

The Role of Performance-Based Engineering in Tall Building Design

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PEER Annual Meeting, 1/20/06



Personal Observations

Tall buildings are special

- Socio-economic perspective -- DVs
 - Potential huge impact of the three D's (Dollars, Downtime, Deaths)
 - A disaster can change the landscape of cities
- Engineering perspective -- EDPs
 - Higher mode effects may control structural response
 - P-delta effects may control collapse potential
 - Deterioration together with P-delta will control collapse potential
 - Innovative protective measures deserve much consideration
 - There are phenomena that are not detected in a code analysis
 - plastic hinges in columns
 - Story mechanisms and multiple story mechanisms
 - Importance of gravity system
 - shear amplification in shear walls
- Ground motion/hazard perspective -- IMs
 - Unfortunately we don't know enough about long period frequency content



Design/Assessment Options

Equiv. Static Force Procedure

- Designing for an elastic code base shear and elastic drift limit will result in structures with vastly different damage potential and collapse probability

Linear Dynamic Procedure

- Still the same problems, except accounts for higher mode effects

Nonlinear Static Procedure (NSP)

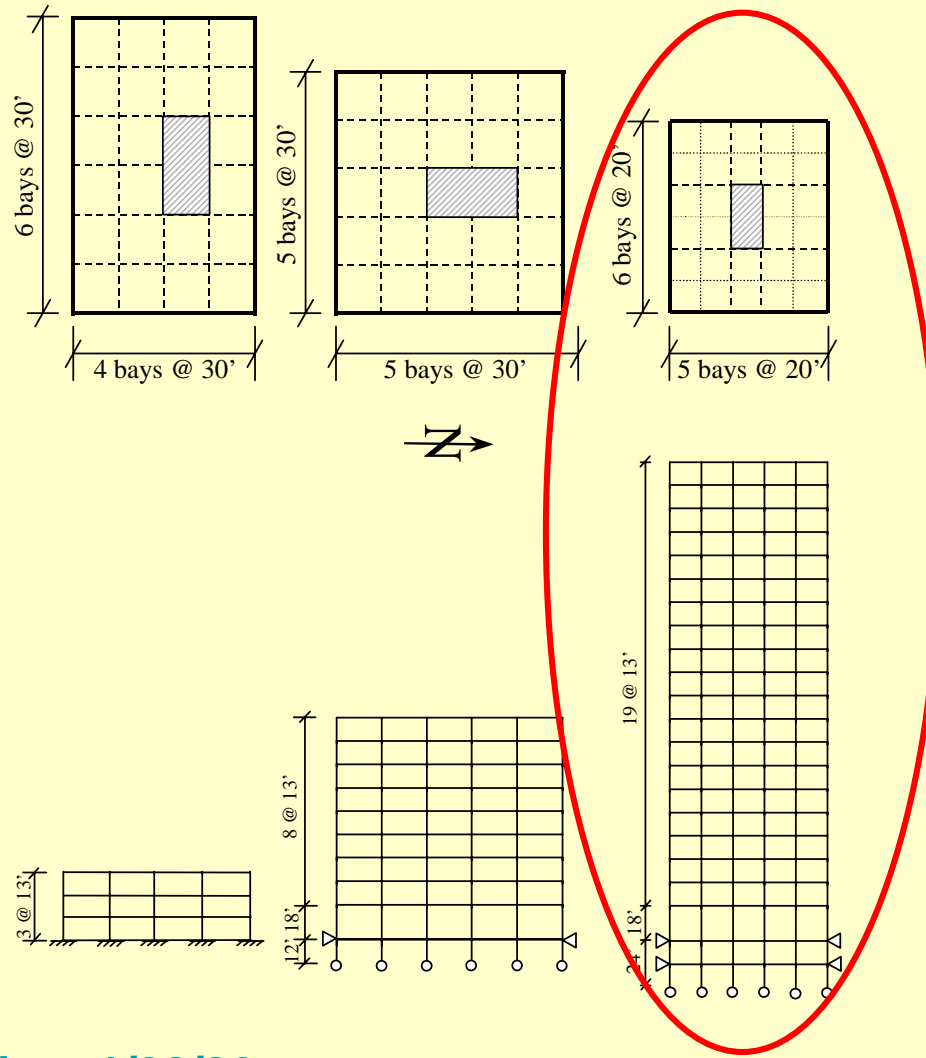
- Problems with higher mode effects
- Does not detect dynamic redistribution problems such as shear force amplification in wall structures
- Does not capture collapse potential

Nonlinear Dynamic Analysis (NDP)

- Addresses most of the issues, BUT needs performance criteria



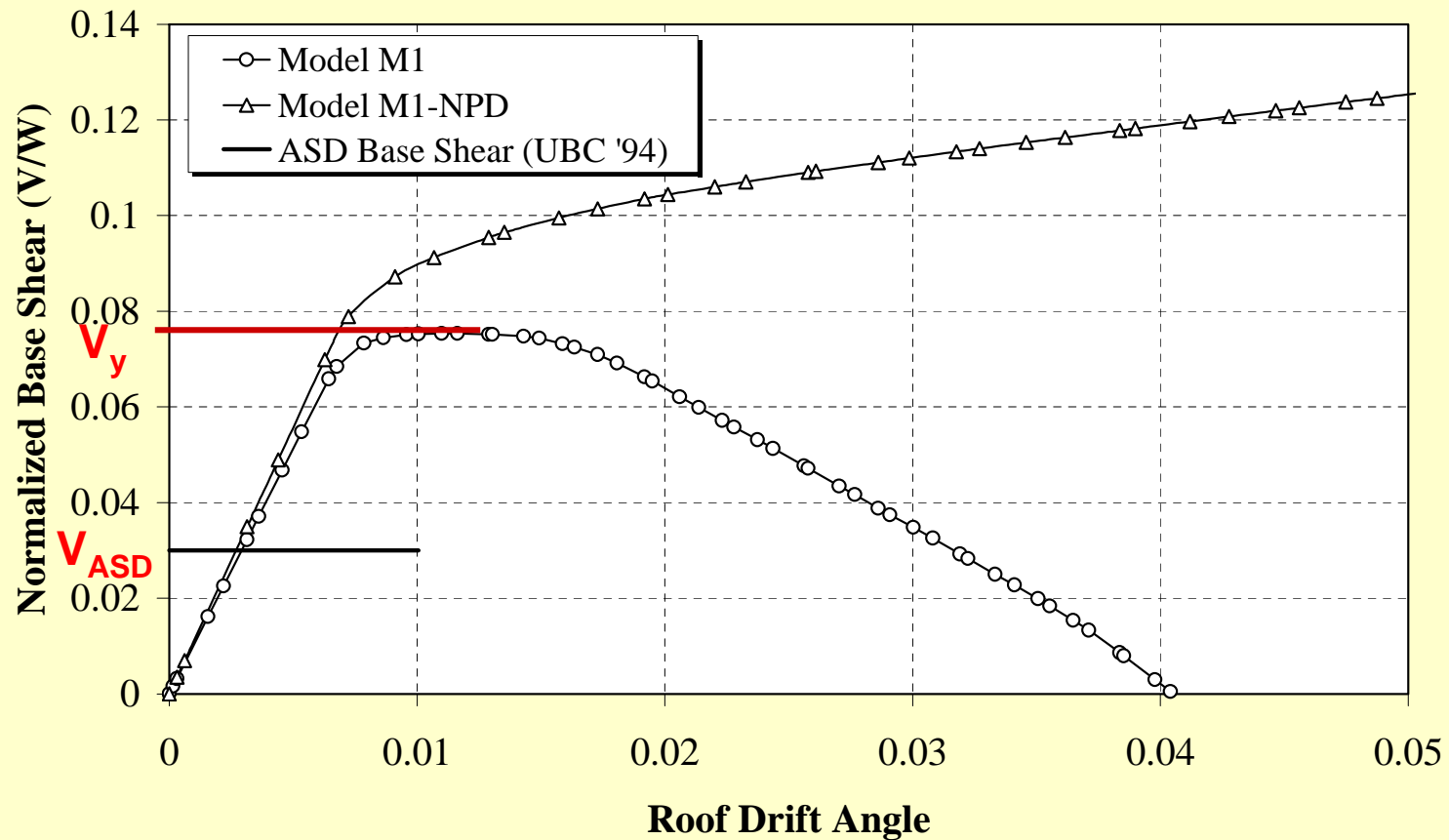
SAC Structures



Global Pushover Curve, LA-20, without and with P- Δ

ROOF DRIFT ANGLE vs. NORMALIZED BASE SHEAR

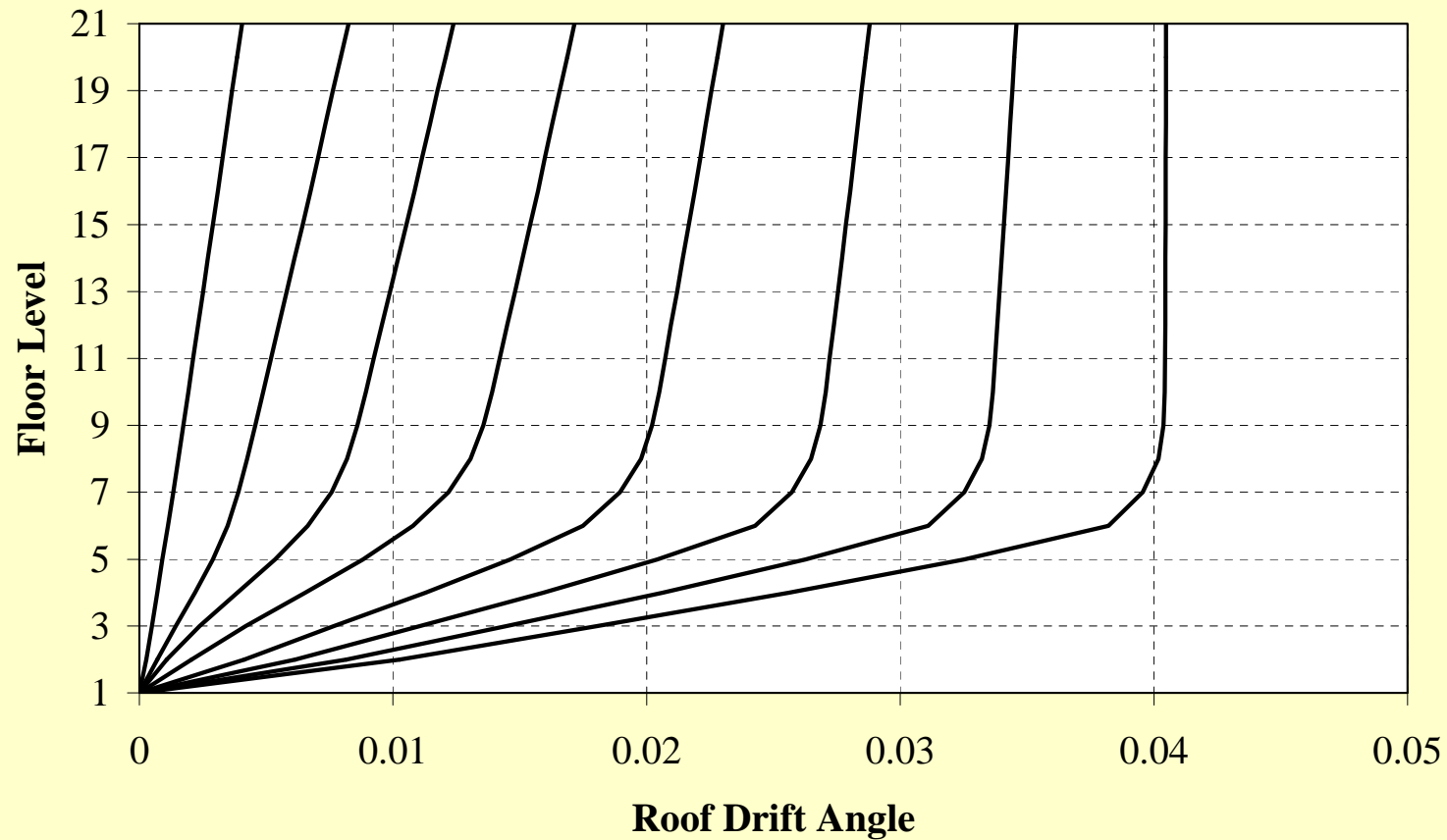
Pushover (NEHRP '94 k=2 pattern): LA 20-Story, Pre-Northridge, M1, M1-NPD



Pushover Deflection Profiles, LA 20-story Structure

DEFLECTED SHAPE DURING STATIC PUSHOVER ANALYSIS

Pushover (NEHRP '94 k=2 pattern): LA 20-Story, Pre-Northridge, M1

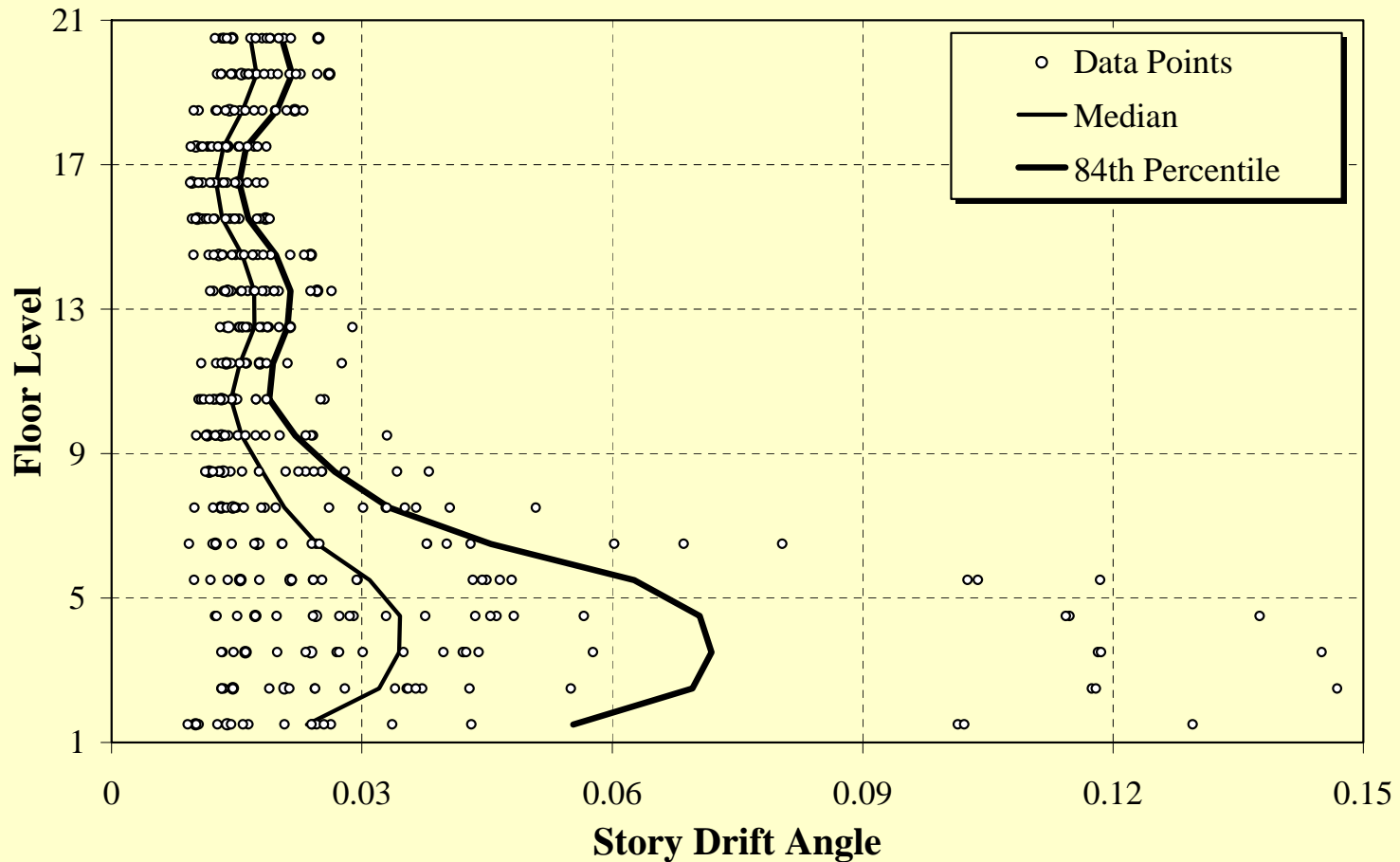


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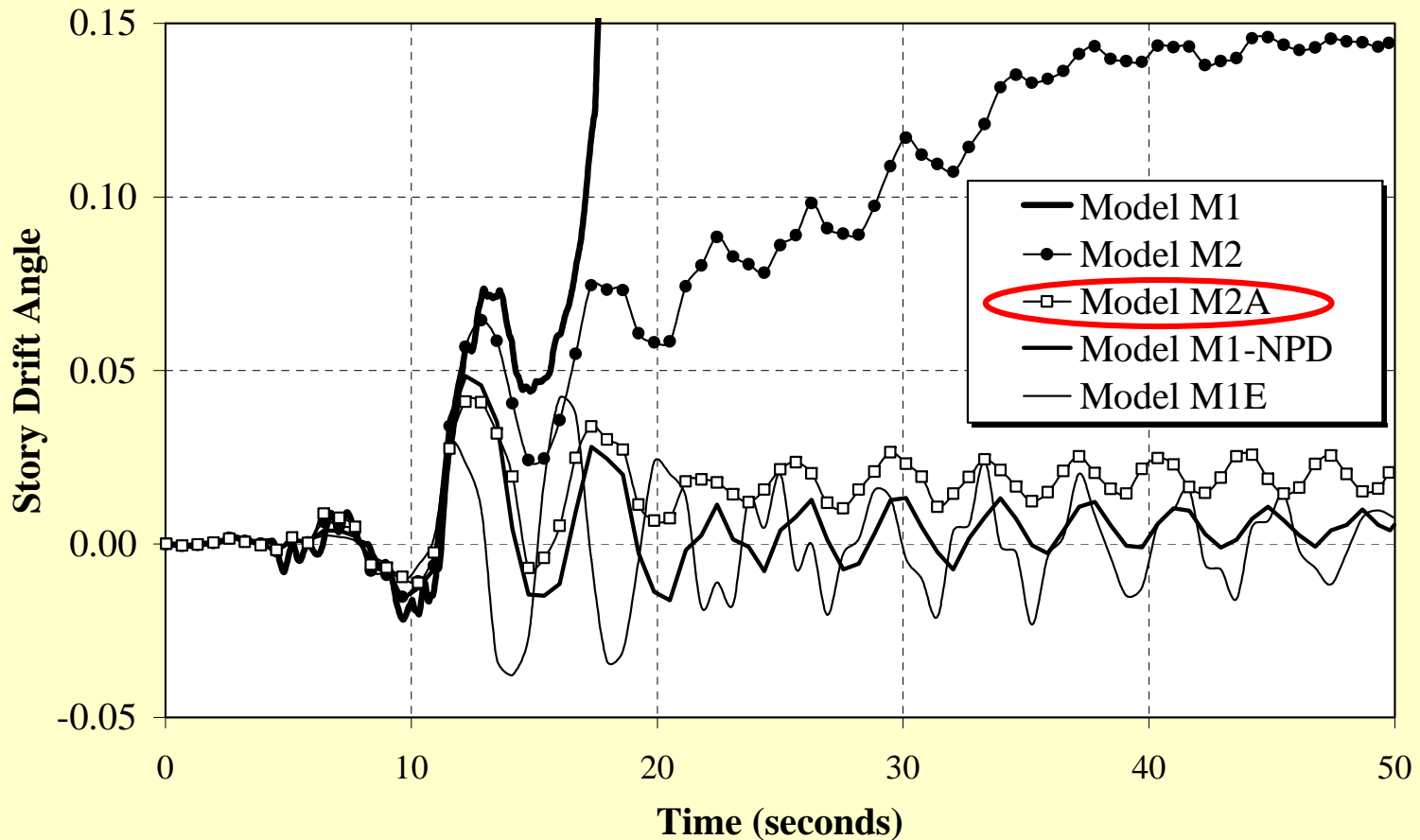
Dispersion in Story Drifts, LA-20, 2/50 Records

STORY DRIFT DEMANDS FOR LA 20-STORY
2/50 Set of Records: Pre-Northridge, Model M2



Story 2 Drift Response, LA-20, Various Models

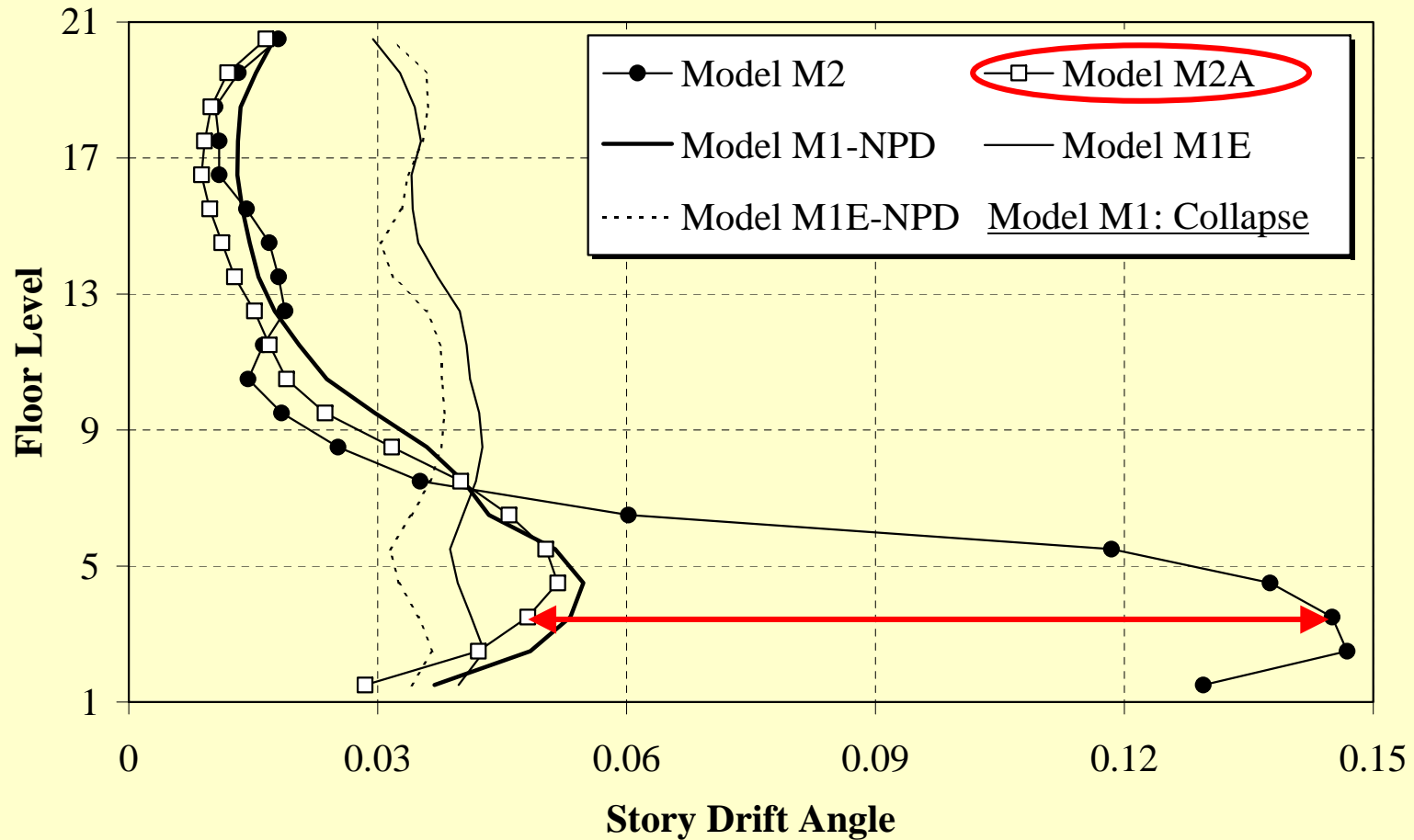
STORY 2 DRIFT ANGLE TIME HISTORIES
Record LA30 (Tabas): LA 20-story, Different Analytical Models



Story Drift Demands – Various Models

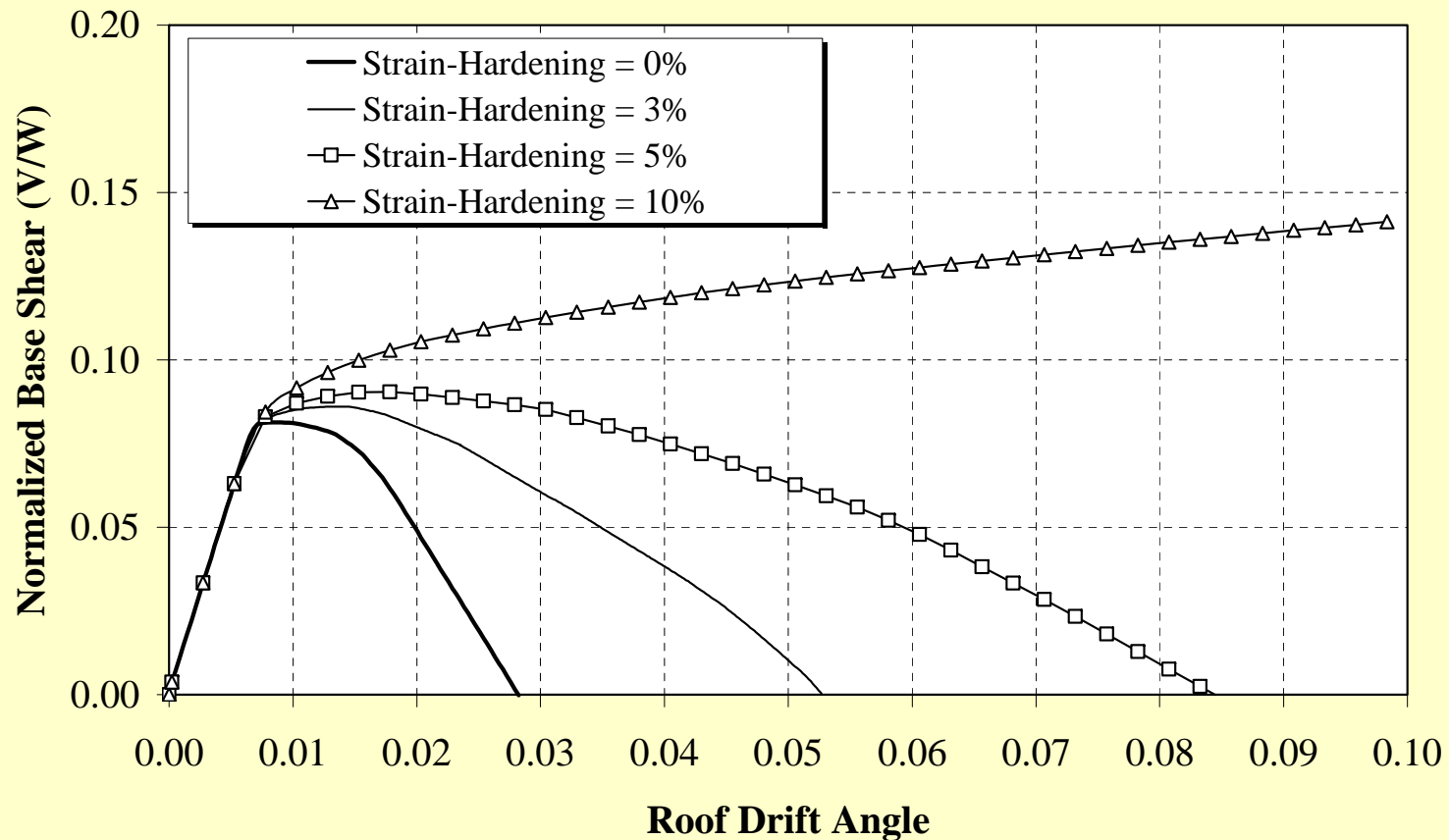
STORY DRIFT ANGLE ENVELOPES

Record LA30 (Tabas): LA 20-story, Different Analytical Models



Sensitivity to Strain Hardening, Pushover, LA-20

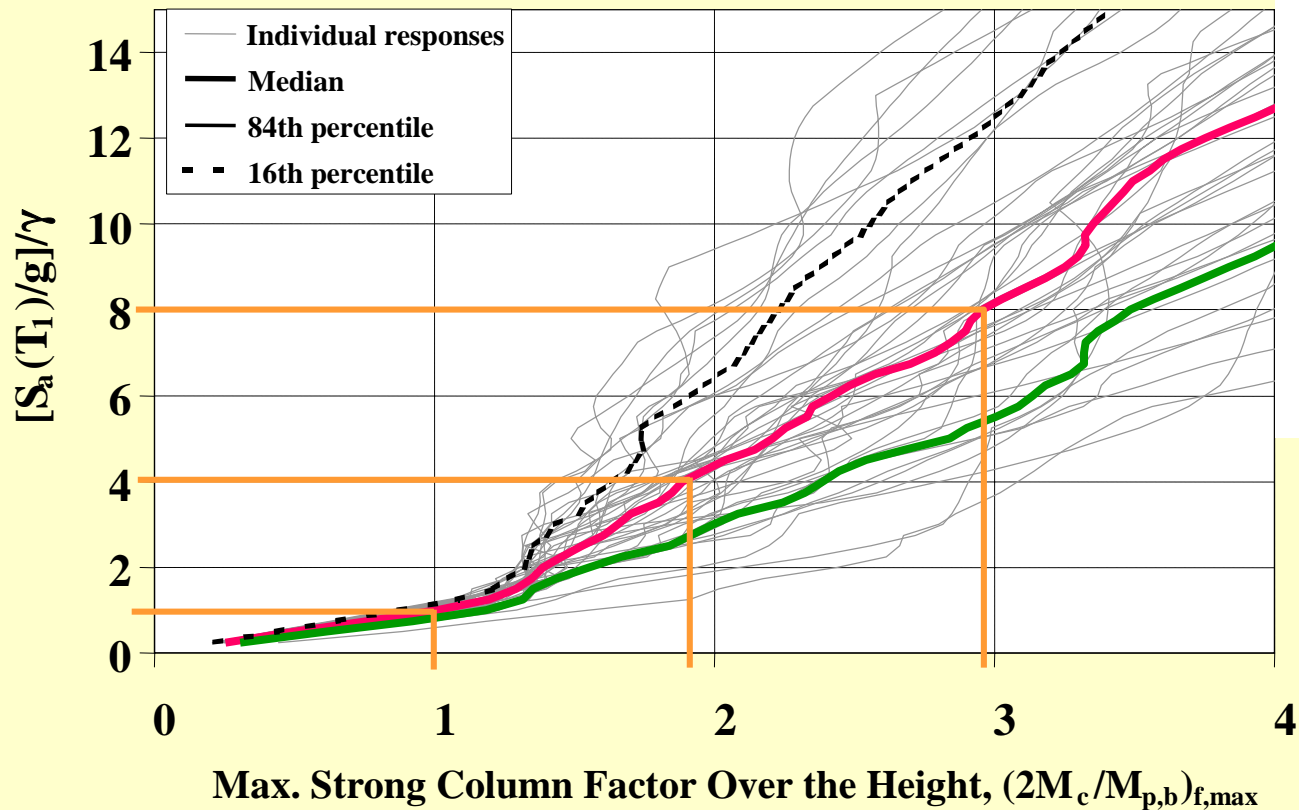
ROOF DRIFT ANGLE vs. NORMALIZED BASE SHEAR
Pushover: LA 20-Story, Pre-Northridge, Model M2, $\alpha = 0\%, 3\%, 5\%, 10\%$



Dependence of Strong Column Factor on R_{μ} 9-Story, $T_1 = 0.9$ sec.

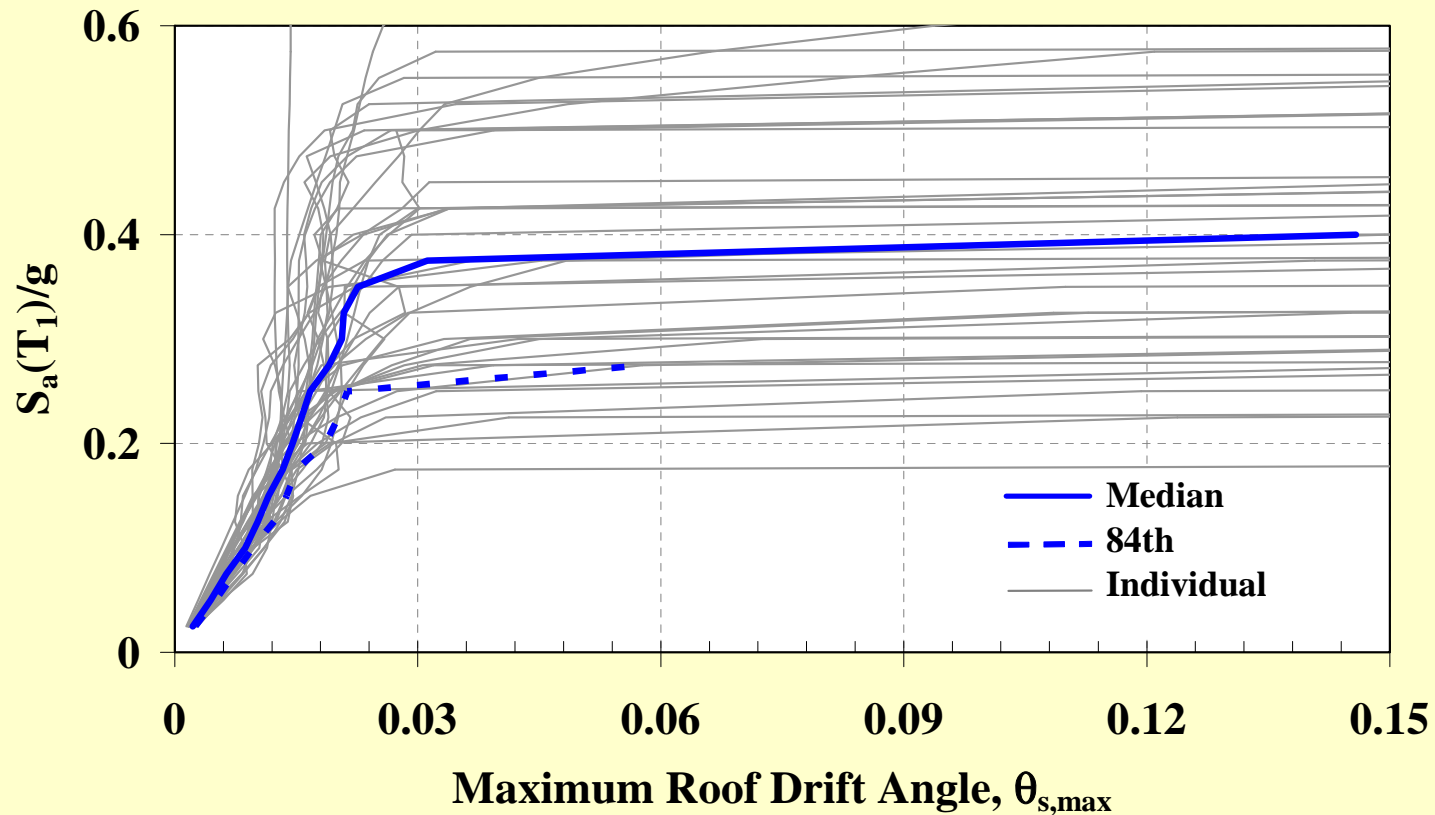
MAXIMUM STRONG COLUMN FACTOR

$N=9$, $T_1=0.9$, $\xi=0.05$, Peak-oriented model, $\theta=0.015$, BH, K_1 , S_1 , LMSR-N



IDAs to Collapse P-Delta Included, no Deterioration

$S_a(T_1)/g$ vs MAXIMUM ROOF DRIFT ANGLE, $\gamma=0.1$
N=18, $T_1=3.6$, BH, Peak Oriented Model, LMSR-N, $\xi=5\%$,
 $\alpha_s=0.03$, $\delta_c/\delta_y=\text{inf.}$, $\alpha_c=\text{N.A.}$, $\gamma_{s,c,k,a}=\text{Inf}$, $\lambda=0$

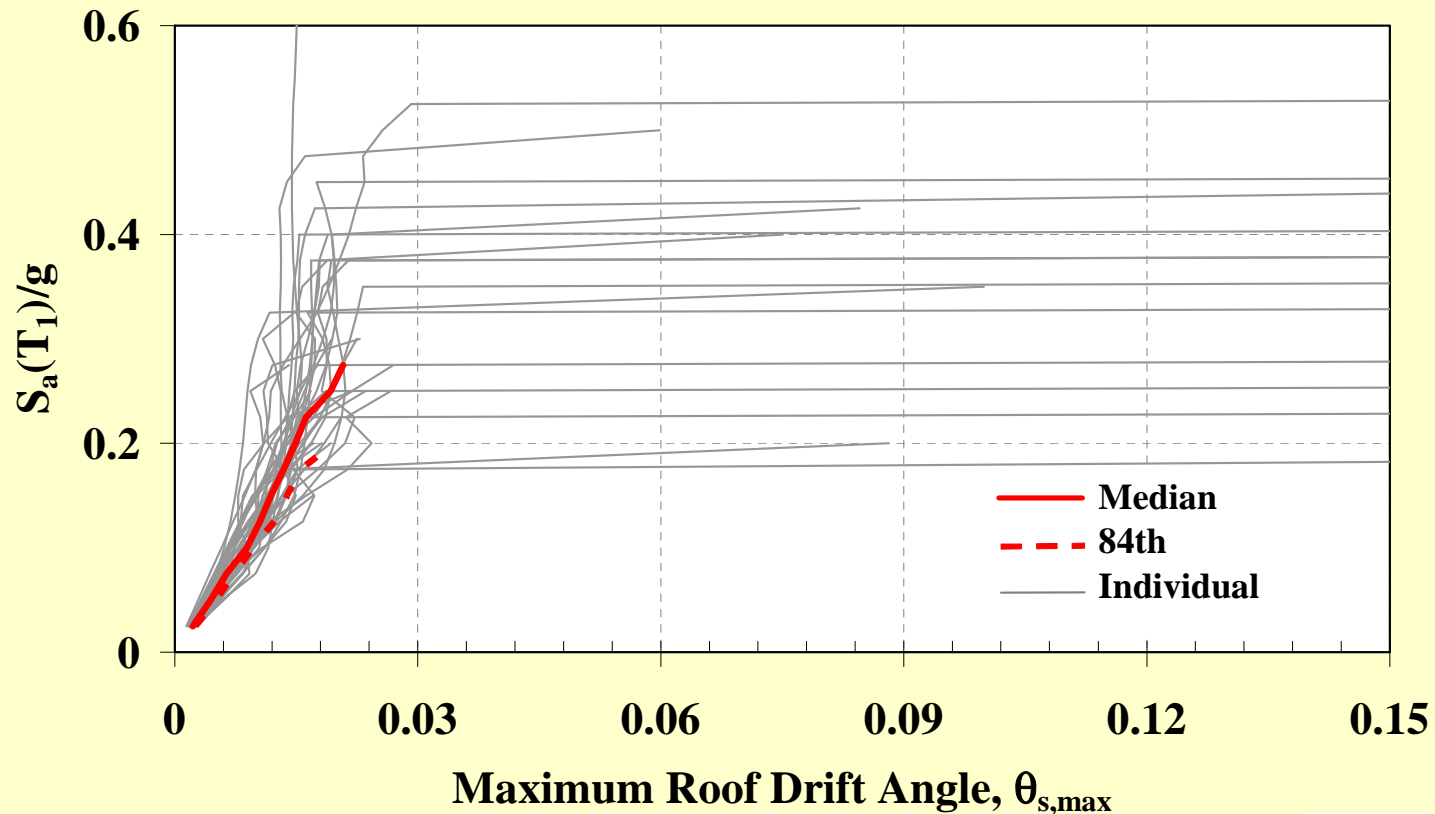


IDAs to Collapse P-Delta Included, with Deterioration

$S_a(T_1)/g$ vs MAXIMUM ROOF DRIFT ANGLE, $\gamma=0.1$

$N=18$, $T_1=3.6$, BH, Peak Oriented Model, LMSR-N, $\xi=5\%$,

$\alpha_s=0.03$, $\delta_c/\delta_y=4$, $\alpha_c=-0.10$, $\gamma_{s,c,k,a}=\text{Inf}$, $\lambda=0$

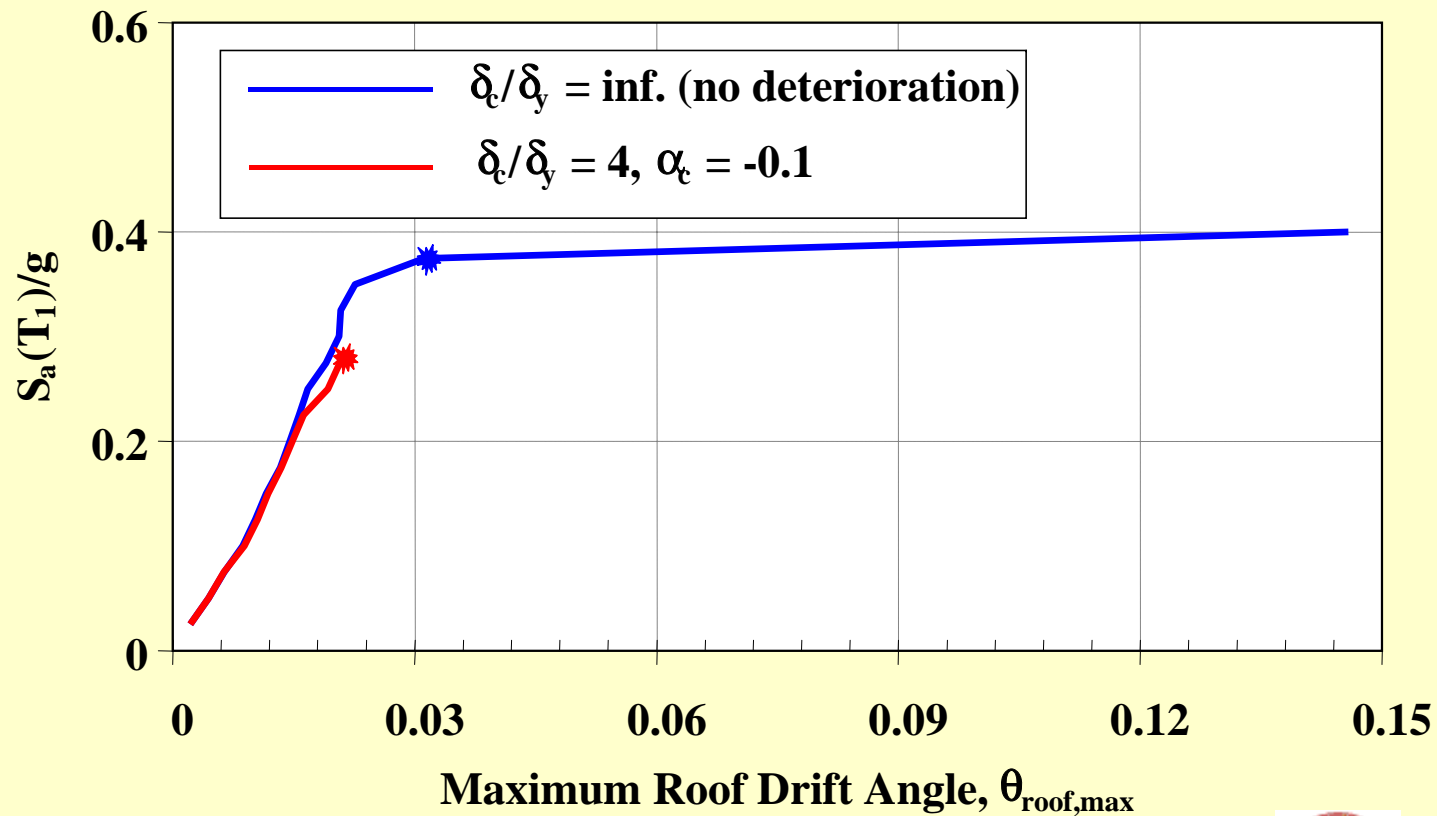


Median IDAs to Collapse P-Delta without and with Deterioration

$S_a(T_1)/g$ vs Median Max ROOF DRIFT ANGLE, $\gamma=0.1$

$N=18$, $T_1=3.6$, BH, Peak Oriented Model, LMSR-N, $\xi=5\%$,

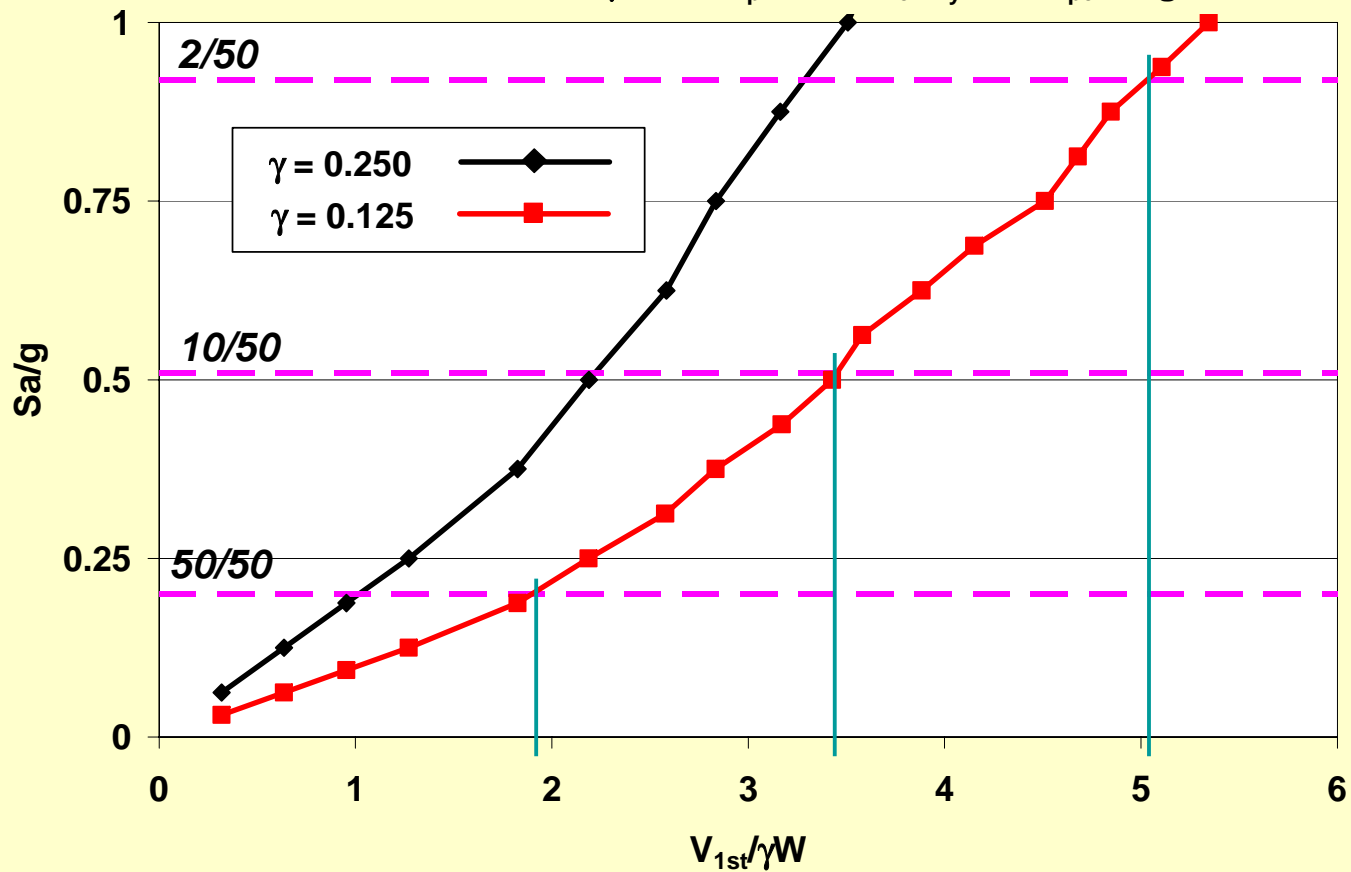
$\alpha_s=0.03$, $\delta_c/\delta_y=\text{var.}$, $\alpha_c=\text{var.}$, $\gamma_{s,c,k,a}=\text{Inf}$, $\lambda=0$



Amplification of Shear Demand in Tall Wall Structures

Median of Shear Magnification @ 1st Story

Shear Wall, N=16, T=1.6sec, γ =var. , $\theta_p=0.02$, $M_c/M_y=1.1$, θ_{pc} =large



Does NDP Solve all the Problems

- **Not without performance criteria for**
 - Acceptable direct (\$) loss
 - Acceptable downtime loss
 - Tolerable probability of collapse
- **Not without consideration of uncertainties**
 - Aleatory uncertainties due to RTR variability
 - Epistemic uncertainties inherent in
 - Structural modeling assumptions
 - DM-EDP fragility functions
 - Repair cost functions
 - Economic consequence analysis
- **Not without modeling of deterioration for collapse assessment (better analytical models)**
- **Not without better probabilistic description of ground motion hazard in long period range**
 - $PGV = 1 - 2 \text{ m/sec}??!$

