

**Simpson Gumpertz & Heger Inc.** Consulting Engineers

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Building Engineering ■ Infrastructure and Special Structures ■ Construction Engineering

# Potential of PBEE in the Selection and Comparative Performance of Structural Systems

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# Purpose of the Study

**Capabilities of Current Practice**  
**How do Different Structural Systems Perform**  
**When Evaluating both Drift and Acceleration**

**VS**

**Potential of PBEE**  
**in Comparing Performance of Structural Systems**

# Framing Schemes Included in Comparative Study

- Moment Frame
- Buckling Restrained Braced Frame
- Viscously Damped Frame
- Base Isolated Braced Frames

# Project Overview

- 3, 9 and 20 story SAC buildings designed using 1997 UBC
- BRB's - 45 Ksi yield level.
- Viscously damped moment frames designed for 75% of the base shear and the dampers designed to meet the 2% drift criteria with a 0.4 velocity coefficient.
- Base Isolated Braced Frame – 2.5 sec. period
- Focus today on 3 Story Building Results

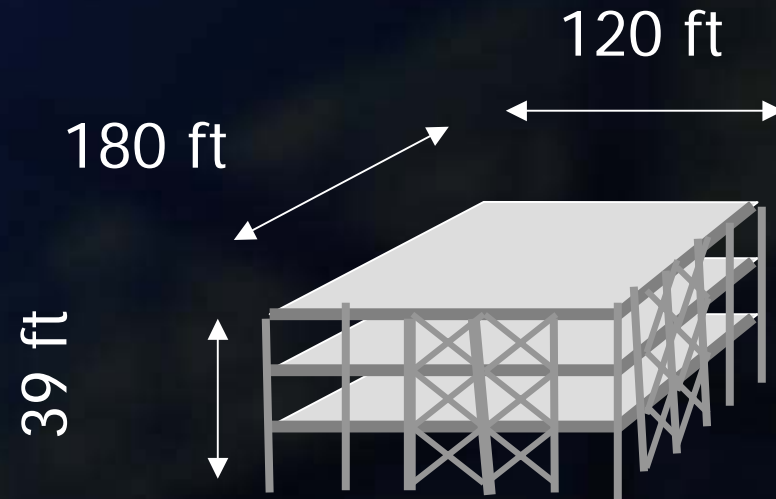
# Structural Systems

- 4 Standard Code Compliant Designs
  - Moment Frame, Buckling Restrained Braced Frame, Viscously Damped Frame and Base Isolated Brace Frame
- 2 Higher Performing (Lower Drift) "Hospital" Designs
  - BRB with  $R=3.5$  rather than 7
  - Viscous damper force increased from 133K to 220K

# 3 Story SAC Buildings

## Building Description:

- 3 Story Building
- 6 x 4 - 30' bays
- All Stories = 13' tall



# Earthquakes

- The design earthquakes were:
  - 50% in 50 year – moderate event
  - 10% in 50 year – design event
  - 2% in 50 year – maximum credible event
  - Near fault events – within 10 miles of a fault
- Key results from 5 time histories for each event are averaged – SAC time histories were used.
- Focus on the results of the 50% and 10% in 50 year events.
- All seismic resisting elements were modeled with their non-linear properties using RAM-Perform

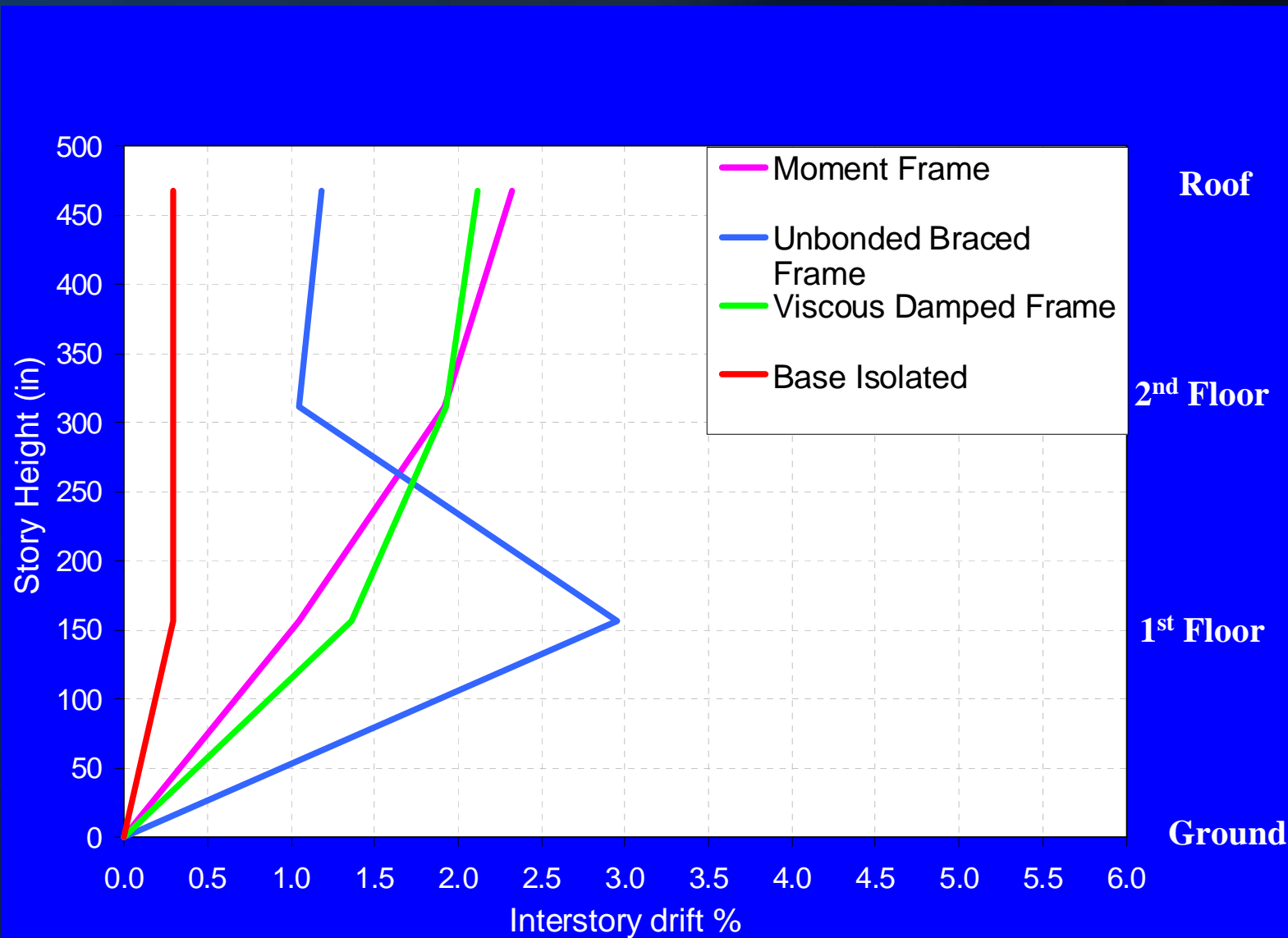
# 1<sup>st</sup> Key Performance Parameter

- Inter-story Drift – key code design parameter
  - Impacts structural frame, building facade, piping and ductwork, partitions
  - 2% drift with 13 ft. story height is 3 inches of inter-story displacement

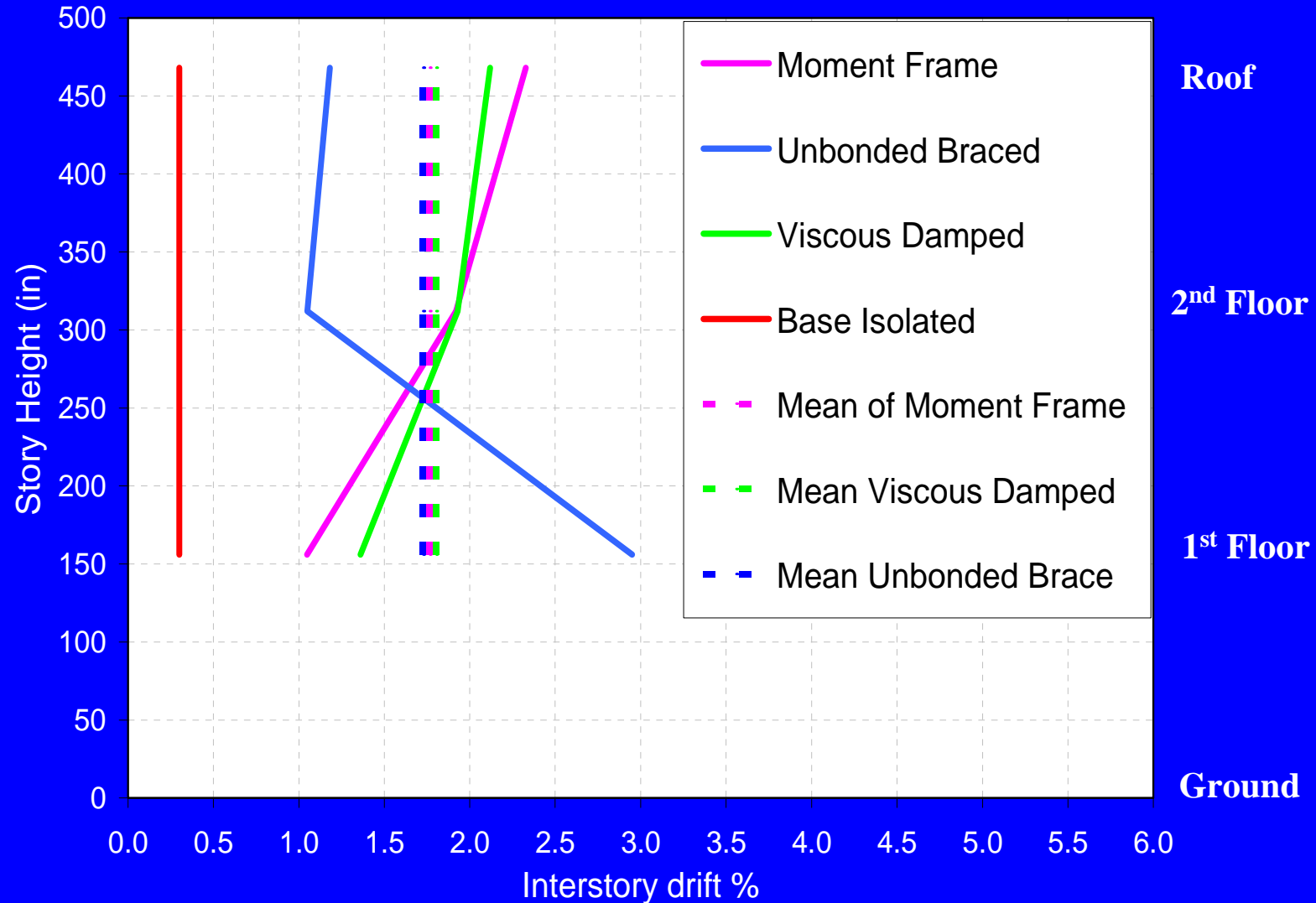


# % Interstory Drift

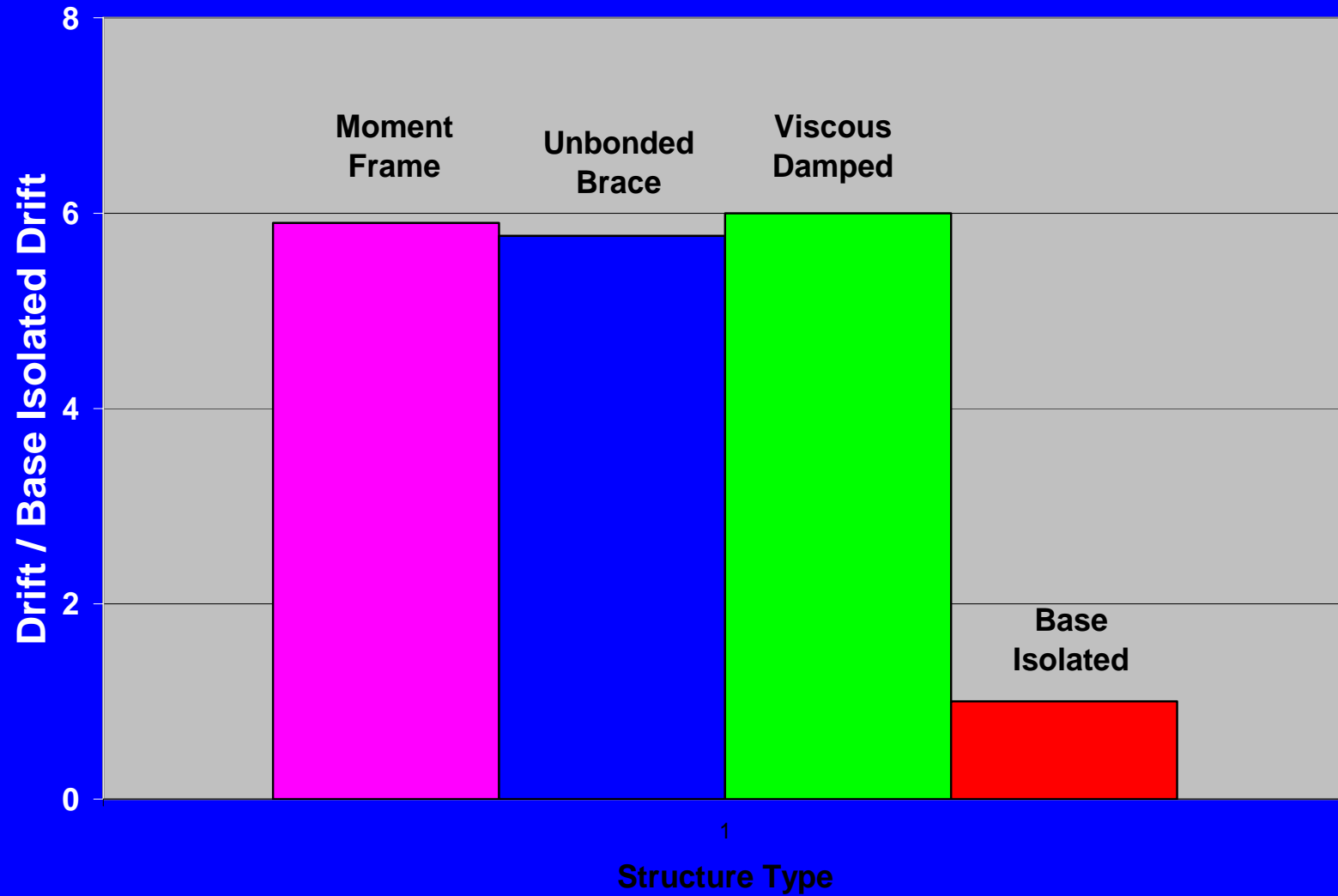
## 3 Story - 10% in 50yr Event



# % Interstory Drift including Mean of 3 Floors 3 Story - 10% in 50yr Event



# Normalized Mean Interstory Drift 3 Story - Design E/Q (10% in 50 Year Event)

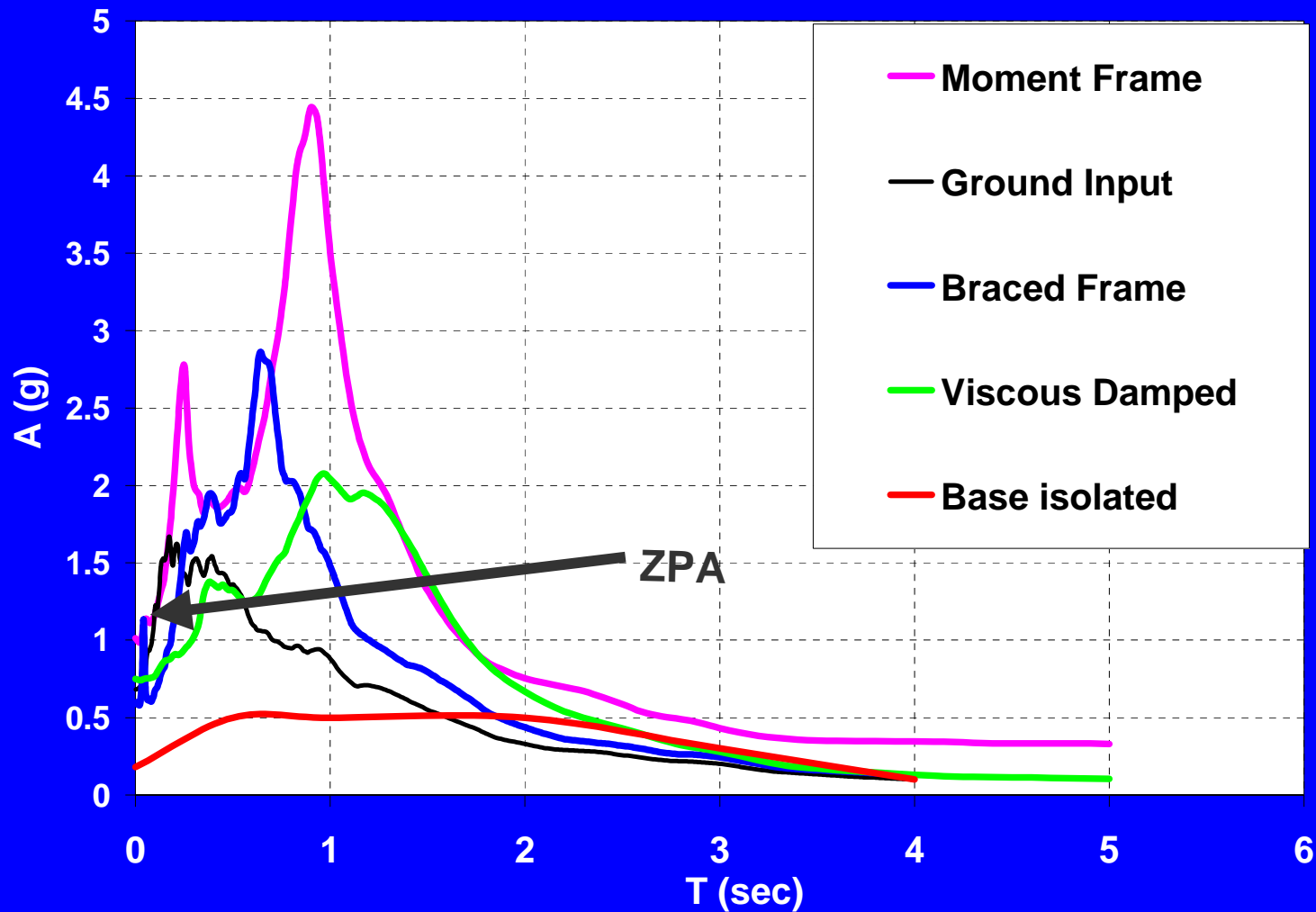


## 2<sup>nd</sup> Key Performance Parameter

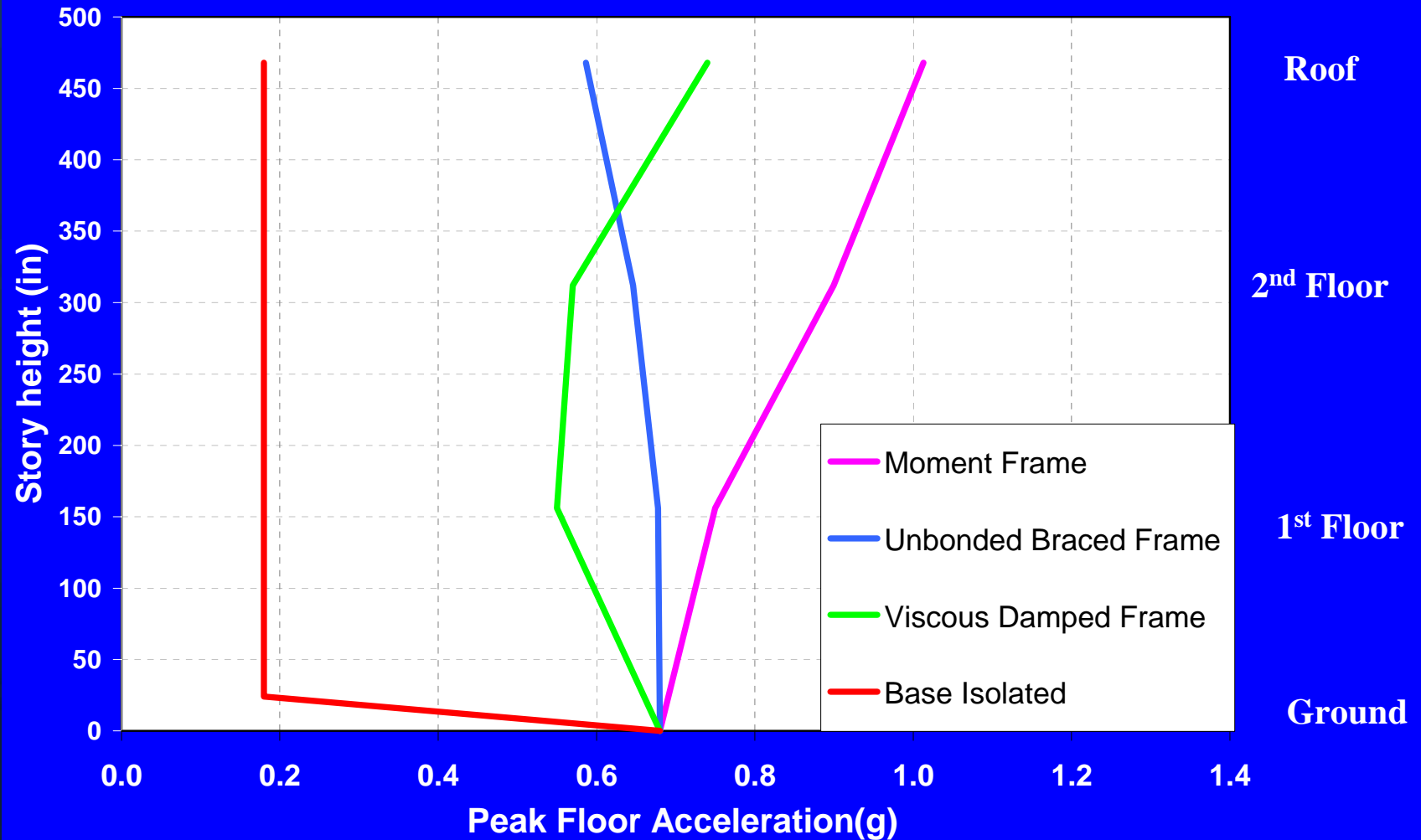
- Peak floor accelerations and floor response spectra – not required by code and rarely evaluated in the design process
  - Impacts contents, mechanical and electrical equipment, elevators and ceilings and lights.

# Peak Floor Acceleration (ZPA) Damage to Rigid Contents and MEP

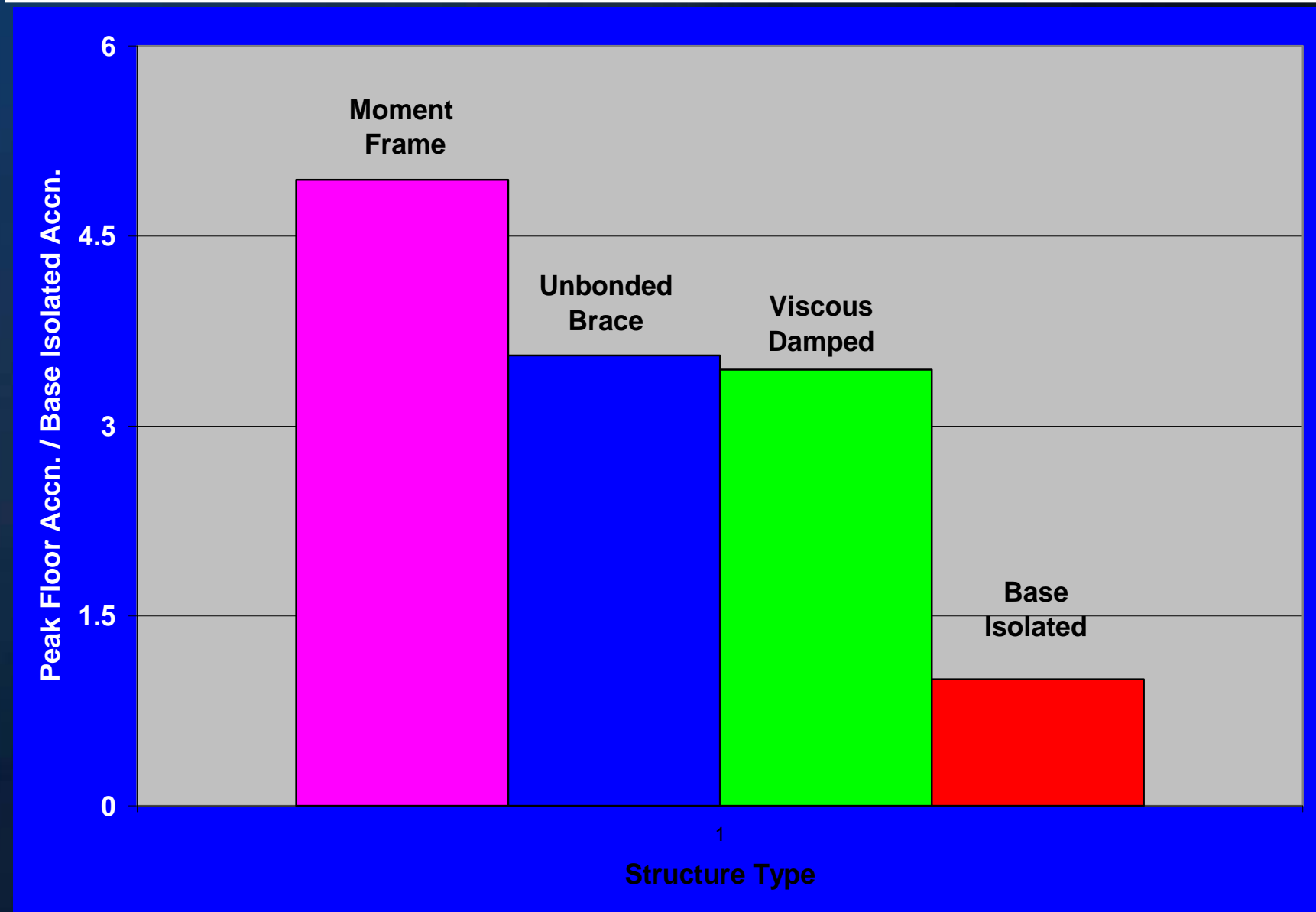
Average Floor Spectra for 10% in 50yr Event  
3 Story - 3rd Floor



# Average Peak Floor Acceleration (ZPA) 3 Story - 10% in 50yr Event

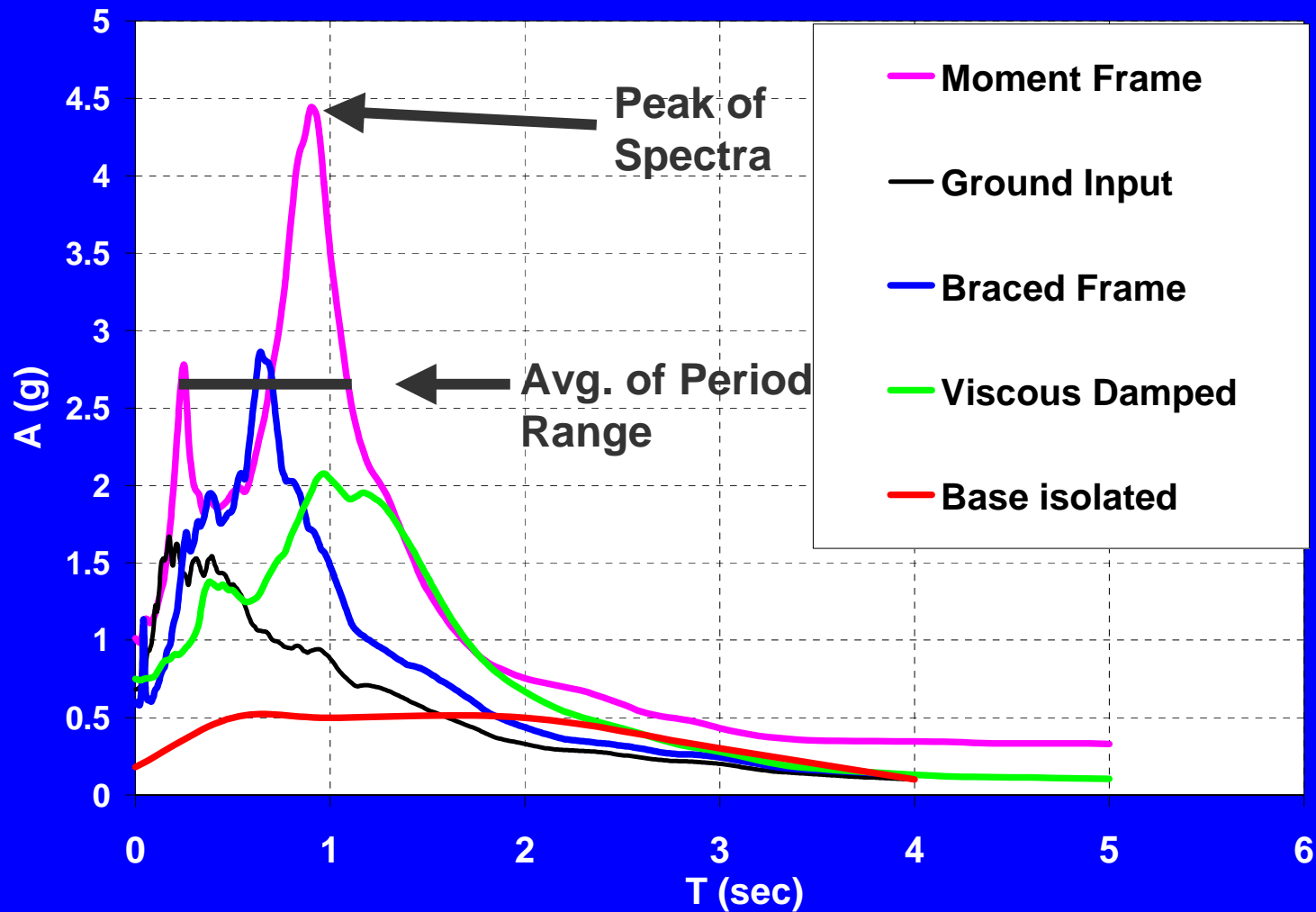


# Normalized Mean Peak Floor Acceleration 3 Story - Design E/Q (10% in 50 Year Event)



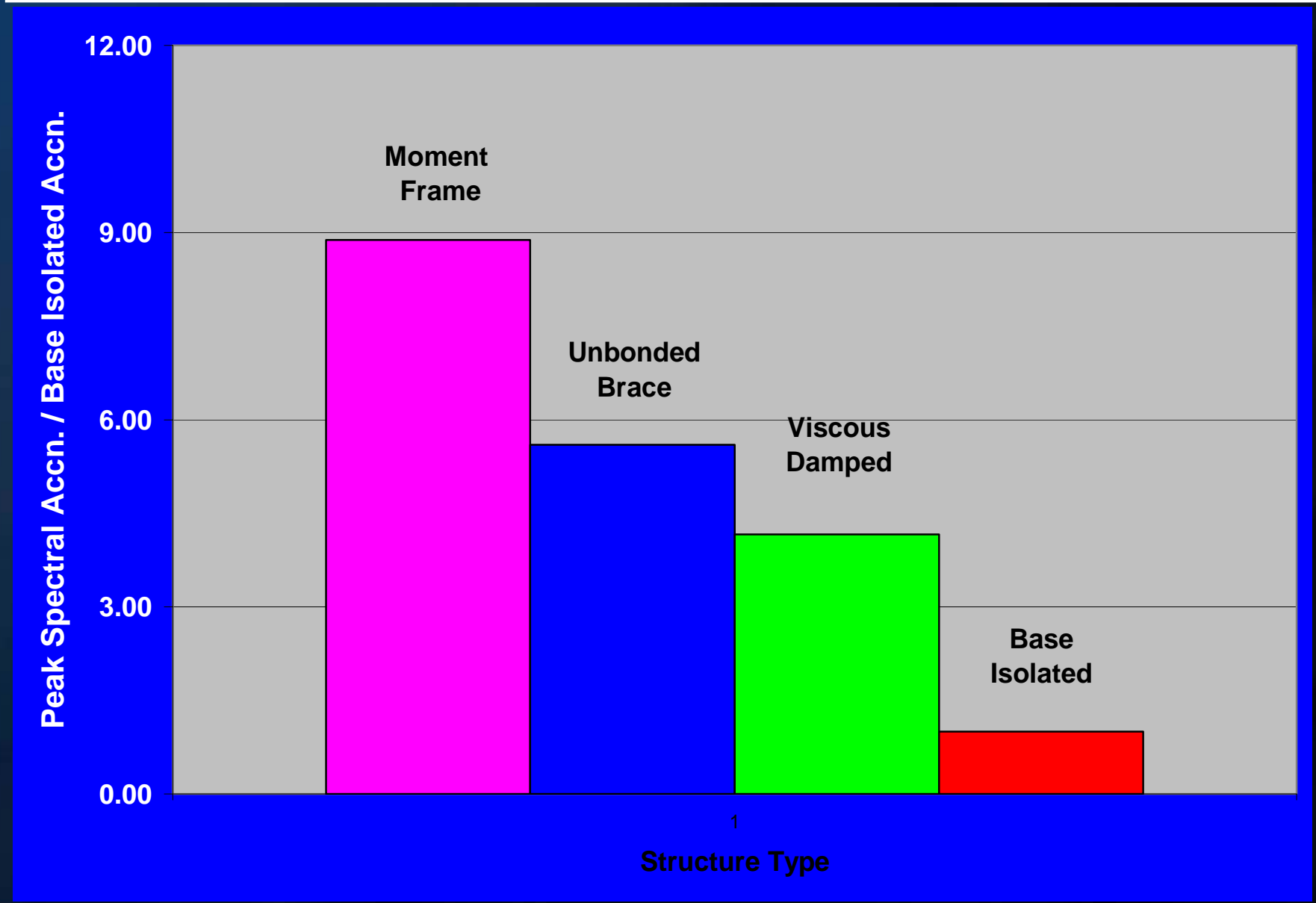
# Peak of Spectra or Average Over a Period Range of Floor Spectra - Damage to Flexible Contents and MEP

Average Floor Spectra for 10% in 50yr Event  
3 Story - 3rd Floor





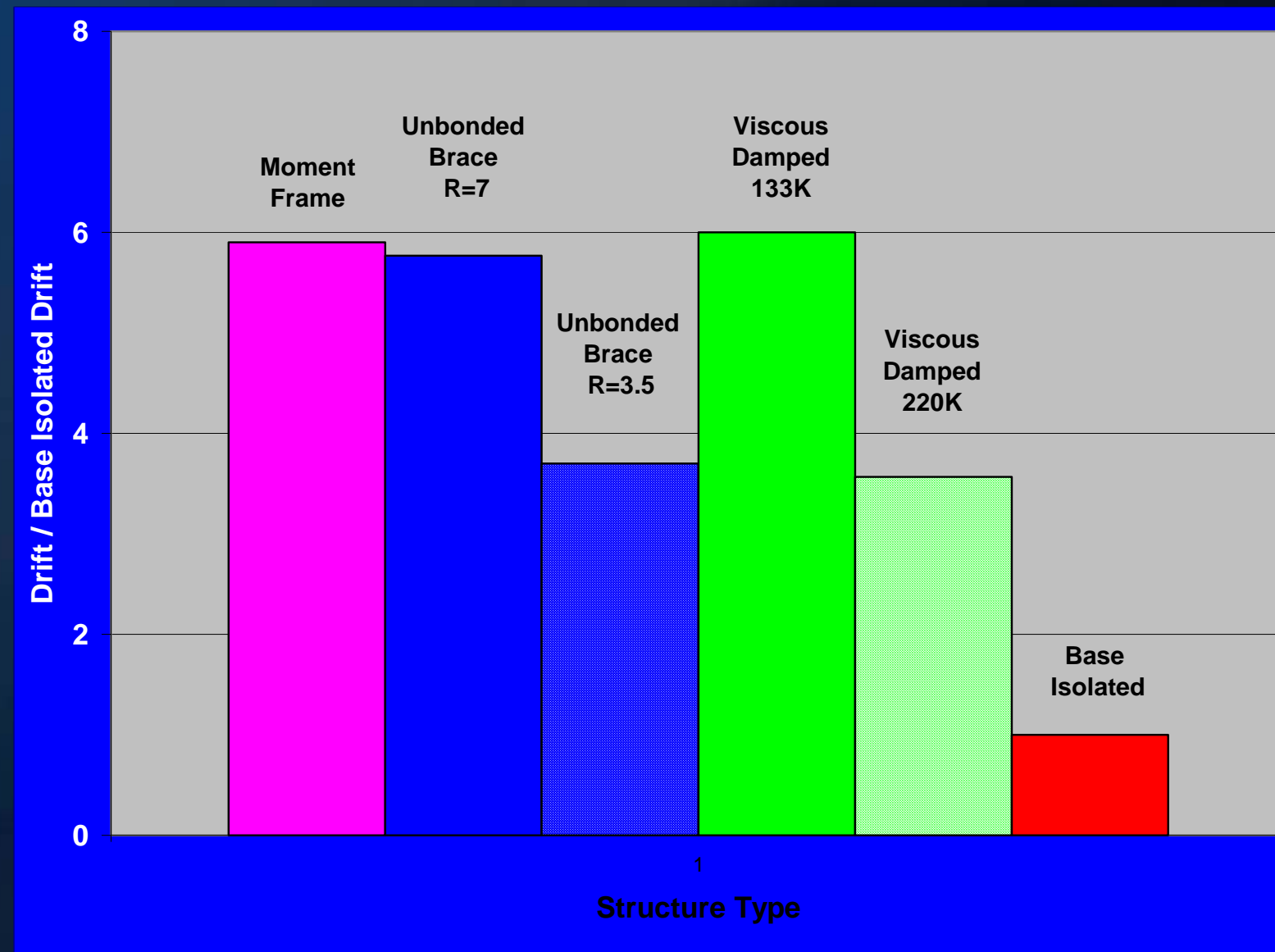
# Normalized Peak Floor Spectral Acceleration 3 Story - Design E/Q (10% in 50 Year Event)



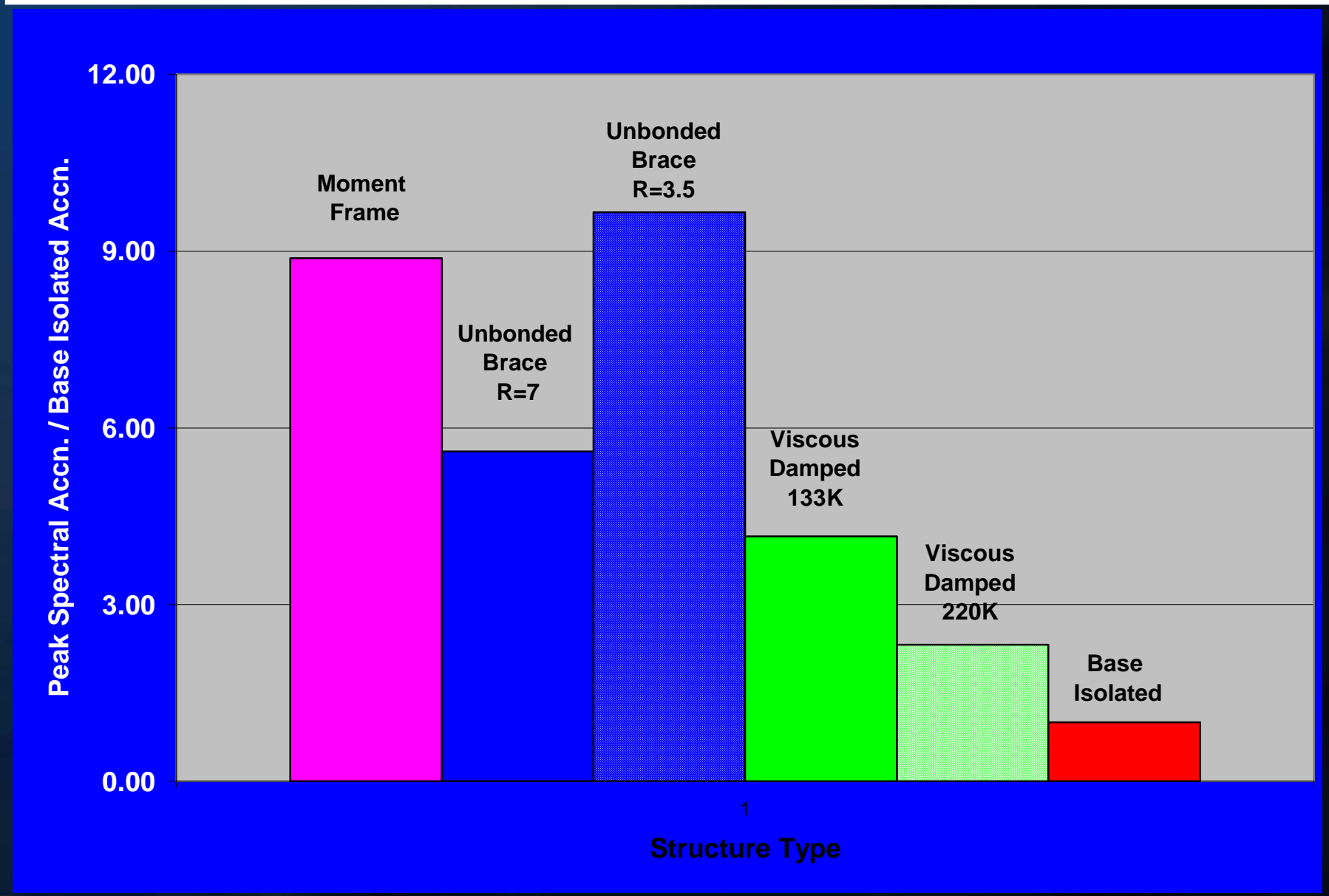
# Comparison of Improved Drift Performance Higher Performance

- BRB – reduce the R-Factor from 7 to 3.5
- Viscously damped frame – increase damper force from 133 K to 220 K

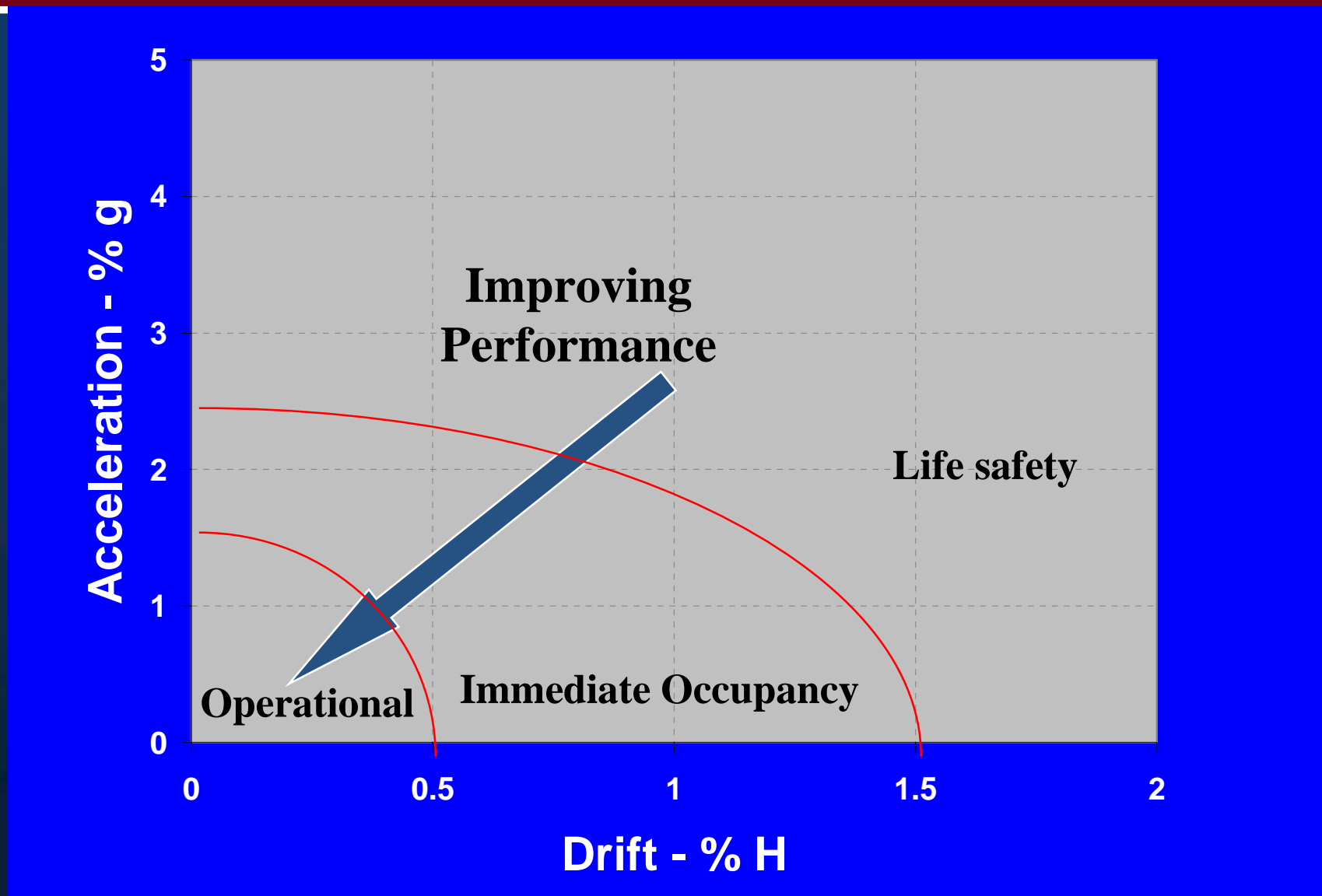
# Normalized Mean Interstory Drift 3 Story - 10% in 50 Year Event



# Normalized Peak of the Floor Response Spectra 3 Story - 10% in 50 Year Event

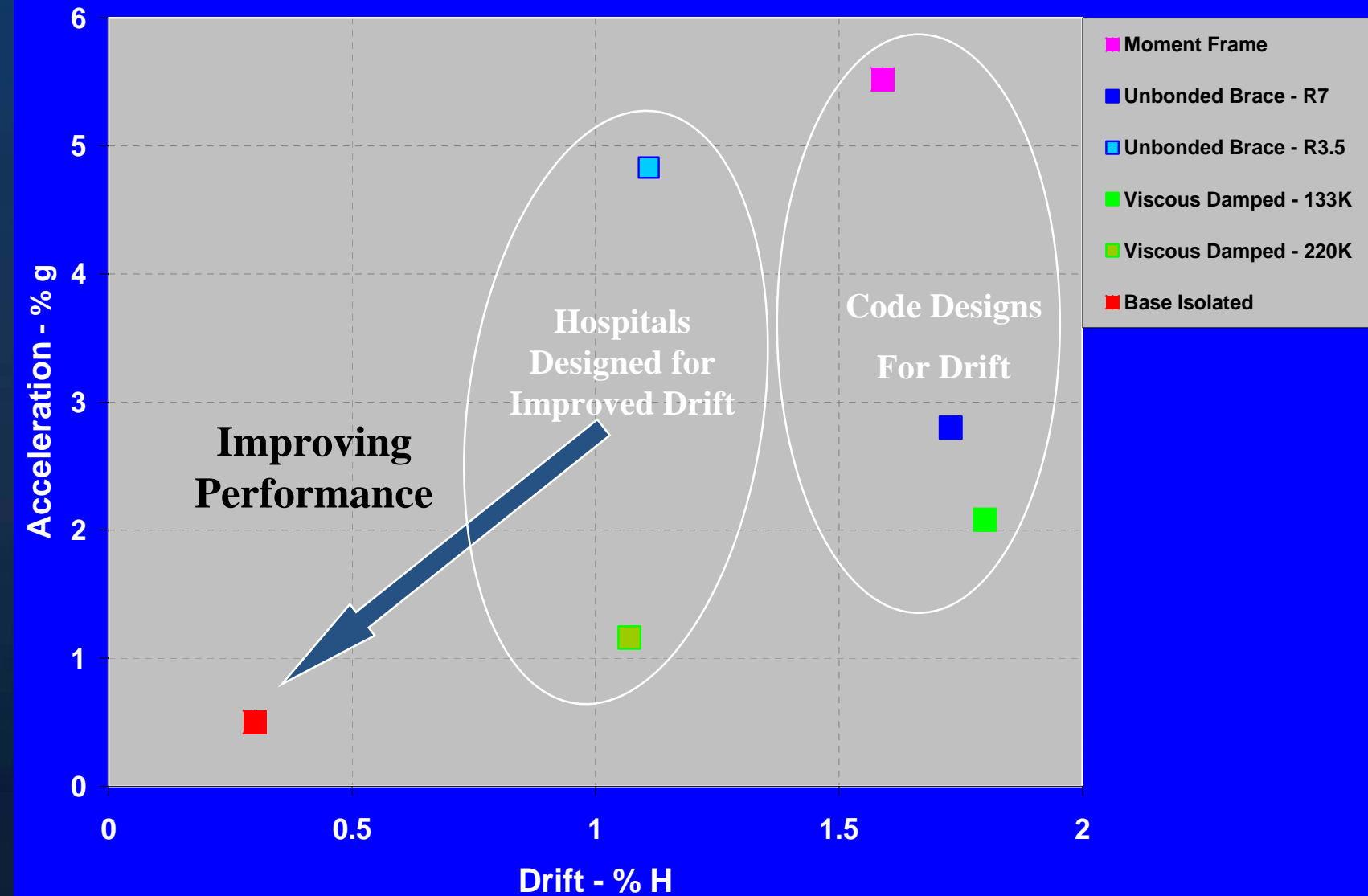


# PBD – Compare Both Drift and Acceleration



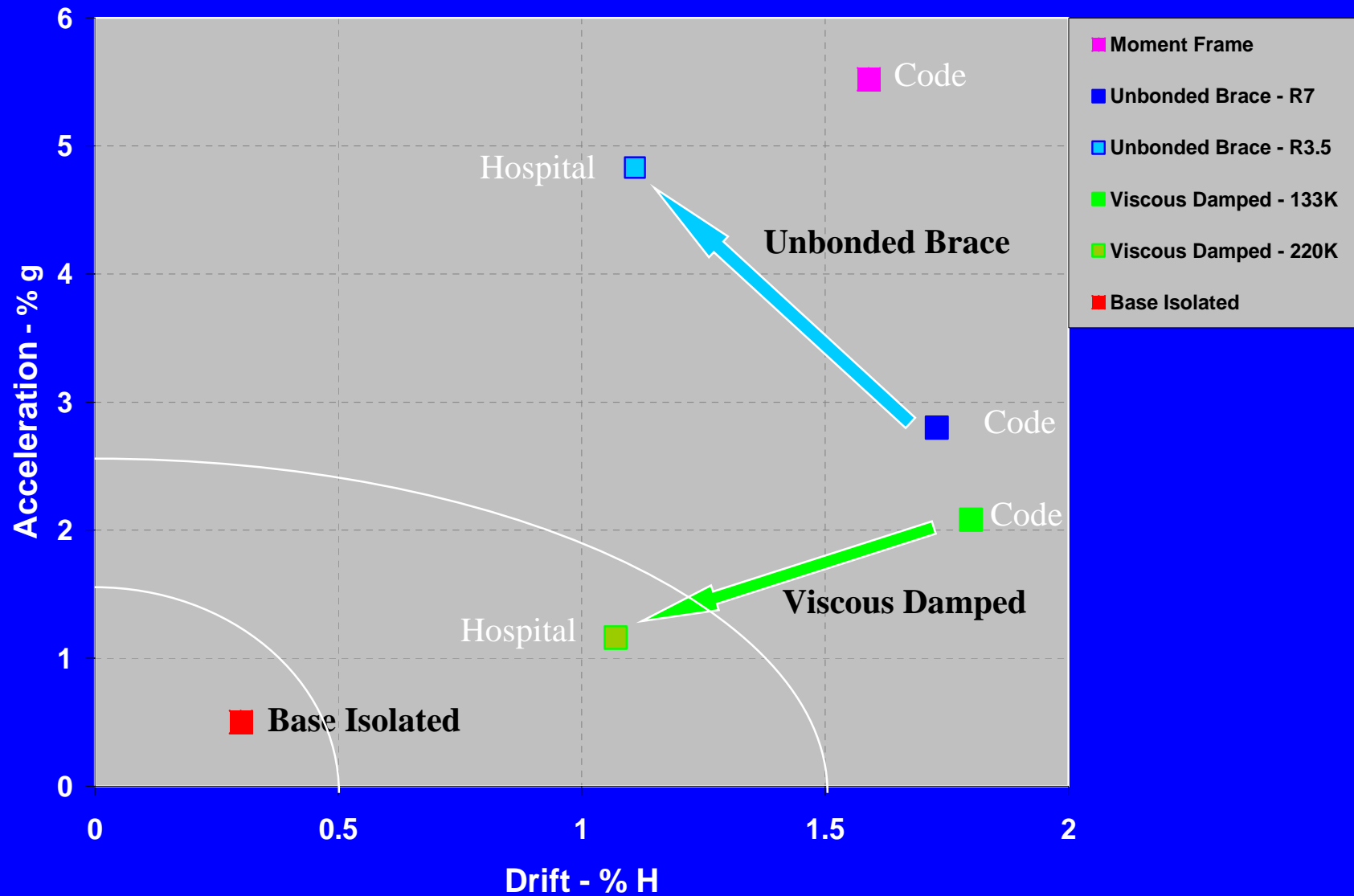
# Acceleration vs Drift

## 3 Story – Design Earthquake – SE Decision Event



# Acceleration vs Drift

## 3 Story – Design Earthquake



Relative Costs – 3 Story Office Building			
Code Design	Framing Cost \$ / sq. ft.	Total Cost \$ / sq. ft.	Increase Cost Over MF
Moment Frame	\$19.60	\$100.00	Baseline
Viscous Damped	\$20.20	\$100.60	0.6%
Buckling Restrained R=7	\$18.50	\$98.90	- 1.1%
Base Isolated	\$28.90	\$109.30	9.3%
Concentric Braced	\$15.90	\$96.30	- 3.7%
<b>Improved Drift Performance</b>			
Viscous damped	\$21.00	\$101.40	1.4%
Buckling Restrained R=3.5	\$19.30	\$99.70	- 0.3%

Note: A base isolated building is an additional \$39 / sq. ft. of plan area or \$13 / sq. ft. on a concentric braced frame.



## Relative Costs – 3 Story Higher Performance

Code Design	Framing Cost \$ / sq. ft.	Total Cost \$ / sq. ft.	Increase Cost Over MF
Moment Frame	\$27.10	\$250.00	Baseline
Viscous Damped	\$27.70	\$250.60	0.2%
Buckling Restrained R=7	\$26.00	\$248.90	- 0.44%
Base Isolated	\$39.00	\$261.90	4.7%
Concentric Braced	\$23.40	\$246.30	- 1.5%
<b>Improved Drift Performance</b>			
Viscous damped	\$28.50	\$251.40	0.56%
Buckling Restrained R=3.5	\$26.80	\$249.70	- 0.1%

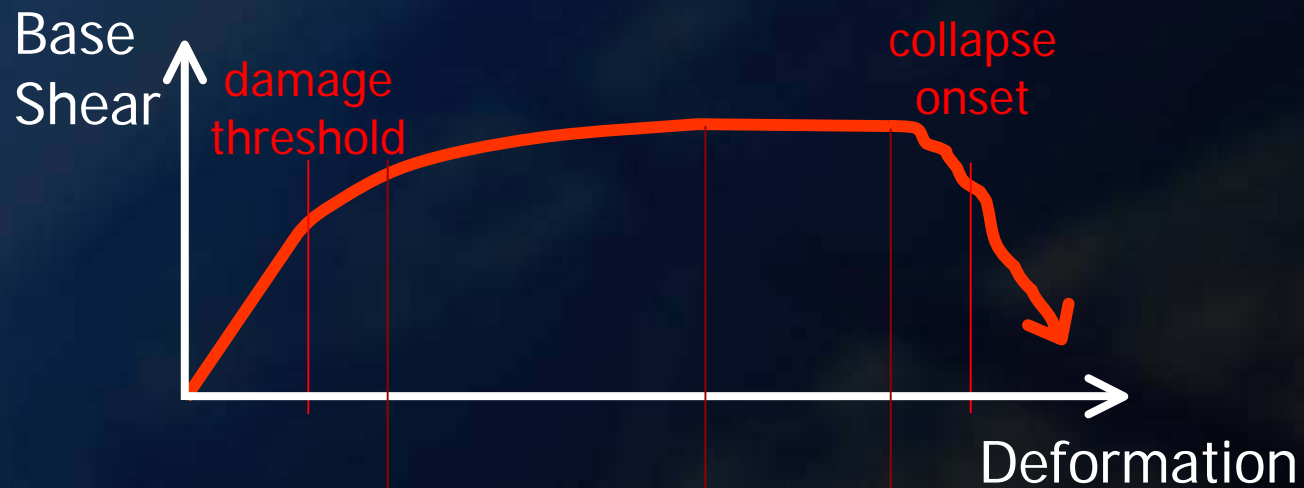
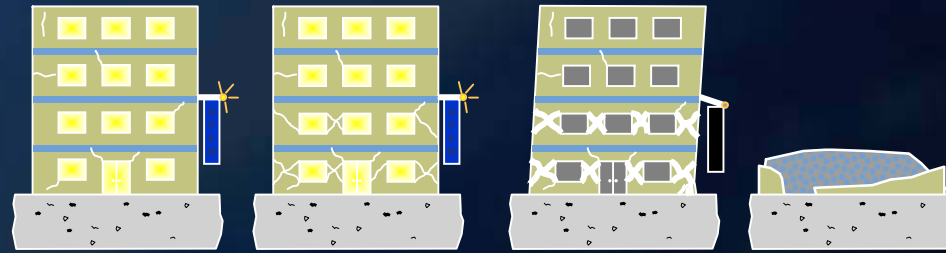
Note: Base isolation cost is \$39 / sq. ft. of plan area or an additional \$13 / sq. ft for a concentric braced frame.

## Potential of PEER & ATC 58 PBEE

Conduct a PBEE study to assess the performance of the 6 structural systems using the data developed as part of ATC 58 and PEER project



# Performance measures for decision-making



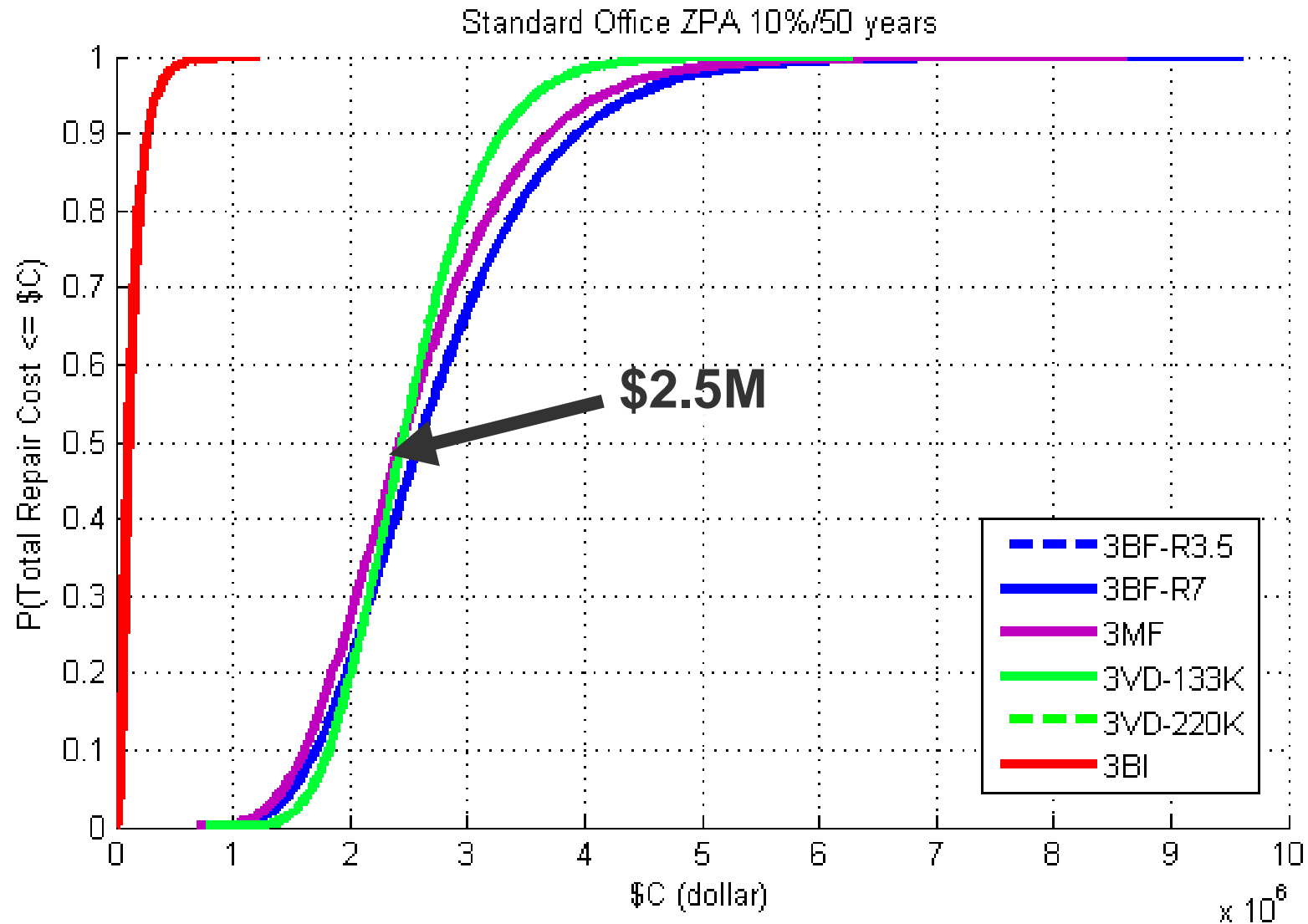
0	25%	50%	100%	\$, % replacement	
0.0	0.0001	0.001	0.01	0.25	Casualty rate
0	1	7	30	180	Downtime, days

# Office Building

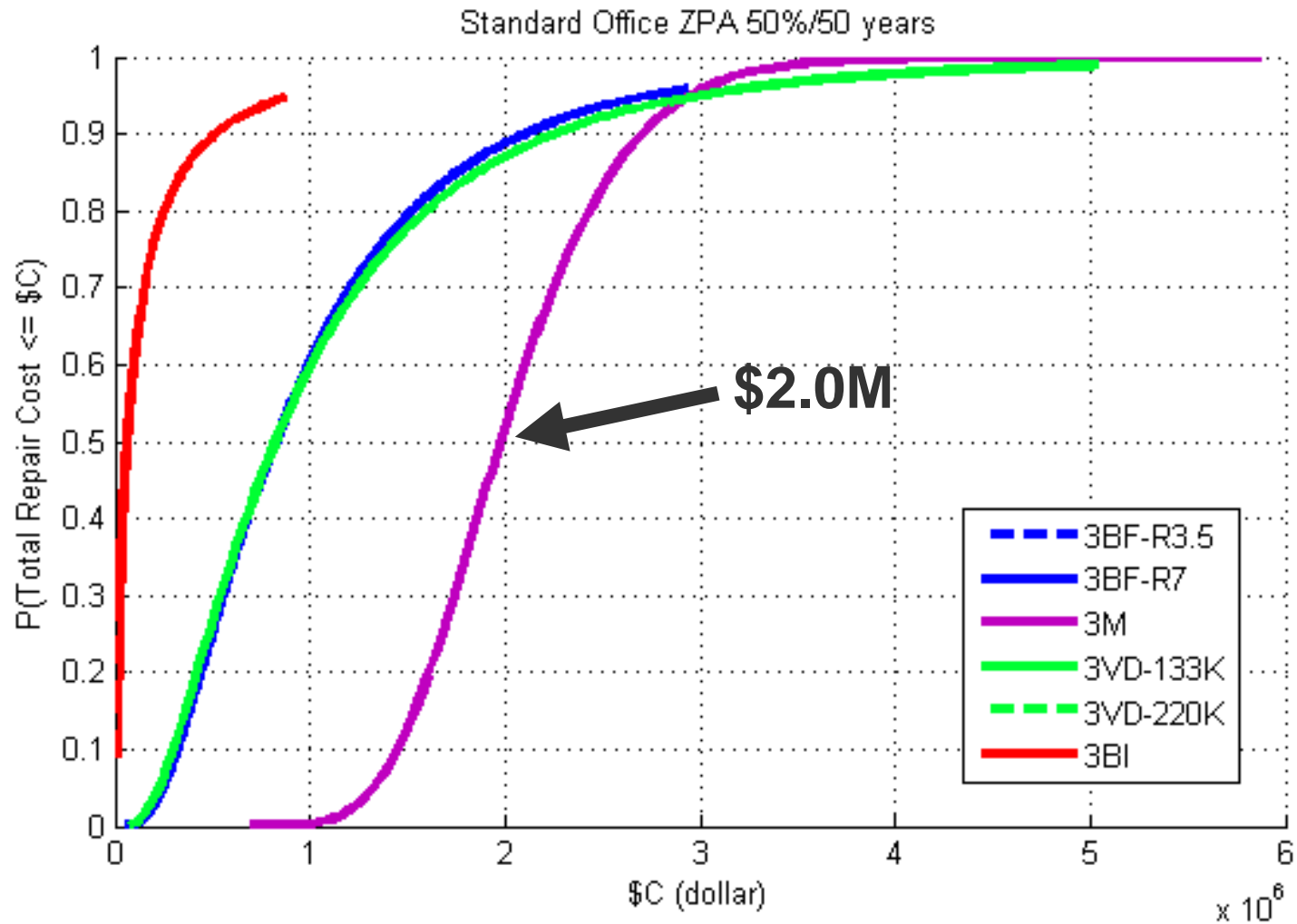
- Original damage data was based on the premise that the building was an office building ((70,000 sq. ft.) with a computer center on 3<sup>rd</sup> floor - \$13M building cost - \$25 /sq. ft. contents and total damage potential of \$9M



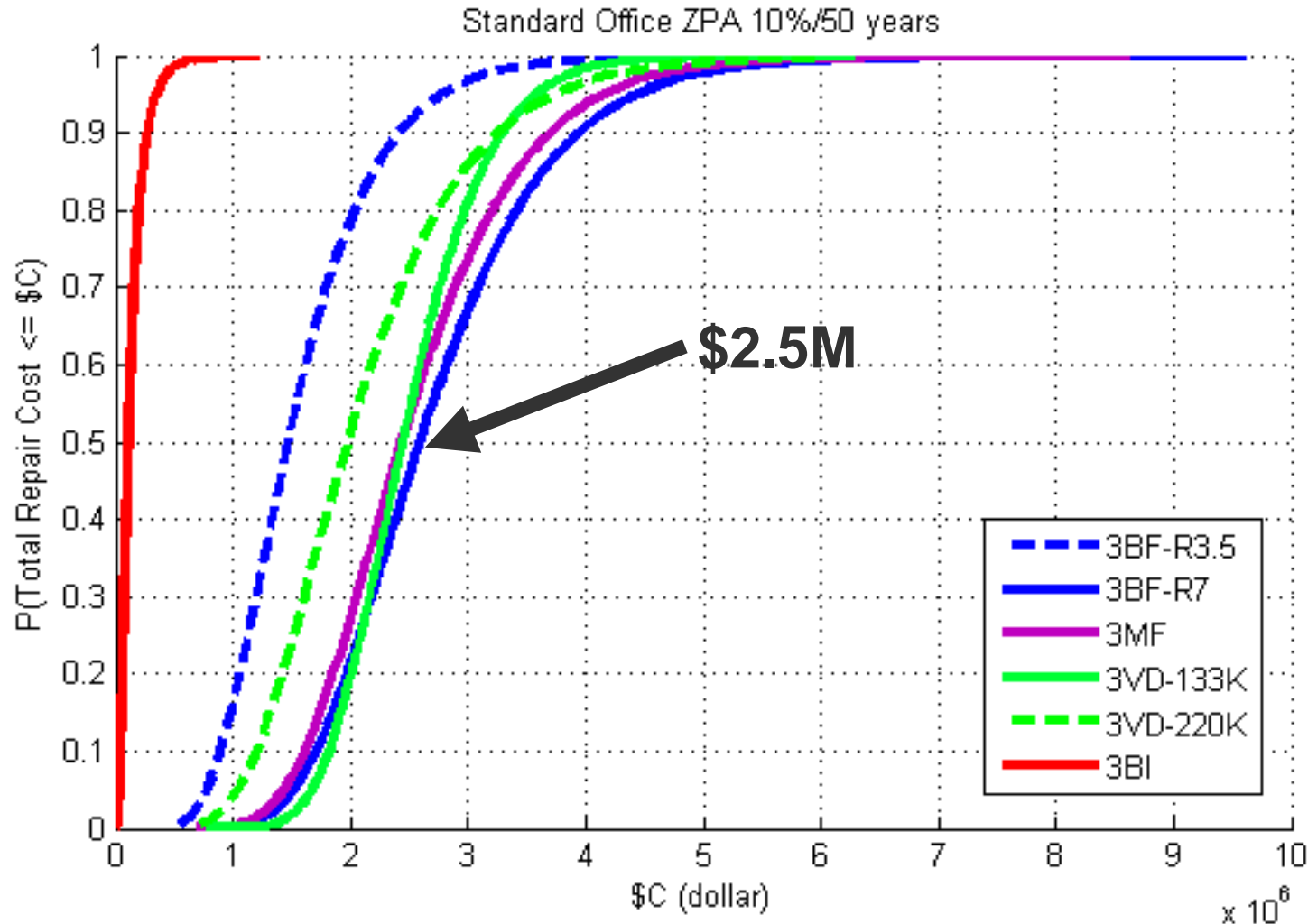
# Office Building – 10% / 50 Years Code Designs



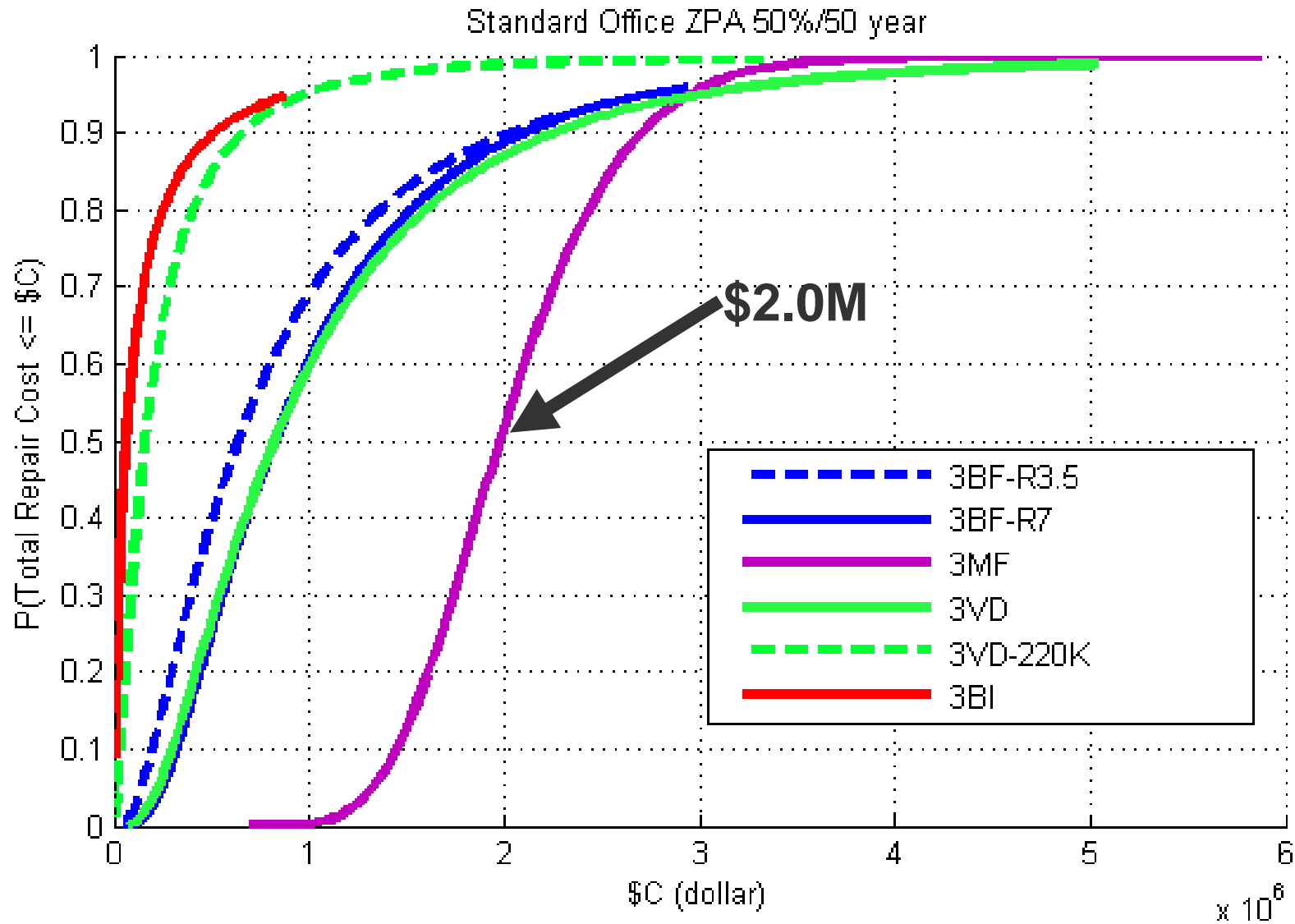
# Office Building – 50% / 50 Years Code Designs



# Office Building – 10% / 50 Years Code & Higher Performance Designs



# Office Building – 50% / 50 Years Code & Higher Performance Designs



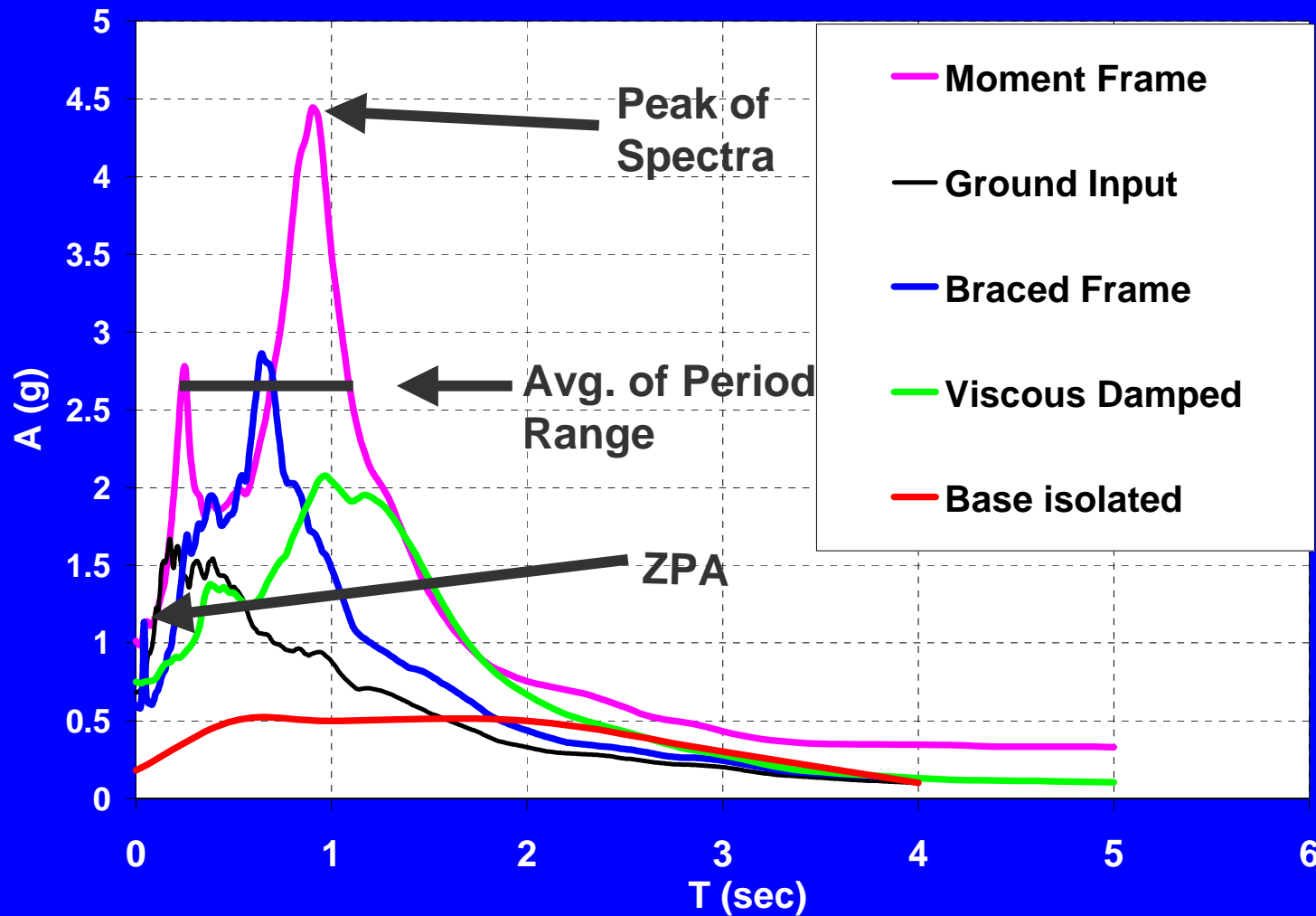


# High Tech Manufacturing

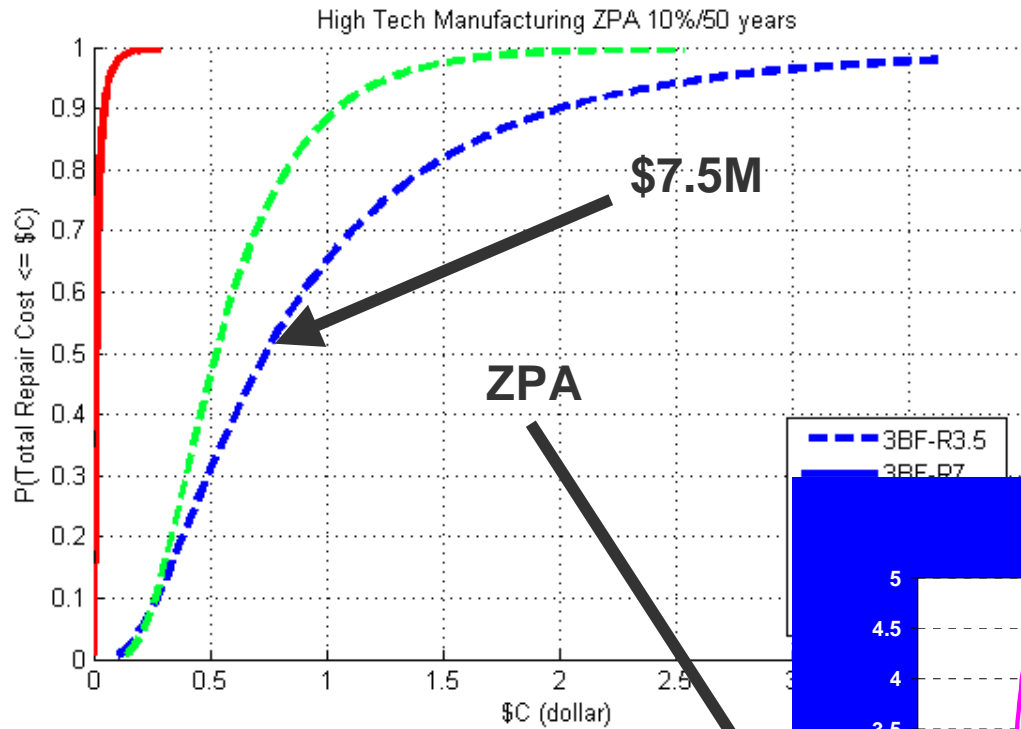
- Assumed the same office structure became a high tech manufacturing facility with \$33M in contents. The only change was the potential damage to the contents – all other damage functions remained the same
- A basic limit was we did not change the structure damage states and repair costs so results are interesting from a comparative perspective but not accurate

# Use of ZPA, Peak of Spectra or Average Over a Period Range of Floor Spectra for Acceleration Damage

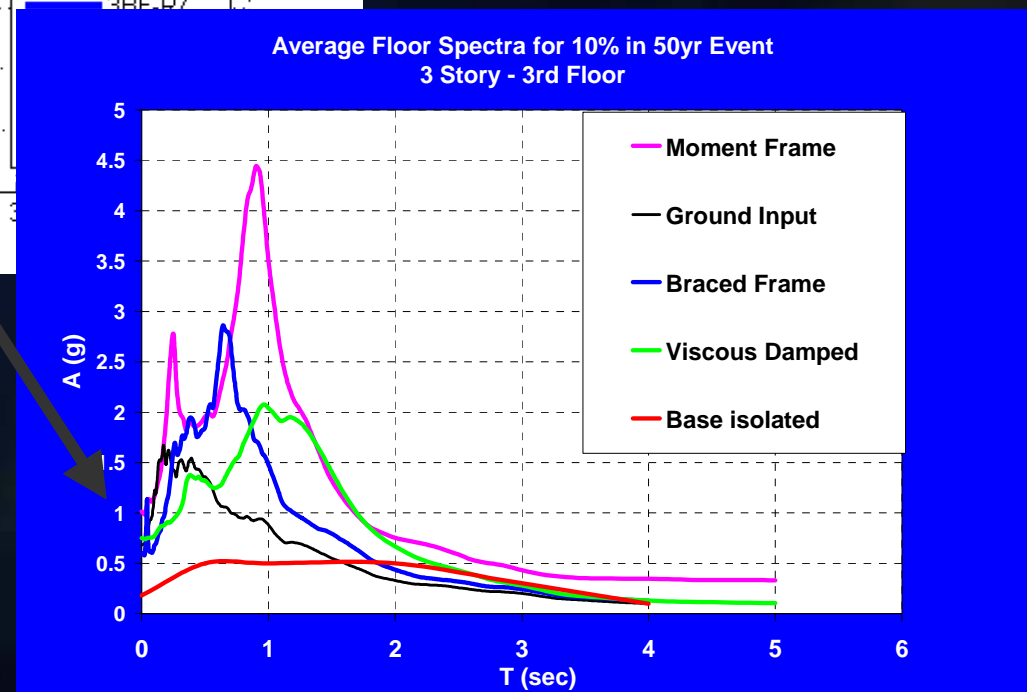
Average Floor Spectra for 10% in 50yr Event  
3 Story - 3rd Floor



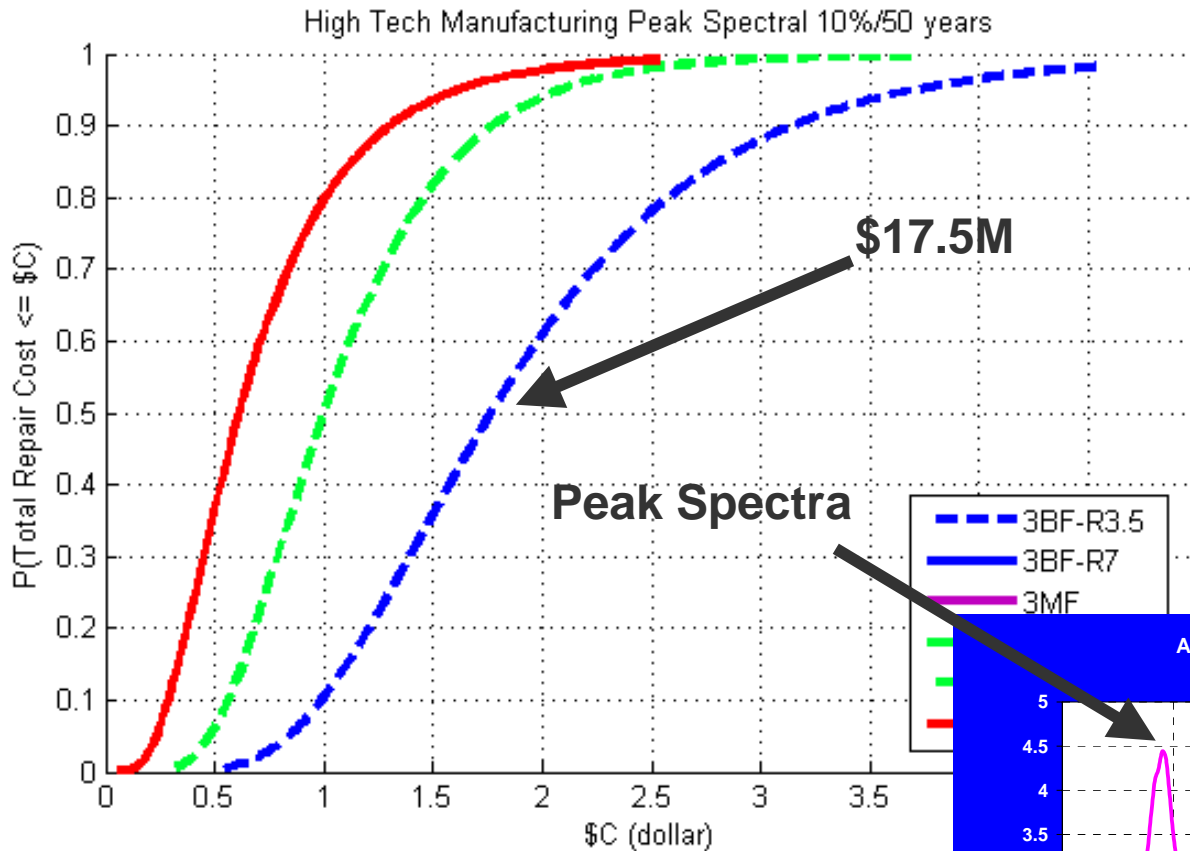
# High Tech Manufacturing – 10% / 50 Years Higher Performance Designs – ZPA



Office Building -  
\$2.5M

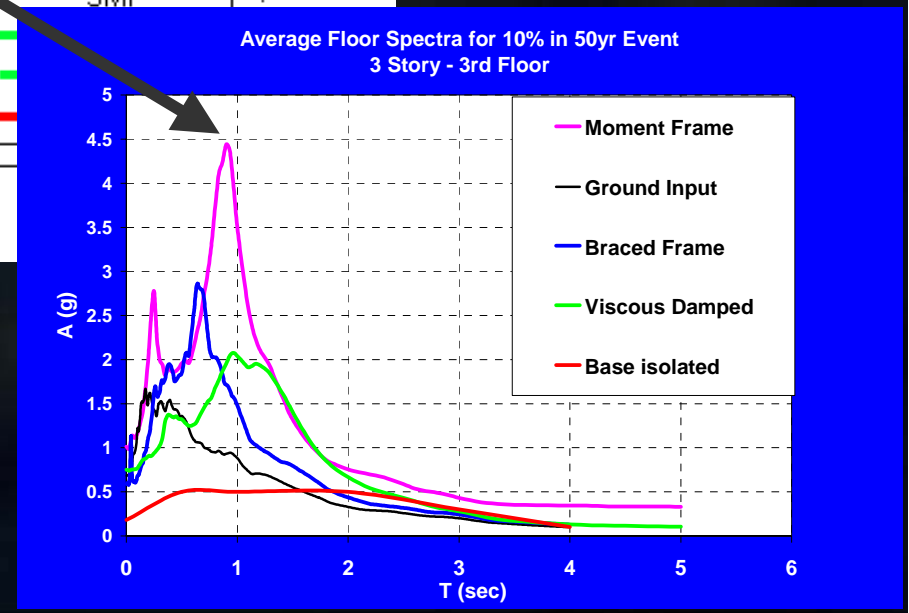


# High Tech Manufacturing – 10% / 50 Years Higher Performance Designs – Peak Spectra

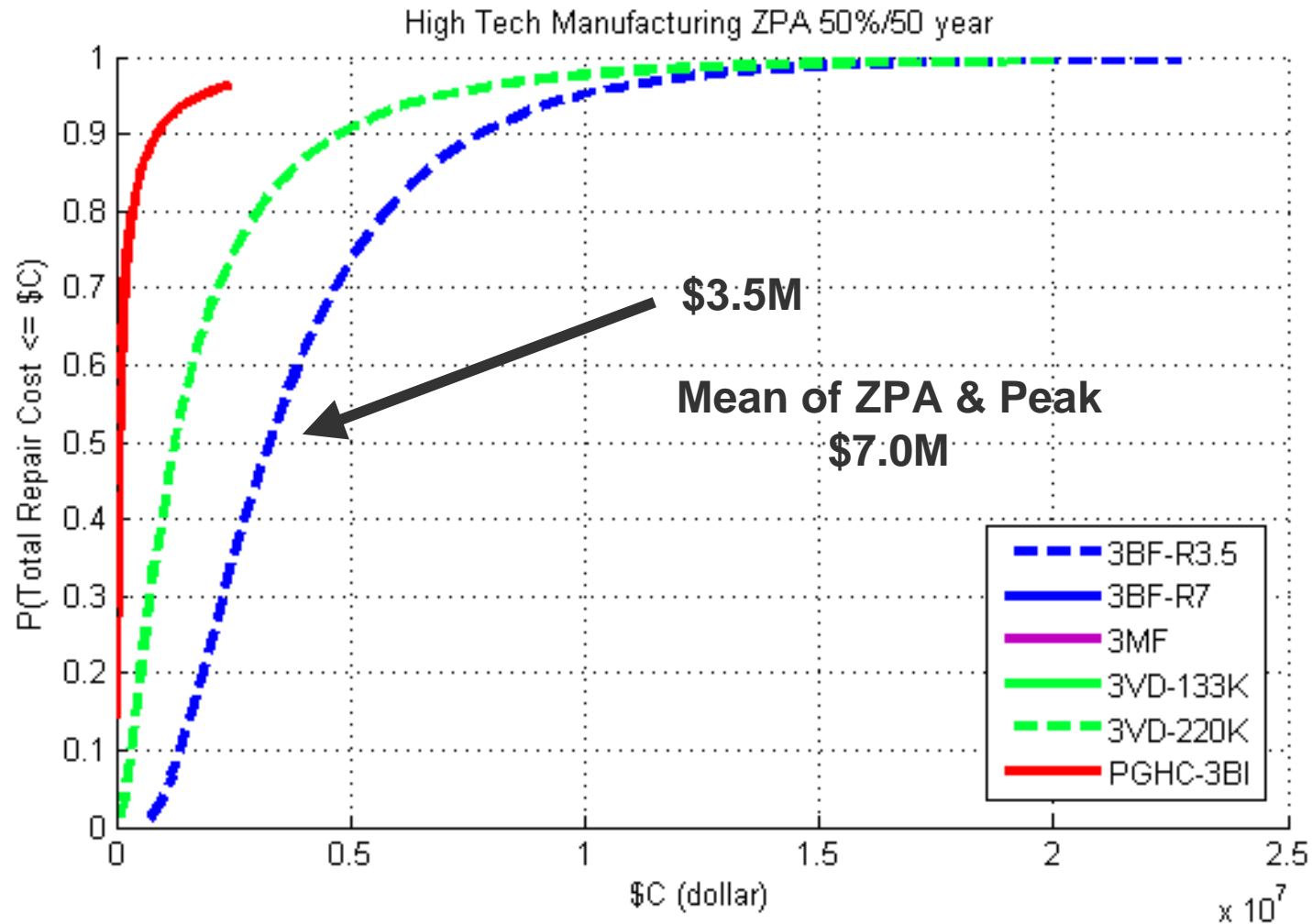


Office Building  
\$2.5M

Mean – ZPA & Peak  
\$12.5M



# High Tech Manufacturing – 50% / 50 Years Higher Performance Designs – ZPA



# Various Expressions of Repair Costs



\$5.2M

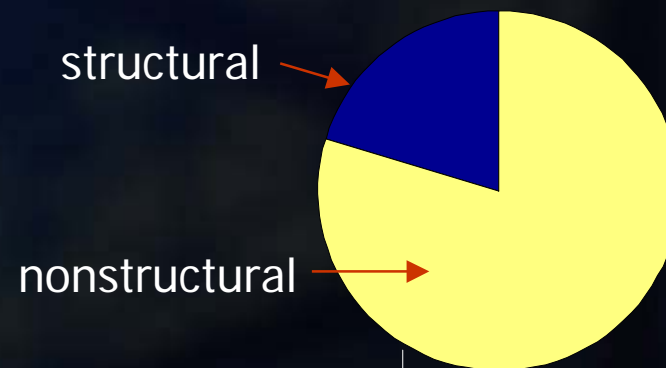
(a) Expected cost for M7 scenario

\$8.0M

(b) 90% confidence cost does not exceed threshold for 475-yr return period hazard level

\$240,000

(c) Mean cumulative annual total cost of damage



(d) Contributions to total cost for scenario

# Performance-Based Earthquake Engineering

A great tool to assess the relative seismic performance of different structural systems and aid in the selection of the structural system for a particular project

Much work remains to be done on the non-structural and content fragility functions

# Thank You

**Designers should consider floor accelerations  
when they next select or recommend  
a Structural System  
until**

**PEER and ATC 58 have completed the tools to perform PBEE**

**Great Promise of PBEE  
in Transforming Decision Making Process of SE's**