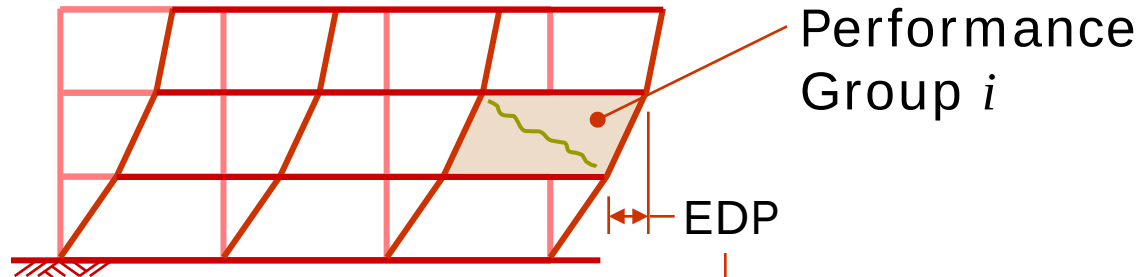
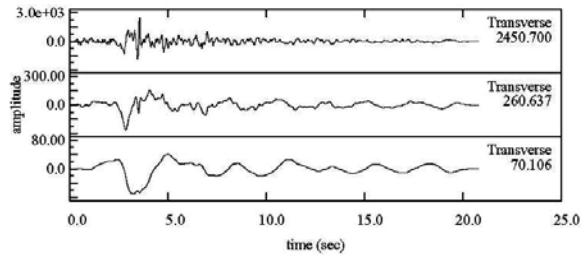
Base Building Costs

	Core Wall 683,000 sq ft	Dual System 683,000 sq ft	BRB 959,000 sq ft
Code Design	\$140 M \$326/ sq ft	\$149 M \$350/ sq ft	\$341 M \$370/ sq ft
PBE-1	\$140 M	\$174 M	\$329 M
PBE-2	\$143 M	\$174 M	\$333 M

Davis Langdon

- Structural system selection has significant impact on construction cost (13%)
- Design basis has relatively little impact on construction cost

Conceptual damage calculation

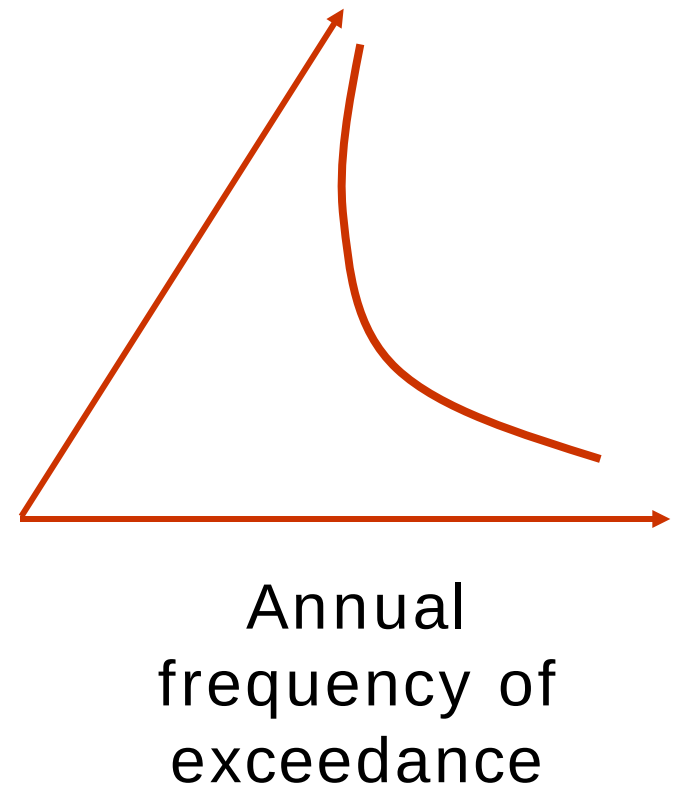
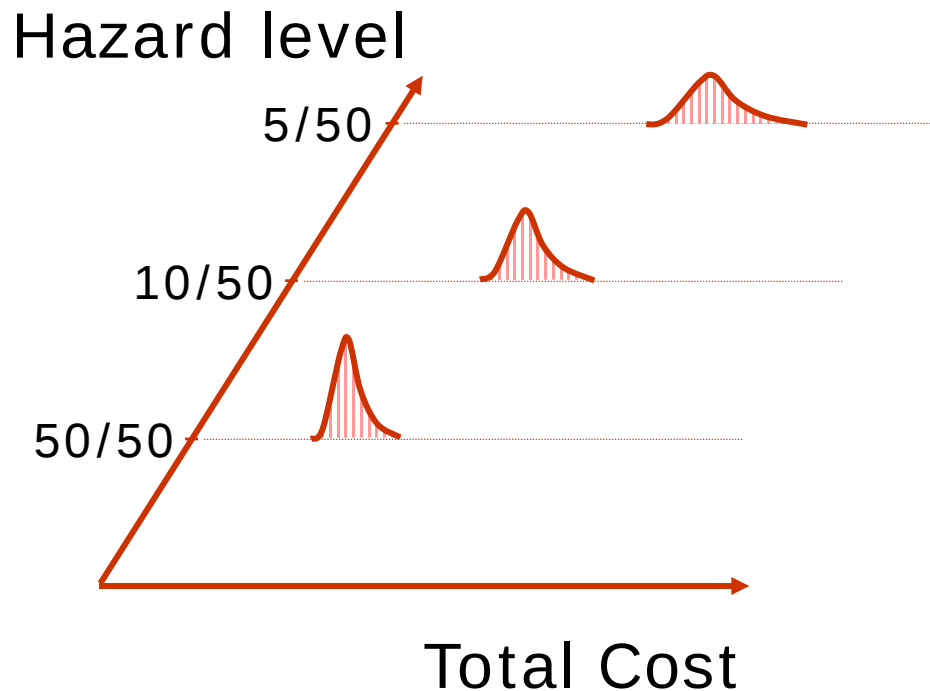


Conceptual repair quantities calculation

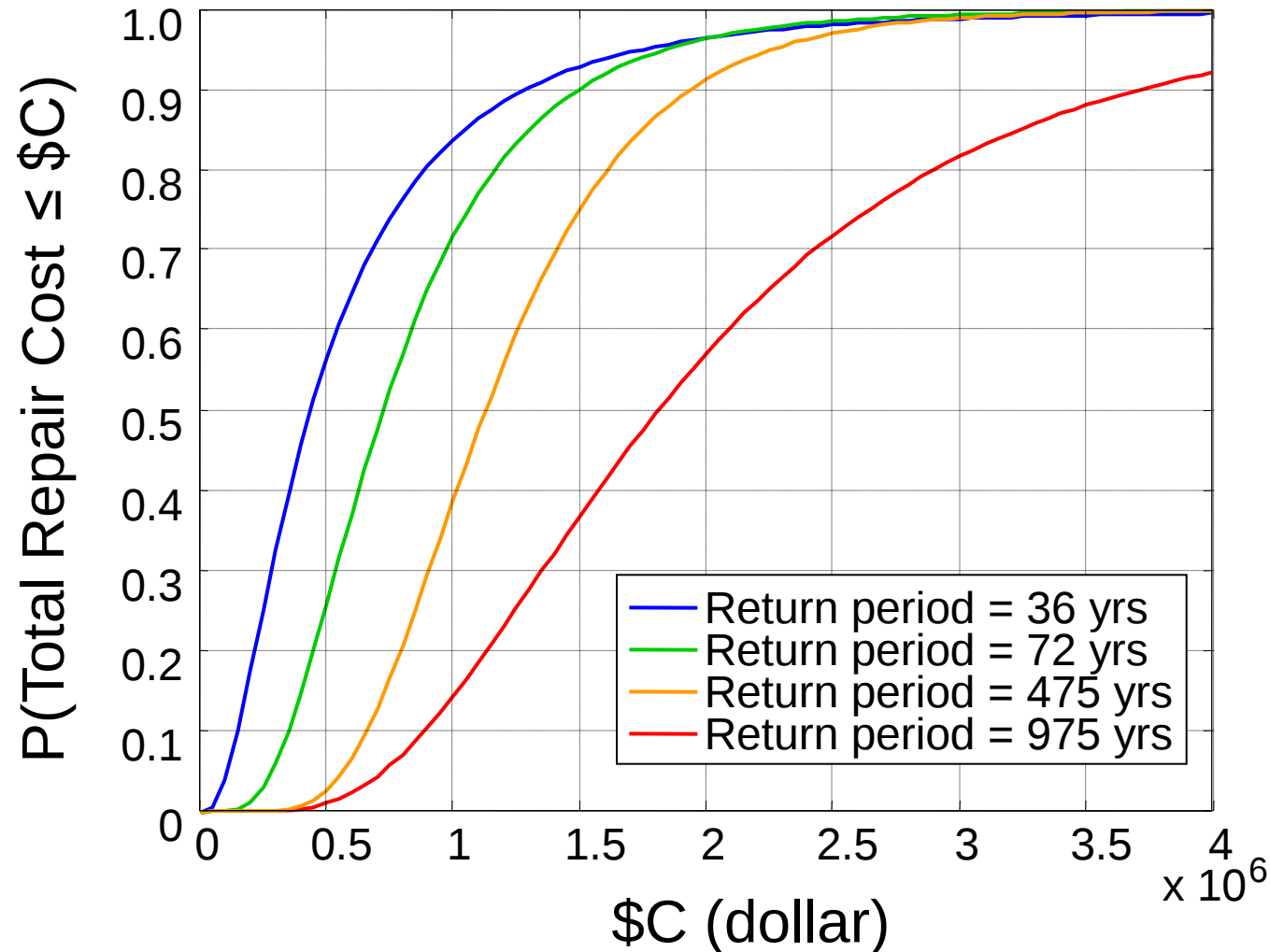
Performance Group i

Quantities	Damage State			
	y_0	y_1	y_2	y_3
concrete	0	0	0	0
steel	0	0	0	0
wallboard	0	0	100	10,000
paint	0	100	1000	10,000
electrical	0	0	0	0
...				

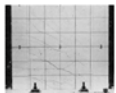
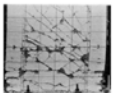
Conceptual repair cost calculation



Summary repair costs

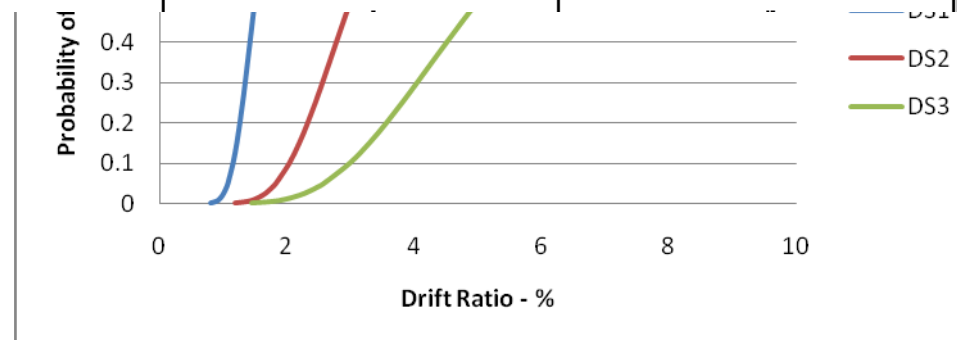
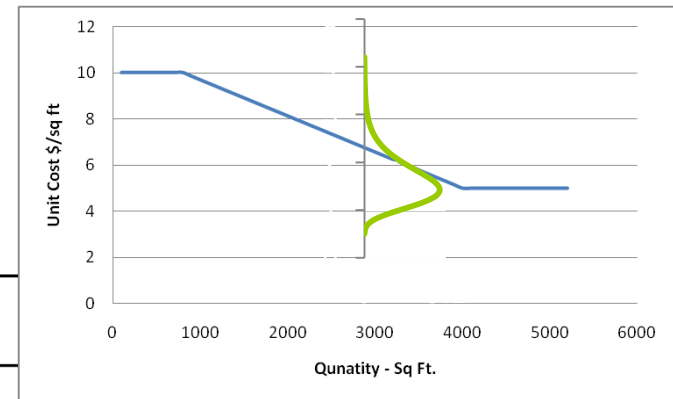


Building Performance Model

Fragility Specification			
B1044.000 Reinforced Concrete Shearwalls			
BASIC COMPOSITION	Reinforced concrete and finishes both sides		
Units for basic quantities	Square feet of wall area		
DAMAGES STATES, FRAGILITIES, AND CONSEQUENCE FUNCTIONS			
	DS1	DS2	DS3
DESCRIPTION	Flexural cracks < 3/16" Shear (diagonal) cracks < 1/16"	Flexural cracks > 1/4" Shear (diagonal) cracks > 1/8"	Max. crack widths > 3/8" Significant spalling/ loose cover
ILLUSTRATION (example photos are shown)			

MEDIAN DEMAND	1.5%
BETA	0.2
	Patch cracks each side with caulk Paint each side Patch and paint

CONSEQUENCE FUNCTION			
Max. consequence up to lower quantity	\$4.00 per sq ft up to 800 sq ft	\$10.00 per sq ft up to 800 sq ft	\$50.00 per sq ft up to 200 sq ft
Min. consequence over upper quantity	\$2.00 per sq ft over 4000 sq ft	\$5.00 per sq ft over to 4000 sq ft	\$30.00 per sq ft over 2000 sq ft
Beta (consequence)	0.2	0.3	0.3



Building Performance Models

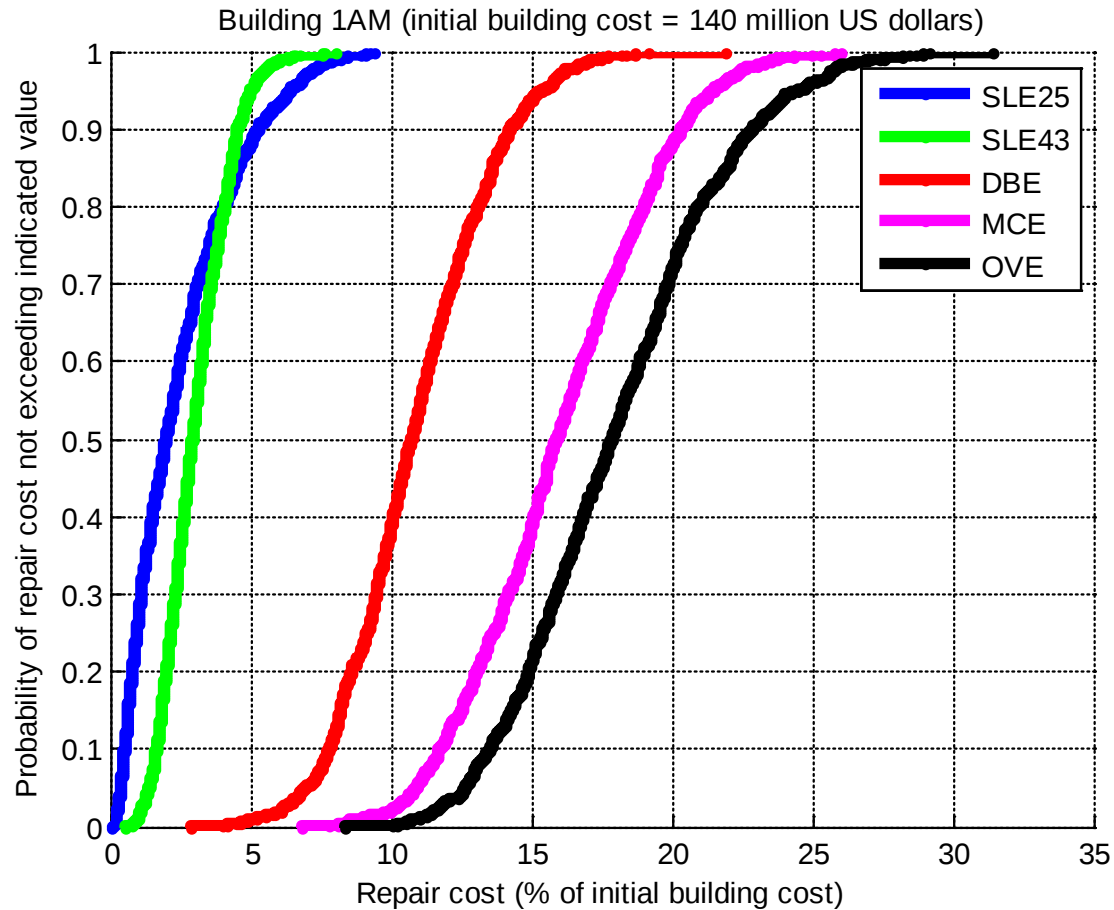
■ Structural

- Shear walls
 - Shear cracking
 - Flexural damage
- Link beams
- Gravity columns
- Moment joints
- Buckling restrained braces

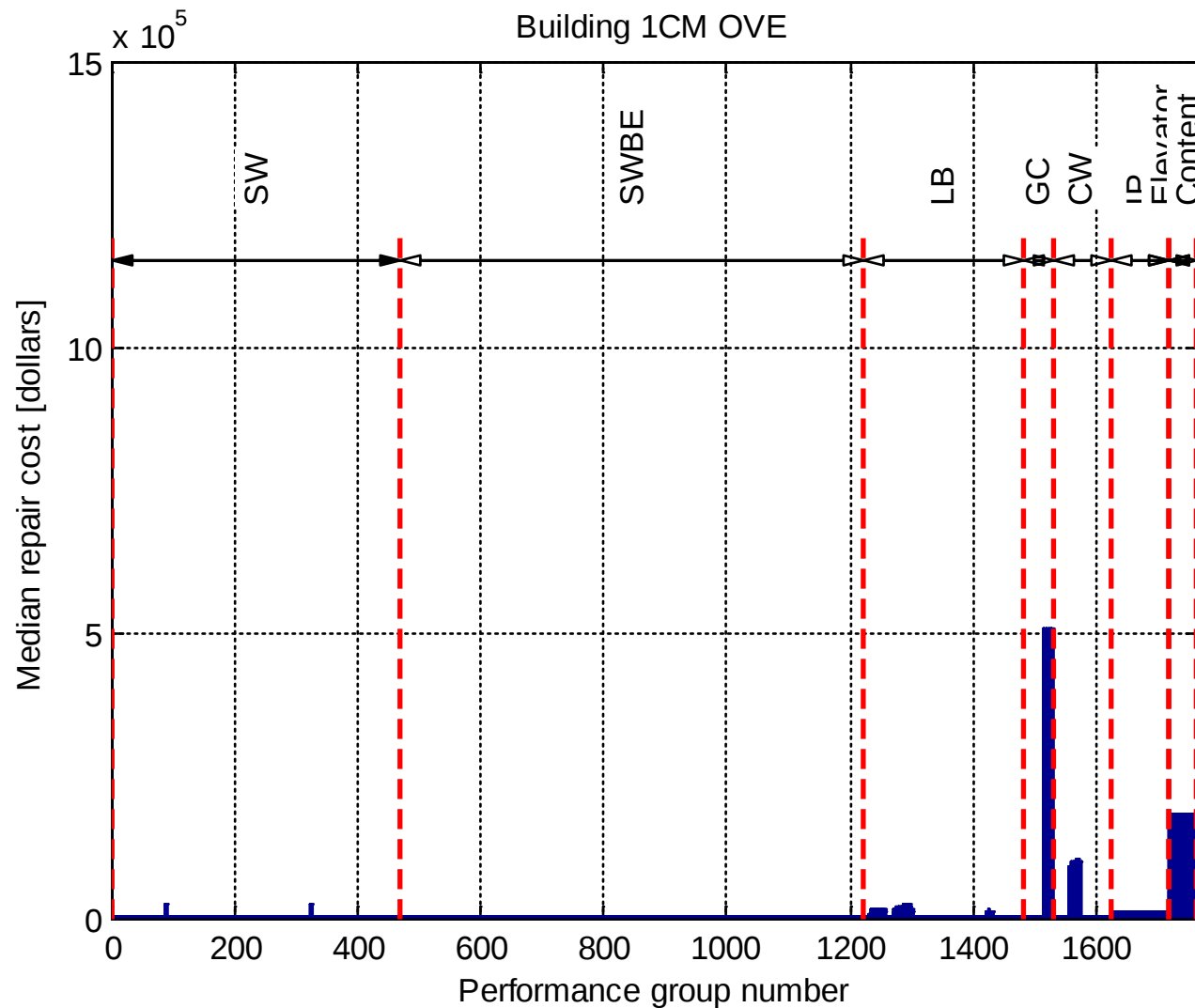
■ Nonstructural

- Curtain walls
- Interior partitions
- Ceilings
- Elevators
- Contents

Loss Results

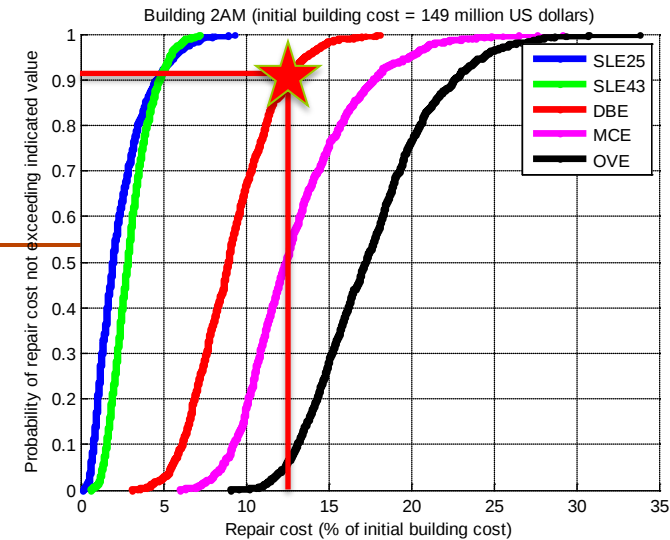


Loss Results



Summary Results

■ Probable Maximum Loss – 500 year



	Core Wall	Dual System	BRB
Code Design	14%	13%	3%
PBE-1	15%	9%	3%
PBE-2	12%	9%	3%

Note – BRBs does not include effect of residual drift

Summary Results

$$\sum_i (\overline{LOSS_{GM_i}})(P(GM_i))$$

- Average Annual Loss
- Annual Insurance Premium

	Core Wall	Dual System	BRBF
Code	\$326,000	\$323,000	\$206,000
PBE-1	\$336,000	\$269,000	\$157,000
PBE-2	\$282,000	\$269,000	\$141,000

Summary Results

- Cost-Benefit Analysis
 - Initial Construction Cost
 - Net Present Value of Insurance Premium (50 years)
 - Time value of money – 5%
 - Normalized to code-based building cost

	Core Wall	Dual System	BRB
Code	1.0	1.0	1.0
PBE-1	1.0	1.1	1.0
PBE-2	1.0	1.1	1.0

PEER Tall Buildings Initiative



PEER

